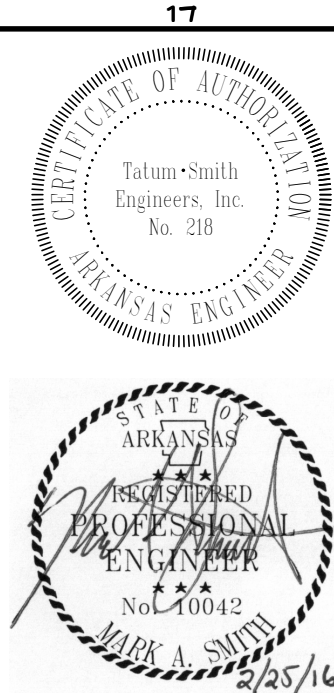


	6	7	8	9
STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL (IBC 1705.2.2)				
STEEL ROOF AND FLOOR DECKS (IBC TABLE 1705.2.2):				
Verification and inspection	Continuous	Periodic	Detailed instructions	
Material verification of cold-formed steel deck	----	X	Confirm that identification markings are provided to conform to ASTM standards specified on construction documents.	
Floor and roof deck welds	----	X	Visual inspection is required to confirm that weld meets acceptance criteria of AWS D1.3. Welder qualifications should also be verified.	
WELDING OF REINFORCING STEEL (IBC TABLE 1705.2.2)				
Verification and inspection	Continuous	Periodic	Detailed instructions	
Verification of weldability	----	X	Verify weldability of reinforcing steel based upon carbon equivalent and in accordance with AWS D1.4.	
Reinforcing steel in intermediate or special moment frames, and boundary elements of special structural walls	X	----	-----	
Shear reinforcement	X	-----	-----	
Other reinforcing steel	----	X	Visually inspect all welds in accordance with AWS D1.4.	
COLD-FORMED STEEL CONSTRUCTION (IBC 1705.2.2.1, 1705.10.3, and 1705.11.3):				
Verification and inspection	Continuous	Periodic	Detailed instructions	
Trusses spanning > 60-feet	----	N/A	Verify that temporary and permanent truss bracing is installed in accordance with approved truss package. Performed by code inspection firm.	
Wind-force-resisting systems or seismic-force-resisting systems	----	X	Periodic inspections of welding operations, if fastener spacing is < 4" o.c. Verify that proper screw attachment, bolting, anchoring and other fastening of shear walls, diaphragms, drag struts, braces, shear panels and holdowns has occurred. Performed by code inspection firm.	
CONCRETE CONSTRUCTION (IBC 1705.3 & 1705.12.1)				
Verification and inspection	Continuous	Periodic	Detailed instructions	
Reinforcing steel, including prestressing tendons	----	X	Verify prior to placing concrete that reinforcing is of specified type, grade and size; that it is free of oil, dirt and rust; that it is located and spaced properly; that hooks, bends, ties, stirrups and supplemental reinforcement are placed correctly; that lap lengths, stagger and offsets are provided; and that all mechanical connections are installed per the manufacturer's instructions and/or evaluation report.	
Cast-in bolts & embeds	----	X	Inspection of anchors or embeds cast in concrete is required when allowable loads have been increased or where strength design is used.	
Post-installed anchors or dowels	----	X	All post-installed anchors/dowels shall be specially inspected as required by the approved ICC-ES report.	
Use of required mix design	----	X	Verify that all mixes used comply with the approved construction documents; ACI 318: Ch. 4, 5.2-5.4; and IBC 1904.3, 1913.2, 1913.3.	
Concrete sampling for strength tests, slump, air content, and temperature	X	-----	-----	
Concrete & shotcrete placement	X	-----	-----	
Curing temperature and techniques	----	X	Verify that the ambient temperature for concrete is kept at > 50°F for at least 7 days after placement. High-early-strength concrete shall be kept at > 50°F for at least 3 days. Accelerated curing methods may be used (see ACI 318: 5.11.3). The ambient temperature for shotcrete shall be > 40°F for the same period of time as noted for concrete. Shotcrete shall be kept continuously moist for at least 24 hours after shotcreting. All concrete materials, reinforcement, forms, fillers, and ground shall be free from frost. In hot weather conditions ensure that appropriate measures are taken to avoid plastic shrinkage cracking and that the specified water/cement ratio is not exceeded.	
Pre-stressed concrete	N/A	-----	-----	
Erection of precast concrete	----	N/A	Verify that all precast elements are lifted, assembled and braced in accordance with the approved construction documents.	
Strength verification	----	N/A	Verify that adequate strength has been achieved prior to the removal of shores and forms or the stressing of post-tensioned tendons.	
Formwork	----	X	Verify that the forms are placed plumb and conform to the shapes, lines, and dimensions of the members as required by the approved construction documents.	
Reinforcement complying with ASTM A 615 in special moment frames, special structural walls and coupling beams	----	N/A	Verify that ASTM A 615 reinforcing steel used in these areas complies with ACI 318: 21.1.5.2 by means of certified mill test reports. If this reinforcing steel is to be welded chemical tests shall be performed in accordance with ACI 318: 9.5.2.	
SOILS CONSTRUCTION (IBC 1705.6)				
Verification and inspection	Continuous	Periodic	Detailed instructions	
Verify subgrade is adequate to achieve design bearing capacity	----	X	Prior to placement of concrete.	
Verify excavations extend to proper depth and material	----	X	Prior to placement of compacted fill or concrete.	
Verify that subgrade has been appropriately prepared prior to placing compacted fill	----	X	Prior to placement of compacted fill.	
Perform classification and testing of compacted fill materials	----	X	All materials shall be checked at each lift for proper classifications and gradations not less than once for each 10,000ft ² of surface area.	
Verify proper materials, densities and lift thicknesses during placement and compaction.	----	X	-----	
INSPECTION OF FABRICATORS (IBC 1704.2)				
Verification and inspection	Continuous	Periodic	Detailed instructions	
Verify fabricator maintains detailed fabrication and quality control procedures	----	X	-----	
Review procedures for completeness and adequacy relative to the Code requirements for the fabricators scope of work	----	X	-----	
Submit of certificate of compliance	----	X	Where work is done on premises of "Approved" fabricator, Fabricator shall submit a Certificate of Compliance to the building official stating work was performed in accordance with the approved construction documents.	

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Structural Steel General Notes

- 5100.1.**
All detailing, fabrication and erection of structural steel shall conform to the requirements of the AISC Specification for the Design, Fabrication and Erection of Structural Steel for Buildings.
- 5100.2.**
Wide Flanges shall conform to ASTM A992 with a yield strength of 50 ksi.
- 5100.3.**
Hollow Structural Section (HSS) shall conform to ASTM A500, Grade B with a yield strength of 46 ksi.
- 5100.4.**
Headed stud anchors (HSA) shall conform to ASTM A108.
- 5100.5.**
All other structural steel shall conform to the requirements of ASTM A36. Angle, plate and beam lintels at exterior wall openings shall be hot-dipped galvanized.
- 5100.6.**
All anchor rods shall conform to ASTM F1554, Grade 36. Nuts for anchor rods shall conform to ASTM A563, Grade A, heavy hex and anchor rod washers shall conform to ASTM F444.
- 5100.7.**
All welding shall conform to the Specifications of the American Welding Society. Welding electrodes shall be E-70 low hydrogen series. Welding shall be done by a certified welder.
- 5100.8.**
High strength bolts shall typically be 3/4"Ø bolts conforming to ASTM A325. Connections shall be designed as bearing type with threads in shear plane. Holes shall be 1/16" larger than bolt size.
- 5100.9.**
All bolts shall be tightened to a snug-tight condition. A snug tight condition is defined as the tightness attained by a few impacts of an impact wrench or the full effort of a man using an ordinary spud wrench. All connected elements must be brought into snug contact.
- 5100.10.**
Bearing ends of all columns shall be square cut.
- 5100.11.**
No openings shall be cut in structural members unless shown on the drawings.
- 5100.12.**
Steel frame is non-self-supporting and column anchor rods are designed for a completed condition only. Metal roof deck, elevated slabs, steel moment frames, braced frames & masonry shear walls are required to provide lateral stability for the frame and resistance to wind and seismic forces. Contractor shall provide all temporary bracing required to maintain stability of structural system.
- 5100.13.**
Anchor rod holes in base plates shall be sized in accordance with AISC "Detailing For Steel Construction".
- 5100.14.**
All exposed edges of plates, beams, etc., shall be shop ground smooth and uniform.
- 5100.15.**
All column base plates supported on footings & pedestals shall bear on non-shrink non-metallic grout.
- 5100.16.**
1/2"Ø sleeve anchors shall be 1/2"Ø x 4" long Dynabolt Sleeve Anchor by ITW Redhead. Dynabolt Sleeve Anchor shall be carbon steel with zinc plating & have a 5" embedment depth.
- 5100.17.**
1/2"Ø expansion bolts shall be 1/2"Ø x 5 1/2" long Trubolt Wedge Anchor by ITW Redhead. Trubolt Wedge Anchor shall be carbon steel with zinc plating & have a 3 1/2" embedment depth.
- 5100.18.**
3/4"Ø expansion bolts shall be 3/4"Ø x 6" long Trubolt Wedge Anchor by ITW Redhead. Trubolt Wedge Anchor shall be carbon steel with zinc plating & have a 4" embedment depth.

GENERAL CONTRACTOR SHALL INCLUDE 2 TONS OF MISCELLANEOUS STEEL BEAMS, CHANNELS AND ANGLES IN ADDITION TO THE FRAMING SHOWN ON THE PLANS & DETAILS. GENERAL CONTRACTOR SHALL INCLUDE ERECTION COST FOR THIS ADDITIONAL FRAMING WITH THE BASE BID. THE USE OF MISCELLANEOUS STEEL IS TO BE RECORDED BY THE GENERAL CONTRACTOR AND ANY UNUSED AMOUNT IS TO BE CREDITED TO THE OWNER.

Steel Joist General Notes

- 5200.1.**
The design, fabrication and erection of steel joists shall conform to the Standard Specifications for Open Web Steel Joists & Joist Girders of the Steel Joist Institute.
- 5200.2.**
All hangers supported from open web joists shall be located at bottom chord panel points and shall be connected without field welding or drilling holes in the bottom chord.
- 5200.3.**
Joists shall have bridging per SJI Specifications. Roof joists shall have additional bridging, as required to resist stress reversal for the net uplift load indicated.
- 5200.4.**
Roof joists shall be designed for a net wind uplift of 14 psf. One-third stress increase of joists for wind uplift will not be allowed.
- 5200.5.**
"K" Joists framing into columns shall be field bolted with (2) 1/2"Ø A307 bolts and have a 1/4" x 6" bottom chord stabilizer plate to provide lateral stability during construction. Do not weld bottom chord to stabilizer plate.
- 5200.6.**
"LH" Joists framing into columns shall be field bolted with (2) 3/4"Ø A325 bolts and have a 1/4" x 6" x 6" bottom chord stabilizer plate to provide lateral stability during construction. Do not weld bottom chord to stabilizer plate. All "LH" joist seats shall be welded to structure with minimum 1/4" x 3" long fillet weld each side of seat.
- 5200.7.**
Joist mfr. shall design joists for mechanical & equipment loads noted on the Framing plans in addition to the uniform dead and live loads. Coordinate equipment size & locations with mechanical contractor.
- 5200.8.**
Joist mfr. shall design all roof joists for the snow drift loadings shown in the Design Loads.
- 5200.9.**
Attach joist top & bottom chord bridging to masonry walls with L3 x 3 x 3/16 x joist depth and (3) 1/2"Ø Dynabolt Sleeve Anchors, minimum. At "LH" joists, use L3 x 3 x 1/4 x joist depth and 1/2"Ø Dynabolt Sleeve Anchors @ 18" o.c. (minimum (3) bolts per angle).

Steel Deck General Notes

- 5300.1.**
Typical roof deck shall be 1 1/2" deep, 22 gauge, painted wide rib type and shall have nested side laps (Vulcraft 1.5B22 or approved equal). See Roof Framing Plan for limits of roof deck.
- 5300.2.**
Roof deck at Press Box shall be 1 1/2" deep, 19 gauge, painted wide rib type and shall have nested side laps (Vulcraft 1.5B19 or approved equal). See Roof Framing Plan for limits of roof deck.
- 5300.3.**
Roof deck shall be welded to the steel framing per the Roof Deck Fastening Pattern Detail 2/51.3.
- 5300.4.**
Roof deck fastening patterns at typical roof deck shall be capable of withstanding a net wind uplift of 47 psf at roof corners, 32 psf at roof perimeters and 20 psf at remainder of the roof. End zone width shall be minimum 11ft. At Press Box, roof deck fastening patterns shall be capable of withstanding a net wind uplift of 62 psf at roof corners, 48 psf at roof perimeters and 27 psf at remainder of the roof. End zone width shall be minimum 13 ft.
- 5300.5.**
Floor deck at elevated floor slabs shall be 1 1/2" deep 22 gauge, galvanized steel composite deck (Vulcraft 1.5VL22 or approved equal).
- 5300.6.**
Floor deck at elevated floor slabs shall be welded to the steel framing per the Floor Deck Fastening Pattern Detail 1/51.3.
- 5300.7.**
All deck shall be fastened per Steel Deck Institute (SDI) requirements.
- 5300.8.**
Deck specified has been determined on basis of 3 span condition; deck supplier shall use heavier gauge if required for one & two span conditions.

Light Gauge Steel General Notes

- 5400.1.**
All structural studs and headers shall be of the type, size, gauge and spacing as shown on the drawings.
- 5400.2.**
All framing members shall be formed from steel, corresponding to the requirements of ASTM A653.
- 5400.3.**
Steel for framing members shall have a minimum yield strength of 33 ksi (u.n.o.).
- 5400.4.**
Exterior studs and headers shall have a 1 5/8" flange with 1/2" return lip (type C5J stud).
- 5400.5.**
Exterior studs and runner track shall be 19 ga. thickness unless noted otherwise on the drawings. See Architectural Drawings for summary of stud sizes; runner track gauge to match stud gauge, U.N.O.
- 5400.6.**
Slide clips at roof perimeter shall be Verticlips "SLB" with (2) vertical slots by the Steel Network, Inc. or approved equal.
- 5400.7.**
Powder actuated fasteners for attachment of bottom runner track shall have a 0.177" minimum shank diameter and a 1 1/2" minium concrete embedment. Space fasteners at 16" o.c. (u.n.o.).
- 5400.8.**
At interior stud walls extending to the underside of Floor 4 roof structures, provide a deflection track at the top of the wall.
- 5400.9.**
Provide bracing to structure for all stud walls and furr-downs.
- 5400.10.**
All load-bearing stud walls shall have horizontal blocking @ 4'-0" O.C. vertically for full height of wall.

Permanent Grandstand General Notes

- 19126.01.**
The design, fabrication and erection of the permanent grandstand shall conform to the requirements of AISC Specifications for the Design, Fabrication and Erection of Structural Steel for Buildings.
- 19126.02.**
All structural steel shall conform to ASTM A572, grade 50 or ASTM A36. Designs using A36 steel modified based on fy greater than 36 ksi will not be allowed.
- 19126.03.**
The general contractor shall purchase and install anchor bolts for permanent grandstand based on the Anchor Bolt Setting Plan provided by the grandstand manufacturer. See the Anchor Bolt Schedule on sheet 51.4 for bolt lengths required based on the bolt diameter called for by the grandstand manufacturer.
- 19126.04.**
All column bases shall be designed as pinned ends that transmit no moment to the foundations.
- 19126.05.**
Bottom of base plate for all grandstand columns shall be at top of the concrete column as shown on the drawings.
- 19126.06.**
Grandstand manufacturer shall certify that the building has been designed for the loads shown on this drawing and the loads defined in the specifications. Certification must state that the building has been designed for all of the seismic criteria noted. The grandstand manufacturer shall prepare complete fabrication and erection drawings, fully engineered and sealed by a registered structural engineer in the state of Missouri.

Pre-Engineered Building General Notes

- 19100.1.**
The design fabrication and erection of the pre-engineered buildings shall conform to the requirements of AISC Specifications for the Design, Fabrication and Erection of Structural Steel for Buildings and the Metal Building Manufacturer Design Practices Manual.
- 19100.2.**
Structural steel for primary framing shall conform to ASTM A572, grade 50 or ASTM A36.
- 19100.3.**
Purlins & girts shall be cold-formed structural steel sections with stiffened flanges and have a minimum 55,000 psi yield strength.
- 19100.4.**
Structural system for the Area "A" building shall be a gabled rigid frame with interior columns as shown. Bay spacing, sloped roof, and eave heights shall be as shown on the architectural drawings.
- 19100.5.**
Lateral stability along the sidewalls shall be vertical rod or cable x-bracing at the locations shown on the drawings and designed for no future expansion.
- 19100.6.**
All columns shall be designed as pinned ends that transmit no moment to the foundations.
- 19100.7.**
Bottom of base plate elevation for pre-engineered building perimeter columns shall be at elevation 100'-0". Bottom of base plates for interior columns shall be 6" below Finished Floor elevation.
- 19100.8.**
Provide rod or cable x-bracing in roof as required to resist wind and seismic loads.
- 19100.9.**
Purlins shall be designed for a maximum live load deflection of L/180, where "L" is the purlin span in inches.
- 19100.10.**
Girts shall be designed for a maximum wind load deflection of L/240, where "L" is the girt span in inches.
- 19100.11.**
Limit lateral deflection of all columns in any direction to H/240, where "H" is the column height in inches.
- 19100.12.**
All rigid frame beams shall be designed for a maximum live load deflection of L/180, where "L" is the beam span in inches.
- 19100.13.**
The General Contractor shall purchase and install pre-engineered building ANCHOR RODs based on the ANCHOR ROD Setting Plan provided by the Metal Building Manufacturer. See the ANCHOR ROD Schedule on 51.1 for bolt lengths required based on the bolt diameter called for by the pre-engineered building manufacturer.
- 19100.14.**
Metal Building Manufacturer shall certify that the building has been designed for the loads shown on this drawing. Certification must state that the building has been designed for all roof loads, wind & seismic criteria noted in the pre-engineered building design loads below. The metal building manufacturer is to prepare complete fabrication & erection drawings, fully engineered & sealed by a registered Structural Engineer in the state of Missouri.
- 19100.15.**
Metal Building Manufacturer shall design purlins & rigid frames for the mechanical equipment in addition to the uniform dead & live loads. Coordinate equipment weight, size, and locations with the Mechanical Contractor & equipment supplier.
- 19100.16.**
Metal Building Manufacturer shall provide miscellaneous steel as required to support the mechanical equipment.

Wood Framing General Notes

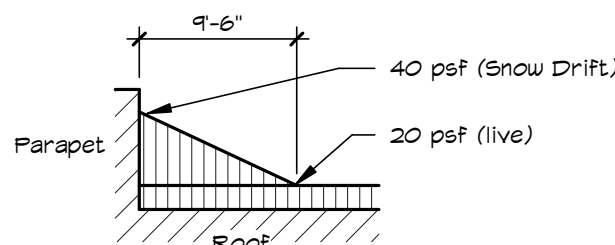
- 6100.1.**
All framing lumber shall be rated by the Southern Pine Inspection Bureau (SPIB), or the Western Wood Products Association. Lumber grades shall be as follows, with a moisture content between 7% and 14%:
- Southern Pine No. 2
- 6100.2.**
All nailing is to be in accordance with the 2006 International Building Code Fastening Schedule, Table 2304.5, unless noted otherwise.
- 6100.3.**
All wood connectors are to be installed according to the manufacturer's specifications and recommendations with the maximum number and size of nails.
- 6100.4.**
All exterior walls shall be sheathed with 15/32" (1/2" nominal) APA rated Oriented Strand Board (OSB) along outside face of studs for full height of wall. Attach OSB to stud walls with #10 screws @ 6" o.c. along panel edges and 12" o.c. at intermediate studs.
- 6100.5.**
All sheathing shall conform to American Plywood Association (APA) design specifications, latest edition. Sheathing shall be continuous over three adjacent spans minimum.
- 6100.6.**
Roof sheathing shall be 14/32" (5/8" nominal) APA rated plywood sheathing, Exposure I (40/20). All roof sheathing shall be fastened to supporting members with 10d nails @ 6" o.c. at panel edges and 12" o.c. at intermediate supports.
- 6100.7.**
Unless other wise notted, all connectors shall be by Simpson Strong-Tie - **NO EXCEPTIONS**

Wood Truss General Notes

- 6200.1.**
The wood and fabrication criteria of all wood trusses are to meet the "National Design Specifications for Stress Grade Lumber and its Fastenings" by the National Forest Products Association, "Timber Construction Standards" by American Institute of Timber Construction, and "Design Specifications for Light Metal Plate Connected Wood Trusses by the Truss Plate Institute.
- 6200.2.**
All connector plates are to be a minimum of 20 gauge. All connector plates are to meet the requirements of ASTM A446 grade A and are to be hot dipped galvanized according to ASTM A529 with 560 coating.
- 6200.3.**
All lumber used for the truss members is to have a moisture content between 7% and 14% at the time of fabrication.
- 6200.4.**
All wood trusses are to be installed and temporarily braced in accordance with "Bracing Wood Trusses: Commentary and Recommendations" by the Truss Plate Institute.
- 6200.5.**
During erection of trusses, care shall be exercised to keep horizontal bending of truss to a minimum. Proper erection bracing shall be installed to hold trusses true and plumb and in a safe condition until the roof decking and permanent truss bracing are installed.
- 6200.6.**
The truss manufacturer is to prepare complete fabrication and erection drawings, fully engineered and sealed by a registered structural engineer in the state of Missouri.
- 6200.7.**
Truss manufacturer is responsible for designing & supplying all wood truss to wood truss connections.
- 6200.8.**
Truss manufacturer shall design and provide permanent bridging and bracing for trusses.
- 6200.9.**
Truss manufacturer shall design trusses for mechanical equipment and snow drift loads in addition to the uniform dead & live loads. Coordinate mechanical equipment size & location with mechanical contractor.
- 6200.10.**
Wood trusses shall be designed for a net wind uplift of 14 psf.
- 6200.11.**
Structural component manufacturer shall have its manufacturing operations inspected at least four (4) times per year by an independent third party inspection agency. In-plant inspections shall be performed in accordance with ANSI/TFP-1 Chapter 3 and the Structural Building Components Association's Score Program. Component manufacturer shall be "Score Elite" certified by the Structural Building Components Association.

Design Loads

1. Roof Dead Load: 20 psf
2. Roof Live Load: 20 psf
3. Press Box Elevated Floor Dead Load (4" slab): 60 psf
4. Press Box Elevated Floor Live Load: 81 psf
5. Mezzanine Floor Dead Load: 20 psf
6. Mezzanine Floor Live Load: 80 psf
7. Snow Load:
- Ground Snow Load: 20 psf
- Flat-roof Snow Load (PF) = 22 psf
- Slope Factor (Cs) = 1.0
- Snow Exposure Factor (Ce) = 1.0
- Snow Load Importance Factor (I) = 1.1
- Thermal Factor (Ct) = 1.0
8. Snow Drift:

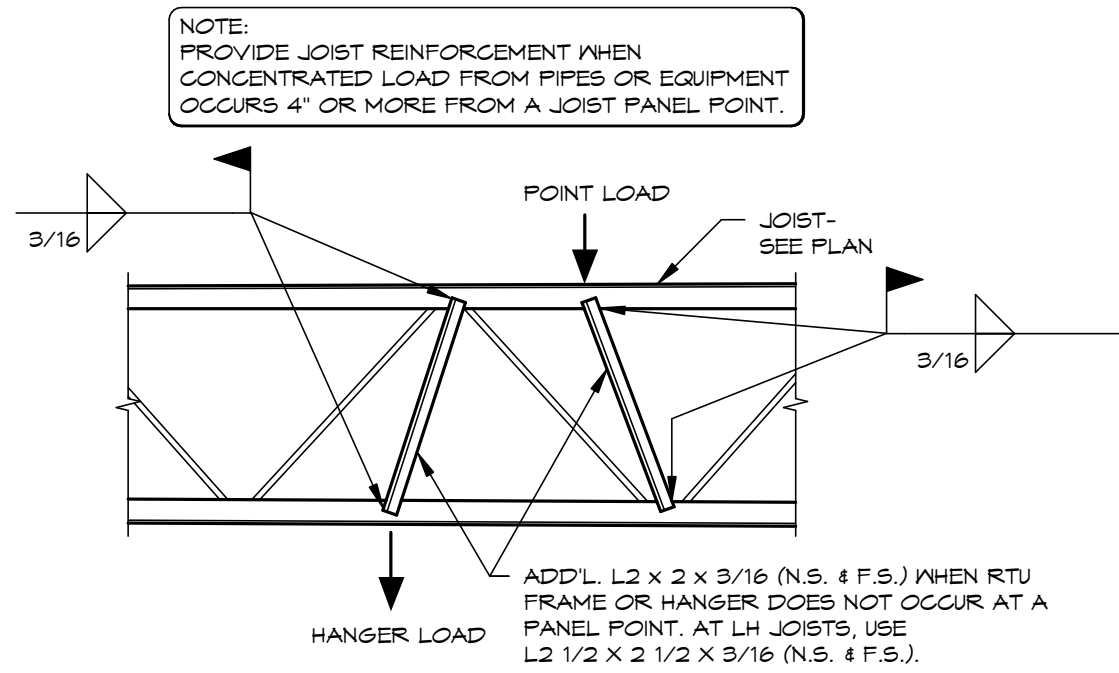


Drift @ Parapet

9. Wind Load:
- Basic Wind Speed 90 mph
- Risk Category III
- Wind Exposure C
- Internal Pressure Coefficient, SCPI = ±0.18
- Components & Cladding Design Wind Pressure (min. 20 p.s.f.) = ±30.0 psf
10. Seismic:
- Risk Category III
- Seismic Importance Factor (Ie) = 1.25
- Ss = 0.150
- Si = 0.077
- Sps = 0.164
- Sps1 = 0.123
- Site Class D
- Seismic Design Category B
- Basic Structural System - Bearing Wall System/ Building Fram System
- Seismic Resisting System - Intermediate Reinforced Masonry Shear Walls/ Steel System not specifically designed for seismic resistance
- Response Modification Coefficient (R) = 3.5/ 3.0
- Deflection Amplification Factor (Cd) = 2.25/ 3.0
- Seismic Response Coefficient (Cs) = 0.073/ 0.074
- Analysis Procedure - Equivalent Lateral Force Procedure
11. Building Code:
- 2006 International Building Code

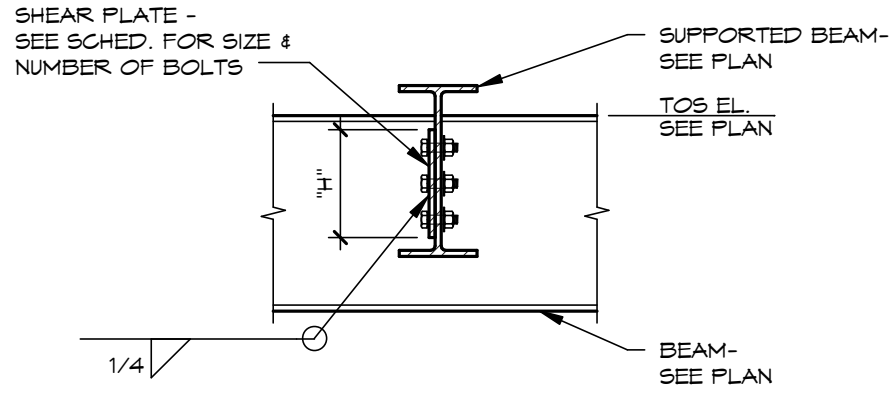
Pre-Engineered Building Design Loads

1. Roof Dead Load: per MBM
2. Roof Collateral Load: 5 psf
3. Roof Live Load: 20 psf
(L.L. reduction shall not be allowed)
4. Snow Load:
- Ground Snow Load: 20 psf
- Flat-roof Snow Load (PF) = 22 psf
- Slope Factor (Cs) = 1.0
- Snow Exposure Factor (Ce) = 1.0
- Snow Load Importance Factor (I) = 1.1
- Thermal Factor (Ct) = 1.0
5. Wind Load:
- Basic Wind Speed 90 mph
- Risk Category III
- Wind Exposure C
6. Seismic:
- Risk Category III
- Seismic Importance Factor (Ie) = 1.25
- Ss = 0.150
- Si = 0.077
- Sps = 0.164
- Sps1 = 0.123
- Site Class D
- Seismic Design Category B
- Basic Structural System - per MBM
- Seismic Resisting System - per MBM
- Response Modification Coefficient (R) = per MBM
- Deflection Amplification Factor (Cd) = per MBM
- Analysis Procedure - per MBM
7. Load Combinations: per MBM
8. Building Code:
- 2006 International Building Code

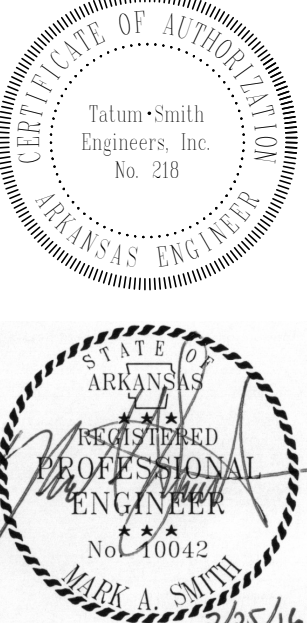


1 TYP. JOIST REINFORCEMENT AT CONCENTRATED LOADS
NOT TO SCALE

BEAM TO BEAM CONNECTION SCHEDULE			
SUPPORTED BEAM SIZE	SHEAR PLATE SIZE (T x H)	NUMBER OF BOLTS	REMARKS
M8, M10, C8, C10	3/8" x 6"	(2) 3/4"Ø A325	-----
M12, M14, C12, MC12	3/8" x 9"	(3) 3/4"Ø A325	-----
C15, M16, M18	3/8" x 1'-0"	(4) 3/4"Ø A325	-----
M21, M24, M27	3/8" x 1'-3"	(5) 3/4"Ø A325	-----
M30, M33	3/8" x 1'-6"	(6) 3/4"Ø A325	-----



2 TYP. BEAM TO BEAM CONNECTION DETAIL
NOT TO SCALE



Hight Jackson
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CARTHAGE R-9 SCHOOL DISTRICT
CARTHAGE STADIUM COMPLEX
FIR RD & RIVER ST, CARTHAGE, MO

DRAWN BY:
EEC
CHECK BY:
MAS
ISSUE DATE:
02/29/2016

PROJECT NO.:
1365

REVISION DATES
DATE DESCRIPTION

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Tatum Smith
Engineers, Inc.
Structural Engineering
479.621.6128
Rogers, Arkansas



BEAM TO COLUMN FLANGE

1" WELD RETURN TOP & BOTTOM OF ANGLE
 1/4"
 I
 5/16" THICK ANGLE (N.S. & F.S.) - SEE SCHED. FOR NUMBER OF BOLTS PER ANGLE
 COLUMN - SEE COL. SCHED.
 A325 BOLTS
 BEAM - SEE PLAN

BEAM TO COLUMN WEB

1" WELD RETURN TOP & BOTTOM OF ANGLE
 COLUMN - SEE COL. SCHED.
 BEAM - SEE PLAN
 5/16" THICK ANGLE - SEE SCHED. FOR HEIGHT & NUMBER OF BOLTS
 I
 1/4" TYP.
 AT INTERIOR COLS., BM. ON ONE SIDE OF WEB SHALL HAVE ONE ADD'L ROW OF BOLTS FOR ERECTION PURPOSES; ADD 3" TO CLIP ANGLE LENGTH



8 TYP. MOMENT CONNECTION DETAIL
NOT TO SCALE



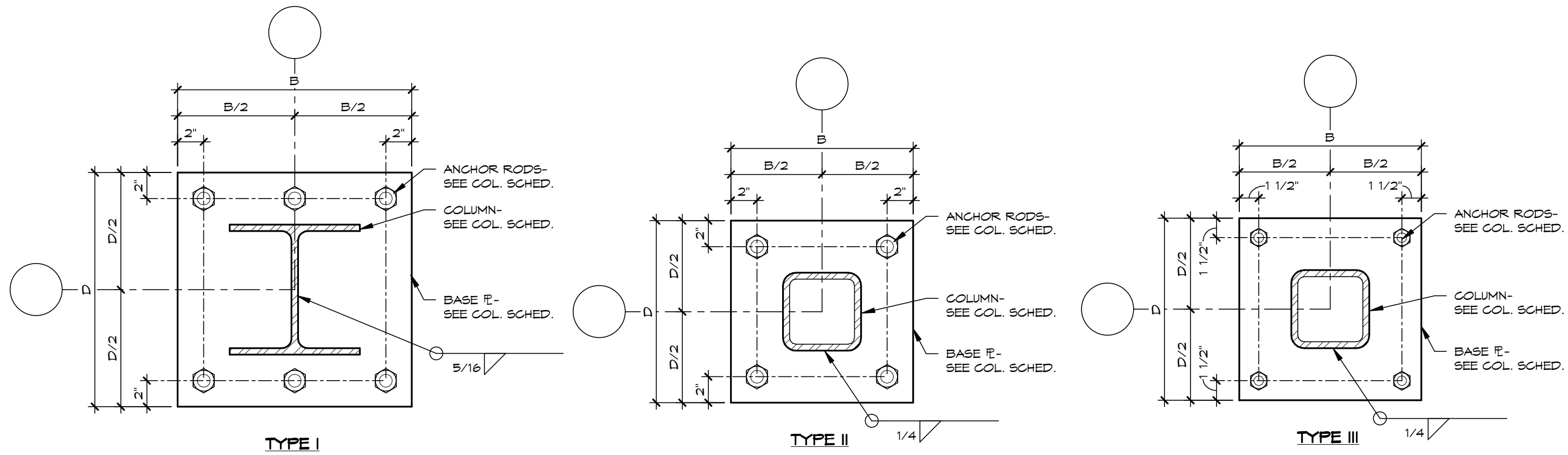
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EEC
CAD Technician

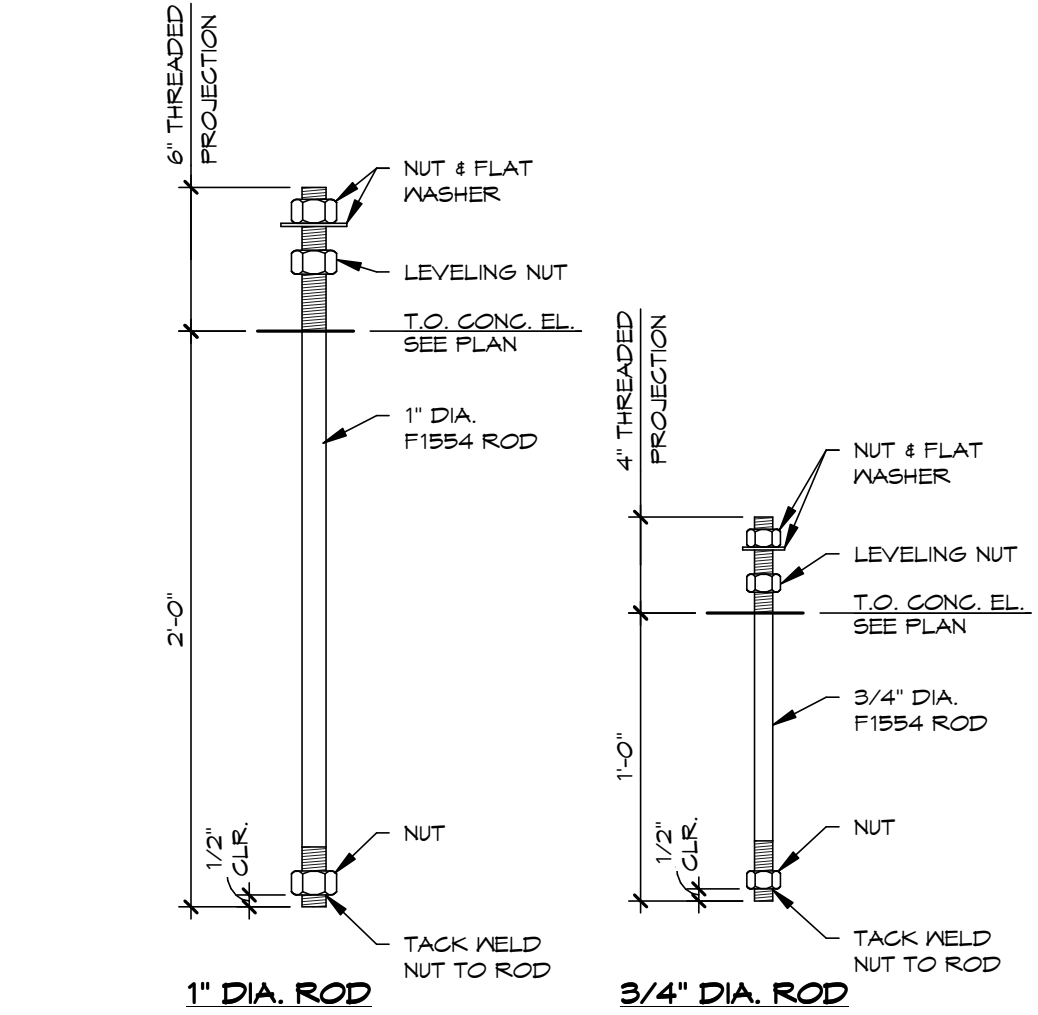
MAS
TAT
Engineer of Record:
Project Designer

STEEL COLUMN SCHEDULE						
MARK	COLUMN SIZE	BASE PLATE SIZE (L x B x D)	BASE PLATE TYPE (SEE DTL. 1/51.4)	ANCHOR RODS (SEE DTL. 2/51.4)	BOTT. BASE PL. EL.	CAP PL. THICKNESS
C1	H558 x 8 x 3/8	1' x 16' x 16'	TYPE II	(4) 1"Ø	98'-10"	3/4"
C2	W10 x 49	1' x 16' x 16'	TYPE I	(6) 1"Ø	98'-10"	3/4"
C3	H554 x 4 x 1/4	3/4' x 10' x 10'	TYPE III	(4) 3/4"Ø	99'-5 1/2"	1/2"
C4	PER MBM	PER MBM	PER MBM	PER MBM	100'-0"	PER MBM
C5	PER MBM	PER MBM	PER MBM	PER MBM	99'-4"	PER MBM
GSC1	PER GRANDSTAND MFR.	PER MFR.	PER MFR.	PER MFR.	1103'-1 3/4"	SEE NOTE #1
GSC2	PER GRANDSTAND MFR.	PER MFR.	PER MFR.	PER MFR.	1100'-5"	SEE NOTE #1
GSC3	PER GRANDSTAND MFR.	PER MFR.	PER MFR.	PER MFR.	PER MFR.	-----

STEEL COLUMN SCHEDULE NOTES:
1. BOTTOM PL. EL. MAY VARY DUE TO GROUT PAD THICKNESS SPECIFIED BY GRANDSTAND MFR. (COORD. IV MFR.)



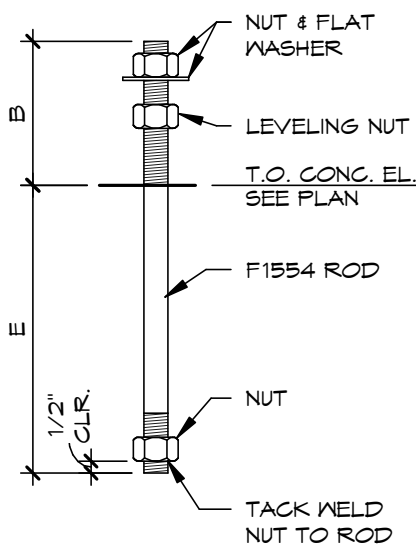
1 TYP. BASE PLATE DETAILS
NOT TO SCALE



2 TYP. ANCHOR ROD DETAILS
NOT TO SCALE

PERMANENT GRANDSTAND ANCHOR ROD SCHEDULE		
ROD DIAMETER	EMBEDMENT LENGTH PL	THREAD PROJ. PL
1 1/4"	2'-6"	5"
1"	2'-0"	4"
7/8"	1'-8"	3"
3/4"	1'-3"	3"
5/8"	1'-0"	3"

PERMANENT GRANDSTAND ANCHOR ROD SCHEDULE NOTES:
1. SEE PERMANENT GRANDSTAND ANCHOR ROD SETTING PLAN FOR SIZE & LOCATION OF ANCHOR RODS.



3 PERMANENT GRANDSTAND ANCHOR ROD DETAIL
1 1/2" = 1'-0"

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CARTHAGE R-9 SCHOOL DISTRICT
CARTHAGE STADIUM COMPLEX
FIR RD & RIVER ST, CARTHAGE, MO

DRAWN BY:
EEC
CHECK BY:
MAS
ISSUE DATE:
02/29/2016

PROJECT NO.:
1365

REVISION DATES
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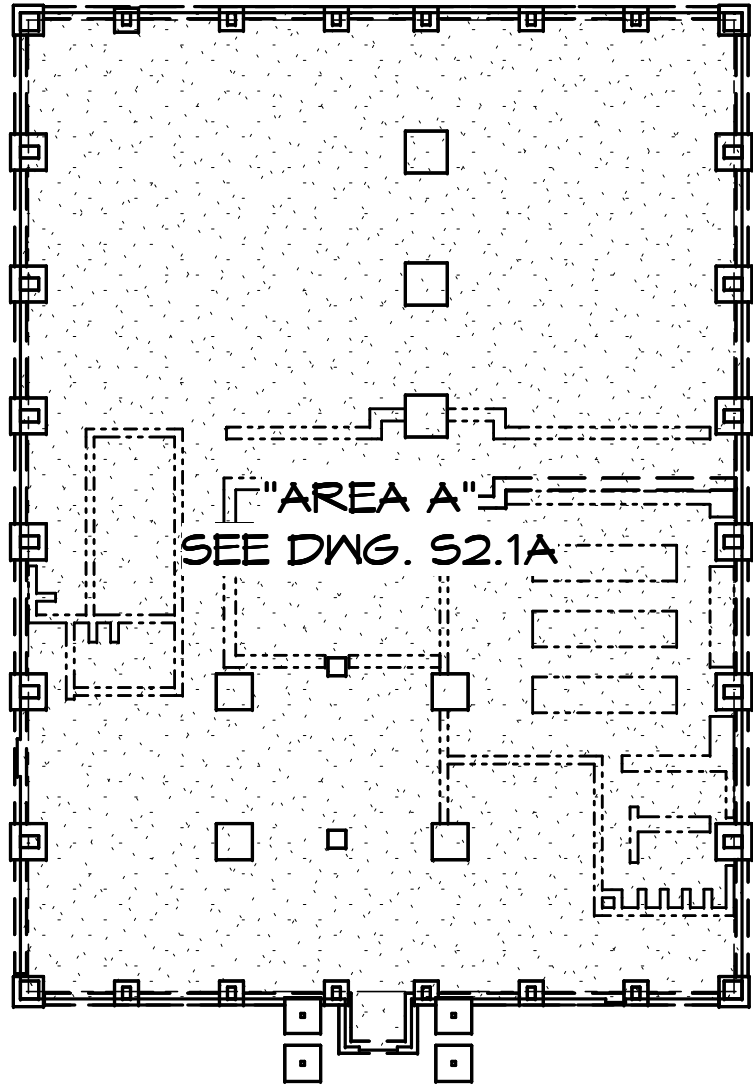
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PROFESSIONAL ENGINEER
No. 10042
WORK A. SMITH
2/25/16

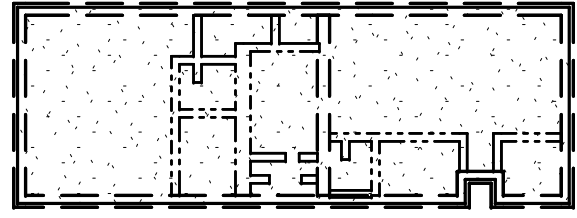
Arkansas
Tatum Smith
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No. 218

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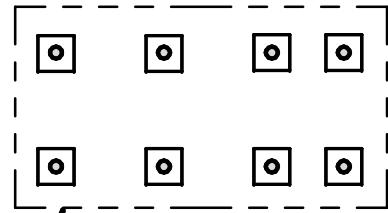
TS Project No.: 151210
CAD Technician: EEC
Engineer of Record: MAS
Project Designer: TAT



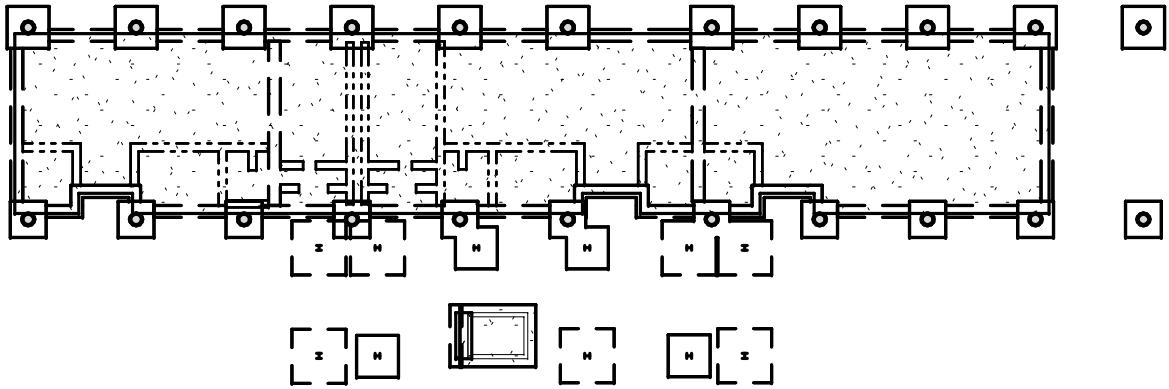
BAND BLEACHER
ALTERNATE
SEE DWGS. S2.2



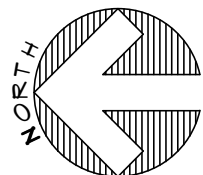
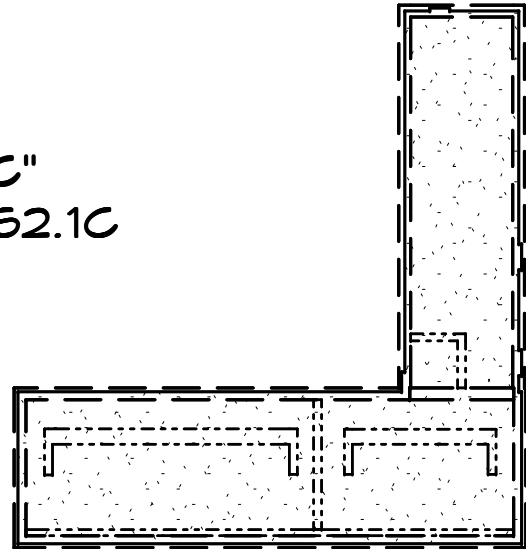
"AREA D"
SEE DWG. S2.1C



"AREA B"
SEE DWG. S2.1B



"AREA C"
SEE DWG. S2.1C



1

OVERALL FOUNDATION PLAN

1/32" = 1'-0"

PLAN NOTES:

1. THIS DRAWING IS FOR REFERENCE ONLY.
2. SEE REFERENCED AREAS FOR ENLARGED PLANS.

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CARTHAGE R-9 SCHOOL DISTRICT

CARTHAGE STADIUM COMPLEX

FIR RD & RIVER ST., CARTHAGE, MO

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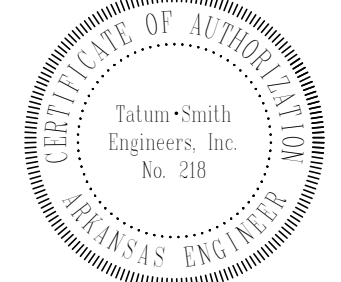
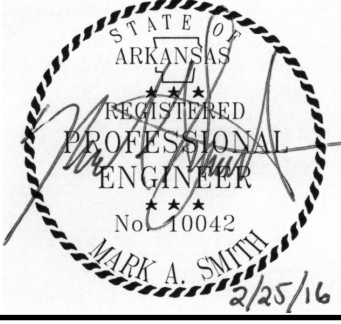
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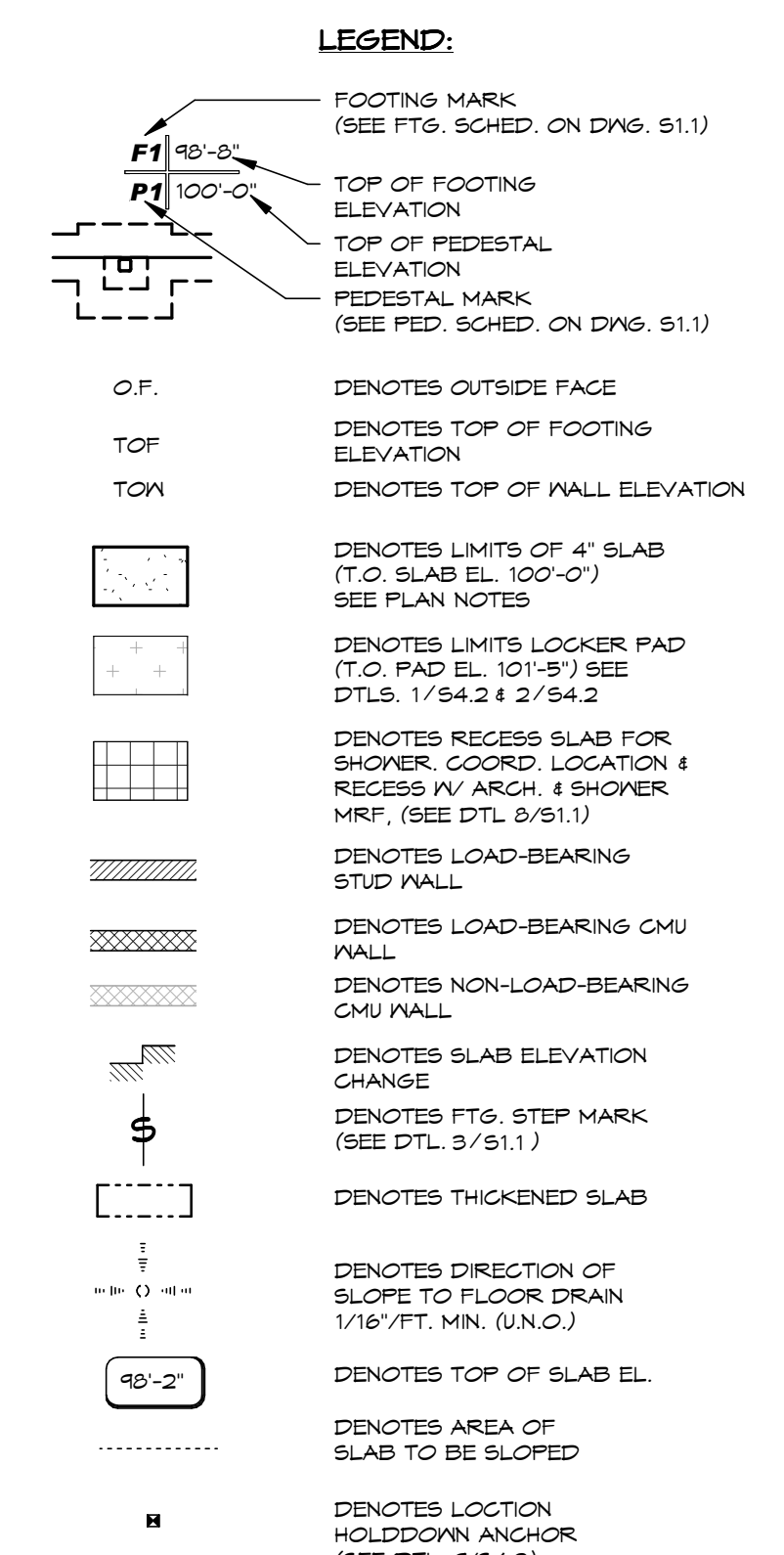
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S H E E T
S2.1

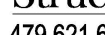
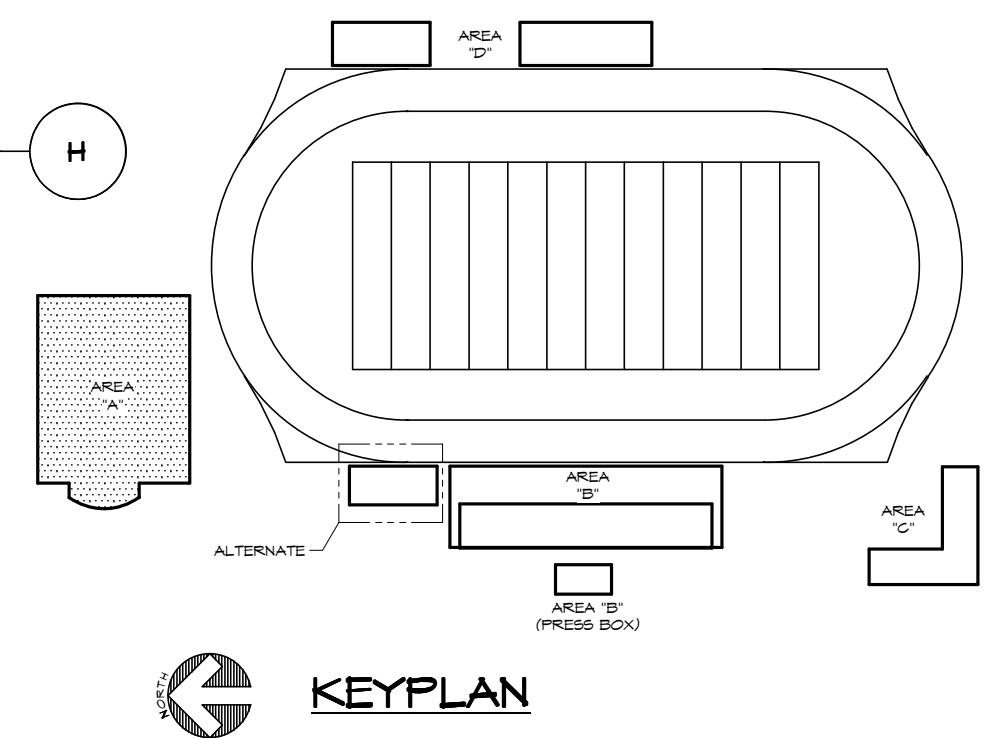
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- PLAN NOTES:**
- 4' 5.0'S. CONSTRUCTION: 4' CONCRETE SLAB REINFORCED W/ 6 X 8-1/4" X 1/4" W/F ON 10 MIL POLYETHYLENE FILM COVER 6' CRUSHED STONE TYPE IV FILL 12" MIN. C/C. CONCRETE TOP OF SLAB ALL ELEVATIONS BASED ON FINISH FLOOR EL. 100'-0". ACTUAL FIN. FLR. EL. 107.930" AS ESTABLISHED BY GRAV. SURV. CONSULTING, INC.
- SEE CENTER COLUMN FTGS. UNDER ANCHOR ROD PATTERN, (N.O.)
- CENTER COLUMN FTGS. UNDER COLUMN (N.O.).
- TOP OF FOOTING ELEVATION 98'-8", TYPICAL (N.O.).
- TOP OF FEEDERAL ELEVATION 100'-0", TYPICAL (N.O.).
- "C" DENOTES CORNER OR CONSTRUCTION JOINT. (SEE DTLs. 1/51.1 & 2/51.1).
- COORDINATE LOCATION & LIMITS OF VENEER WITH ARCH. DRAWING.
- COORDINATE DOOR LOCATIONS WITH ARCH. DWGS..
- SEE ARCH. DWGS. FOR LOCATIONS OF NON-LOAD-BEARING MASONRY WALLS BEARING ON THICKENED SLABS.
- COORDINATE LOCATIONS OF PLUMBING LINES W/ PLUMBER PRIOR TO POURING FOOTINGS & SLABS-ON-GRADE.
- SEE DWG. 51.1 FOR GENERAL NOTES, FOOTING SCHEDULE & TYP.
- SEE ARCH./PLUMBING DWGS. FOR EXACT LOCATIONS OF ALL FLOOR DRAINS, SLOPED & RECESSED SLABS.

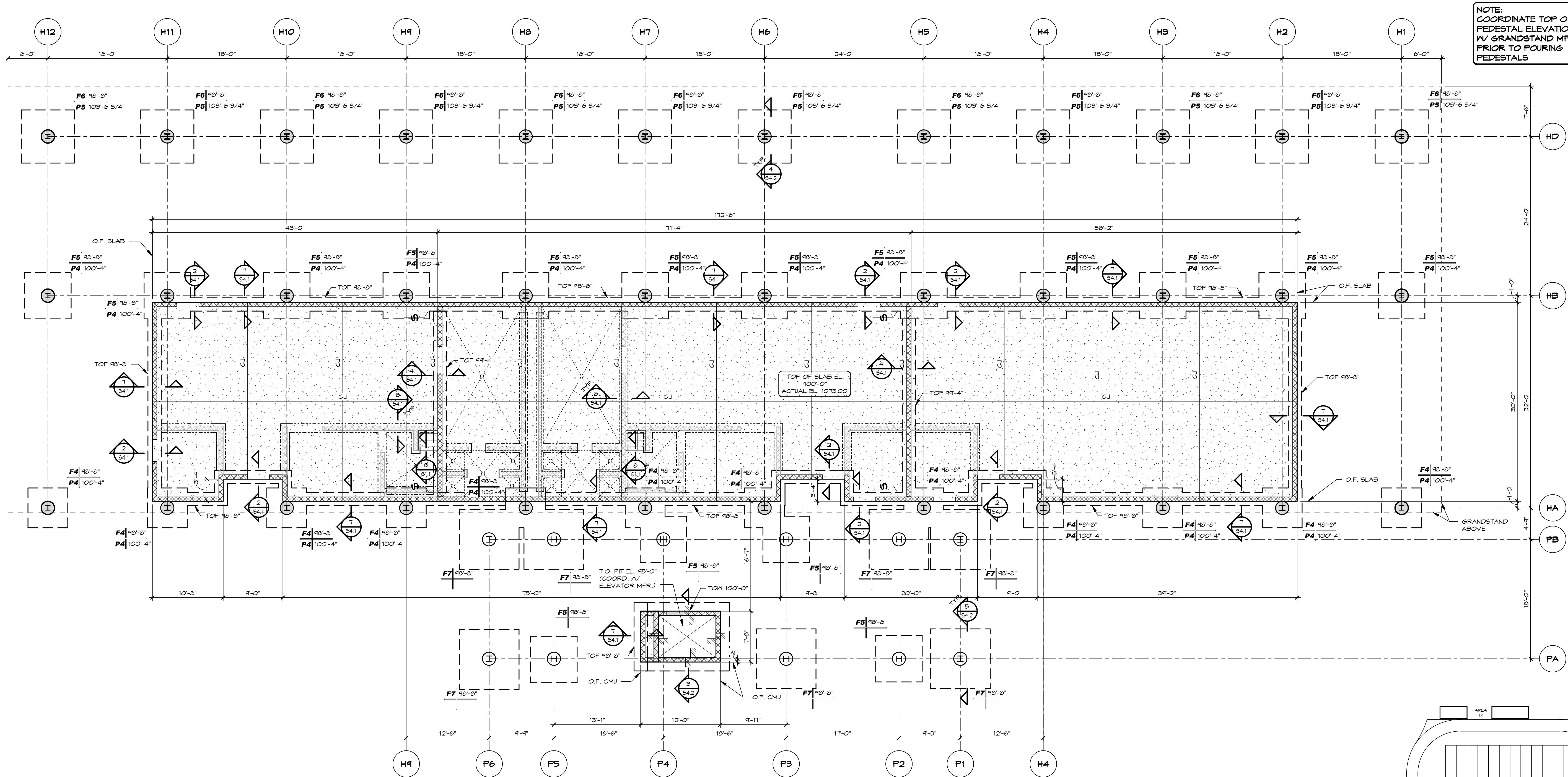


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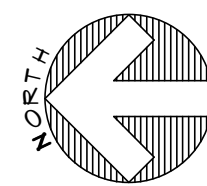
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Project Designer: TAT

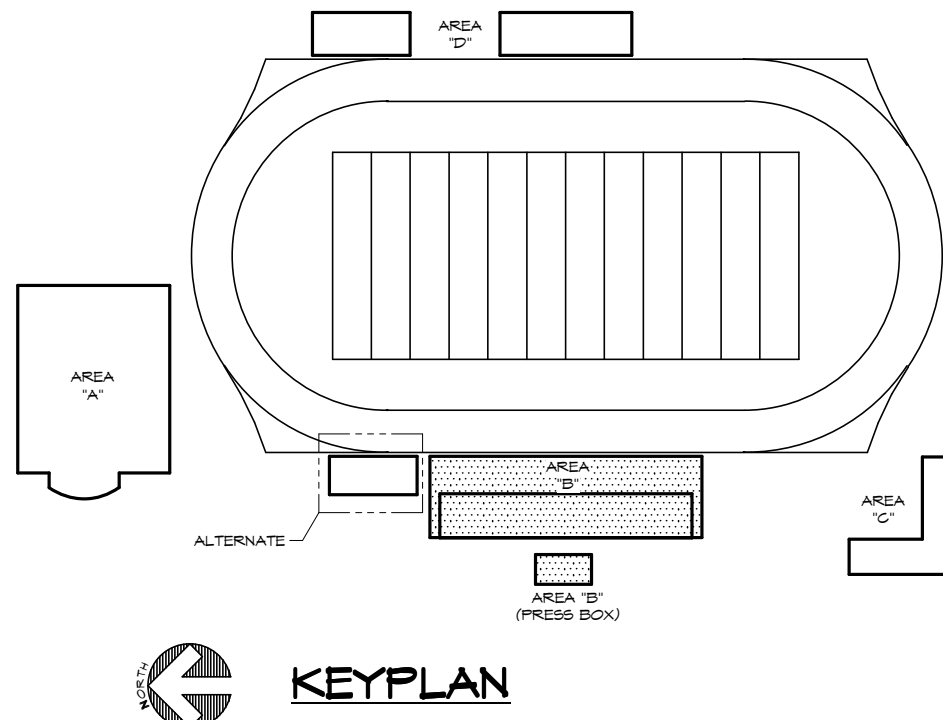


NOTE:
COORDINATE TOP OF
PEDESTAL ELEVATIONS
IV GRANDSTAND MFR.
PRIOR TO POURING
PEDESTALS



1 FOUNDATION PLAN - AREA "B"
1/8" = 1'-0"

NOTE:
SEE DWS, S2.1A FOR
LEGEND & PLAN NOTES



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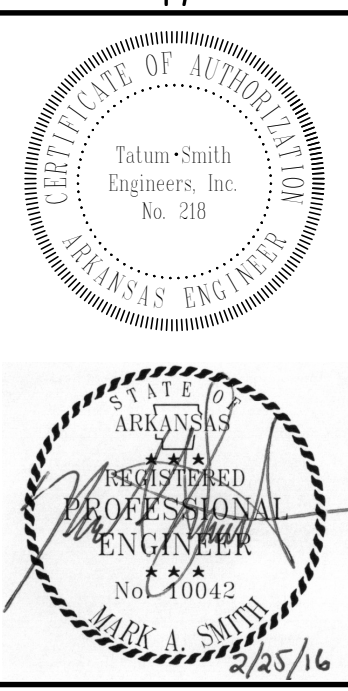
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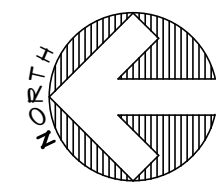
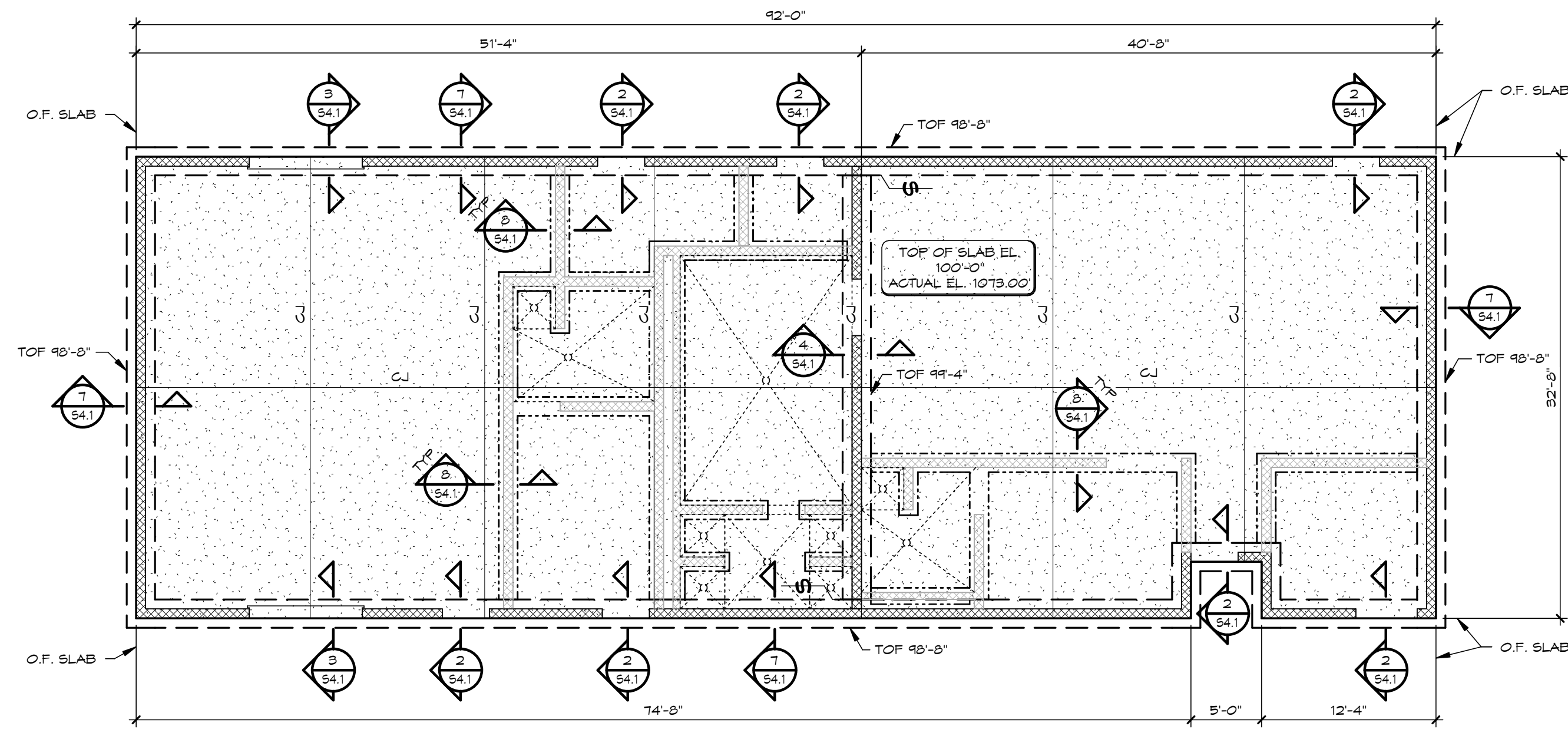
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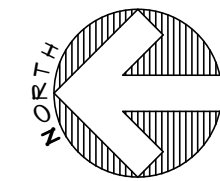
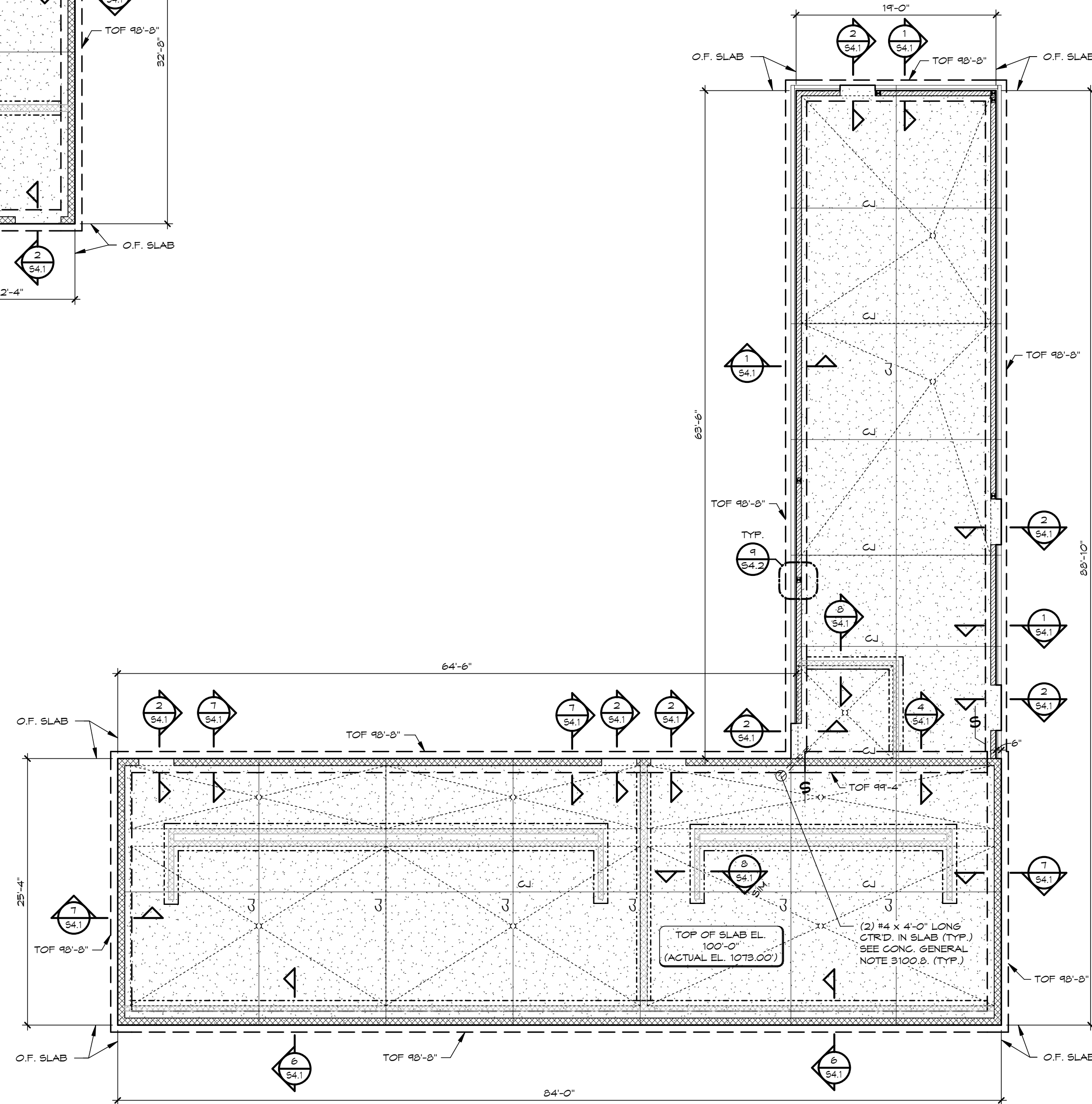
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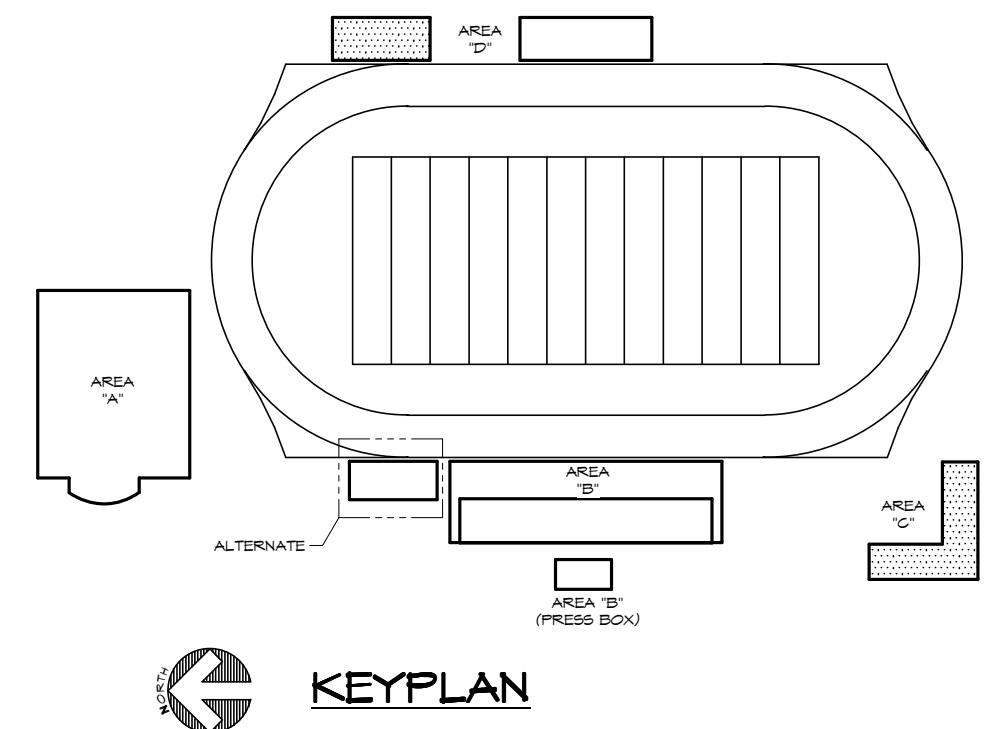
1 FOUNDATION PLAN - AREA "D"
1/8" = 1'-0"

NOTE:
SEE DWG. S2.1A FOR
LEGEND & PLAN NOTES



2 FOUNDATION PLAN - AREA "C"
1/8" = 1'-0"

NOTE:
SEE DWG. S2.1A FOR
LEGEND & PLAN NOTES



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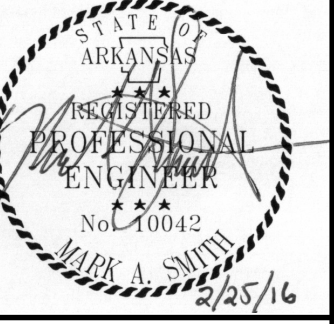
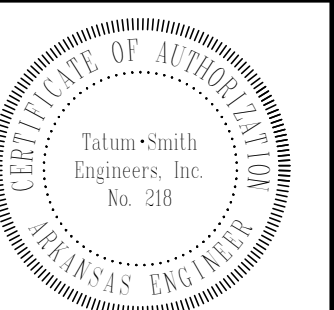
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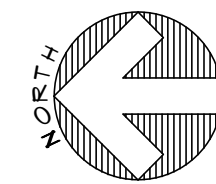
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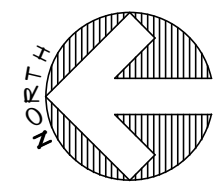
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NOTE:
SEE DWG. S2.1A FOR
LEGEND & PLAN NOTES



NOTE:
SEE DWG. 53.2 FOR
LEGEND & PLAN NOTES



LINTEL SCHEDULE					
LINTEL MARK	WALL LOCATIONS	TYPE & SIZE (L x H)	REINFORCEMENT OR ATTACHMENT	BRICK ANGLE OR PLATES	REMARKS
L1	8" CMU	CMU 8" x 8" CLOSED BOTT. BOND BM.	(1) #5 BOTT.	----	SEE NOTES #1, #2, #4, #5, #6 & #12
L2	8" CMU	CMU 8" x 8" CLOSED BOTT. BOND BM. 1/4" 8" x 8" OPEN BOTT. BOND BM. ON TOP 16" TOTAL DEPTH	(2) #5 TOP & BOTT.	----	SEE NOTES #1, #2, #3, #4, #5 & #6
L3	8" CMU	W16 x 31	----	CONT. E 3/8" x T"	SEE NOTES #1, #2, #7, #8, #9, #10, #11 & DTL. 1/33.0
L4	8" CMU	W16 x 40	----	CONT. E 3/8" x T"	SEE NOTES #1, #2, #7, #8, #9, #10, #11 & DTL. 2/33.0
L5	6" MTL STUDS	(2) 6" x 18 GA. "CSJ" STUDS 1/4" 18 GA. "TSB" TRACK (TOP & BOTT.)	(4) #10 SCREENS @ 6" O.C.	----	SEE NOTES #1, #2, #12, #13 & DTL. 3/33.0
L6	6" MTL STUDS	(3) 6" x 14 GA. "CSJ" STUDS 1/4" 18 GA. "TSB" TRACK (TOP & BOTT.)	(6) #10 SCREENS @ 6" O.C.	----	SEE NOTES #1, #2, #13 & DTL. 3/33.0
L7	6" MTL STUDS	(2) 6" x 18 GA. "CSJ" STUDS 1/4" 18 GA. "TSB" TRACK (TOP & BOTT.)	(4) #10 SCREENS @ 6" O.C.	L5 x 5 x 1/4	SEE NOTES #1, #2, #12, #13, #14 & DTL. 3/33.0
L8	6" MTL STUDS	(3) 6" x 14 GA. "CSJ" STUDS 1/4" 18 GA. "TSB" TRACK (TOP & BOTT.)	(6) #10 SCREENS @ 6" O.C.	BENT E 3/8" x 5" x 8" (LLV)	SEE NOTES #1, #2, #13, #14 & DTL. 3/33.0

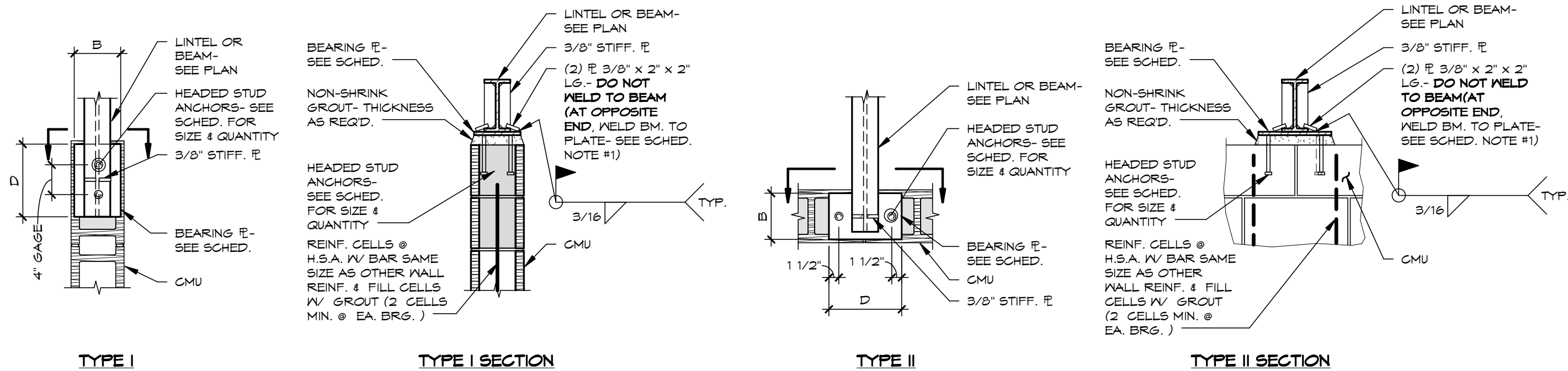
- LINTEL SCHEDULE NOTES:
- SEE ARCH. DWGS. FOR EXACT LOCATION OF ALL LINTELS.
 - COORDINATE ALL BOTTOM PLATE/BOND BEAM/CMU/ANGLE ELEVATIONS WITH ARCH. DWGS.
 - PROVIDE #3 STIRRUPS @ 8" O.C.
 - ALL CMU LINTELS SHALL HAVE 8" MIN. BEARING EACH SIDE OF OPENING.
 - FILL ALL CMU LINTELS WITH 2,000 PSI GROUT.
 - ALL 8" CMU LINTELS SHALL HAVE A MIN. (2) CELLS OF (1) #5 JAMB STEEL EACH SIDE OF OPENING, UNO ON THE DRAWING.
 - ALL STEEL LINTELS SHALL HAVE 8" MIN. BEARING EACH SIDE OF OPENING.
 - WELD TOP & BOTTOM PLATE TO FLANGE OF BEAM WITH 3/16" x 3" LG. FILLET WELD @ 12" O.C. ALONG EACH SIDE OF FLANGE.
 - ALL WIDE FLANGE STEEL LINTELS SHALL HAVE 1/2" DIA. x 6" LG. H.S.A. WELDED TO TOP FLANGE OR TOP PLATE. SEE LINTEL DETAILS FOR NUMBERS AND SPACING OF HEADED STUD ANCHORS.
 - AT EACH END OF STEEL LINTEL, STOP BOTTOM PLATE 1/2" CLEAR OF THE JAMB.
 - PROVIDE BEARING PLATE WITH (2) 1/2" DIA. x 6" LG. H.S.A. AT EACH END OF BEAM. SEE SIZE OF BEARING PLATE & ATTACHMENT TO EACH END OF BEAM PER DETAIL 6/53.0.
 - WHERE MECHANICAL DUCTS PASS THROUGH MASONRY WALLS, PROVIDE "L1" LINTEL AT 8" CMU WALLS AND "L5" AT 6" METAL STUD WALLS FOR OPENINGS NOT TO EXCEED 6'-0" WIDE. FOR OPENING WIDTHS LARGER THAN 6'-0" COORDINATE WITH ARCH./ENGINEER.
 - PROVIDE (2) KING STUDS & (2) JACK STUDS AT JAMBS (TYP. UNO.).
 - BRICK ANGLES SHALL HAVE 8" BEARING EA. SIDE OF OPENING.

MASONRY WALL REINFORCEMENT SCHEDULE					
MARK	WALL LOCATION	BOND BEAM REINF.	BOND BEAM LOCATIONS	VERT. REINF. & FDN. DOVELS	REMARKS
A	8" CMU	(1) #5 BOTTOM	TOP OF WALL, ROOF EL., FLOOR EL. & WHERE NOTED ON DWGS.	#4 @ 48" O.C. #4 DOVEL @ 48" O.C. CENTER IN WALL	SEE NOTES #2 & #6
B	8" CMU	(1) #5 BOTTOM	TOP OF WALL, ROOF EL., FLOOR EL. & WHERE NOTED ON DWGS.	#5 @ 48" O.C. #5 DOVEL @ 48" O.C. CENTER IN WALL	SEE NOTES #2 & #6
C	8" CMU	(1) #5 BOTTOM	TOP OF WALL, ROOF EL., FLOOR EL., 4'-0" O.C. VERTICAL FULL HEIGHT OF WALL	#5 @ 24" O.C. #5 DOVEL @ 24" O.C. CENTER IN WALL	SEE NOTES #2 & #6
D	8" CMU	(1) #5 BOTTOM	TOP OF WALL, ROOF EL., FLOOR EL. & WHERE NOTED ON DWGS.	#4 x 2'-6" LG DOVELS @ 48" O.C. CENTER IN WALL	SEE NOTES #1, #3 & #5
E	6" CMU	(1) #5 BOTTOM	TOP OF WALL, ROOF EL., FLOOR EL. & WHERE NOTED ON DWGS.	#3 x 1'-6" LG DOVEL @ 96" O.C. CENTER IN WALL	SEE NOTES #1, #4 & #5

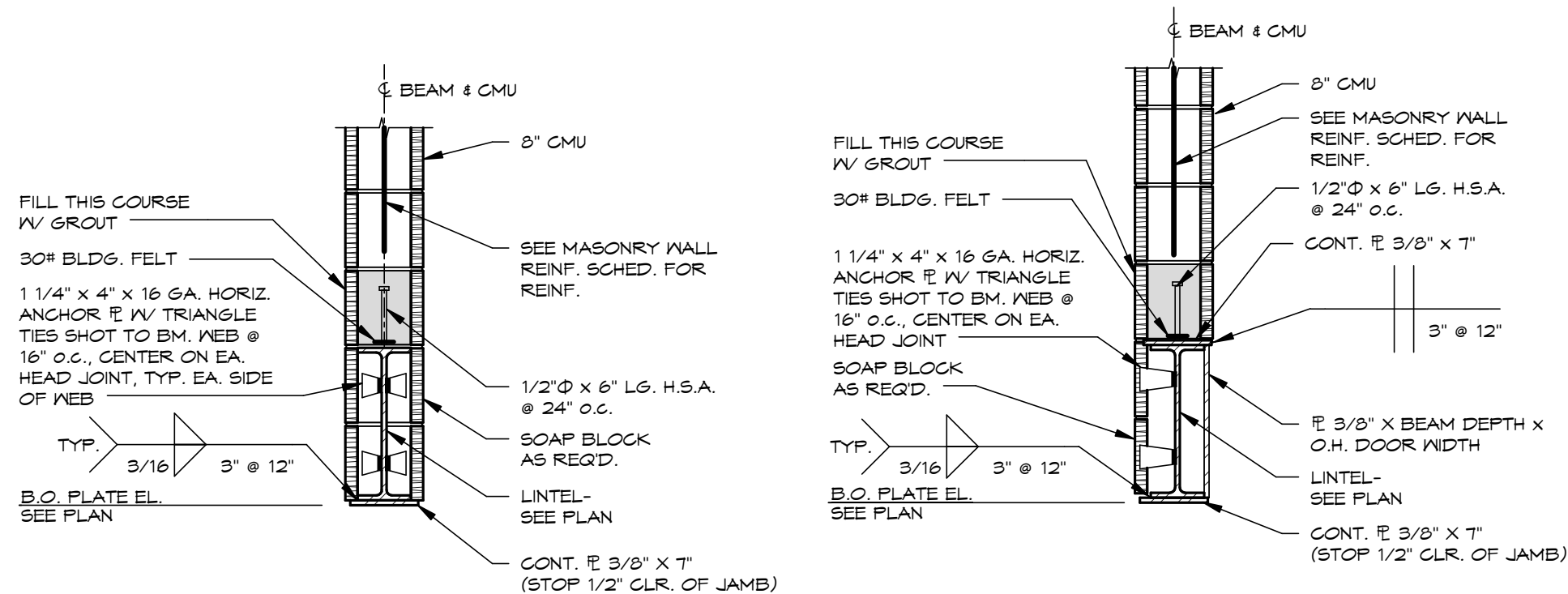
- MASONRY WALL REINFORCEMENT SCHEDULE NOTES:
- INTERIOR NON-LOAD-BEARING WALLS ARE DEFINED AS WALLS SUPPORTED ON AN 8" THICKENED SLAB.
 - DRILL 3" MIN. DEEP HOLES IN SLAB-ON-GRADE & ANCHOR DOVELS WITH EPOXY. MINIMUM OF TWO DOVELS FOR EACH WALL SEGMENT.
 - DRILL 6" MIN. DEEP HOLES IN SLAB-ON-GRADE & ANCHOR DOVELS WITH EPOXY. MINIMUM OF TWO DOVELS FOR EACH WALL SEGMENT.
 - DRILL 3" MIN. DEEP HOLES IN SLAB-ON-GRADE & ANCHOR DOVELS WITH EPOXY. MINIMUM OF TWO DOVELS FOR EACH WALL SEGMENT.
 - BRACE TOP OF INTERIOR NON-LOAD-BEARING WALLS WITH 45° ANGLE DIAGONAL L3 x 3 x 1/4 BRACE WELDED TO THE ROOF FRAMING ABOVE. ATTACH ANGLE BRACE TO TOP OF WALL WITH 3/4" DIA. x 3" LG. EXPANSION ANCHORS & SPACE BRACING AT 8'-0" O.C. MAXIMUM. THIS IS PERMANENT BRACING REQUIRED FOR SEISMIC LOADS (SEE DTLs. 4/33.0 & 5/33.0).
 - LAP DOVELS WITH FOUNDATION BARS.

LINTEL & BEAM BEARING PLATE SCHEDULE					
LINTEL/BEAM SIZE	LINTEL BRG. PLATE SIZE T x B x D	BEAM BRG. PLATE SIZE T x B x D	HEADED STUD ANCHORS	PLATE TYPE	REMARKS
ALL BEAM SIZES	3/8" x T" x 11"	3/4" x T" x 11"	(2) 1/2" DIA. x 6" H.S.A.	I	8" CMU WALL
ALL BEAM SIZES	3/8" x T" x 11"	3/4" x T" x 11"	(2) 1/2" DIA. x 6" H.S.A.	II	8" CMU WALL

- LINTEL & BEAM BEARING PLATE SCHEDULE NOTES:
- USE (2) E 3/8" x 2" x 2" AT ONE END OF BEAM & FIELD WELD BEAM BOT. FLANGE TO BEARING PLATE AT OPPOSITE END OF BEAM.

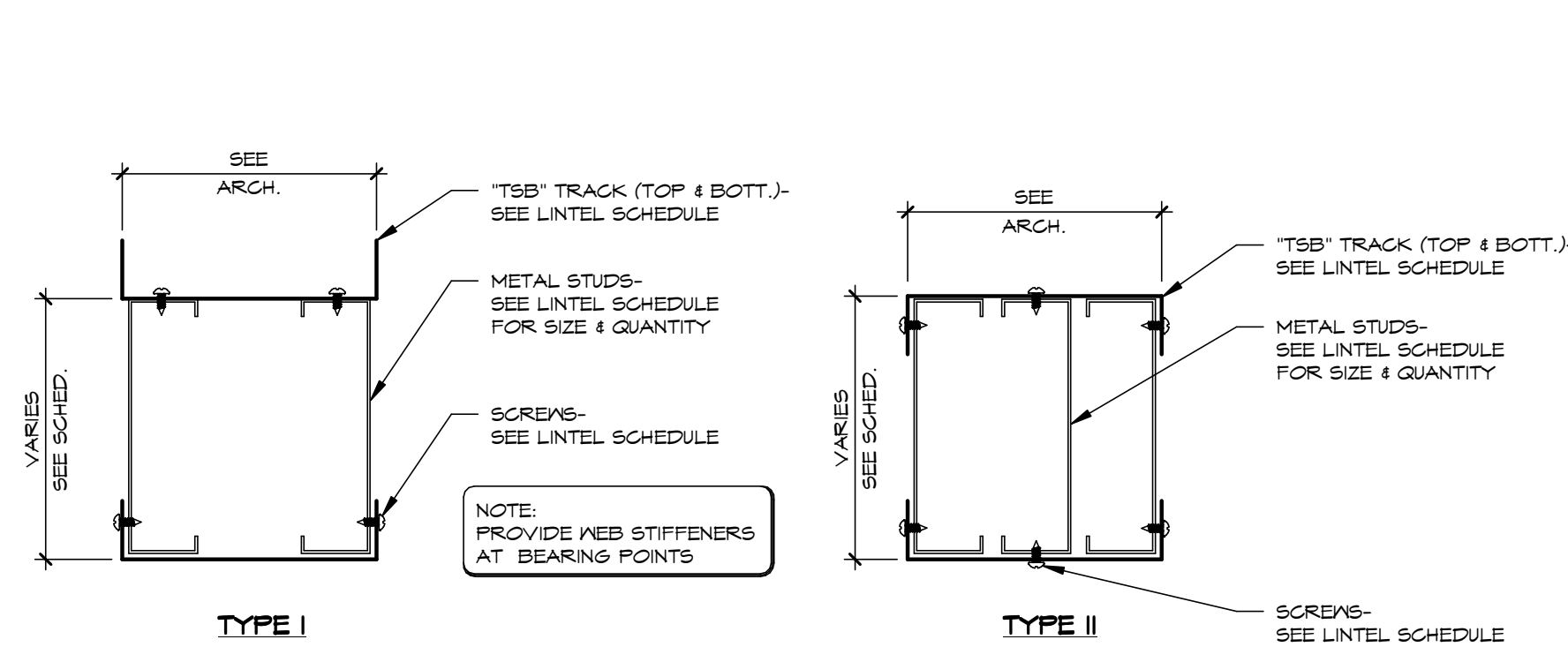


6 TYP. LINTEL & BEAM BEARING PLATE DETAILS
NOT TO SCALE

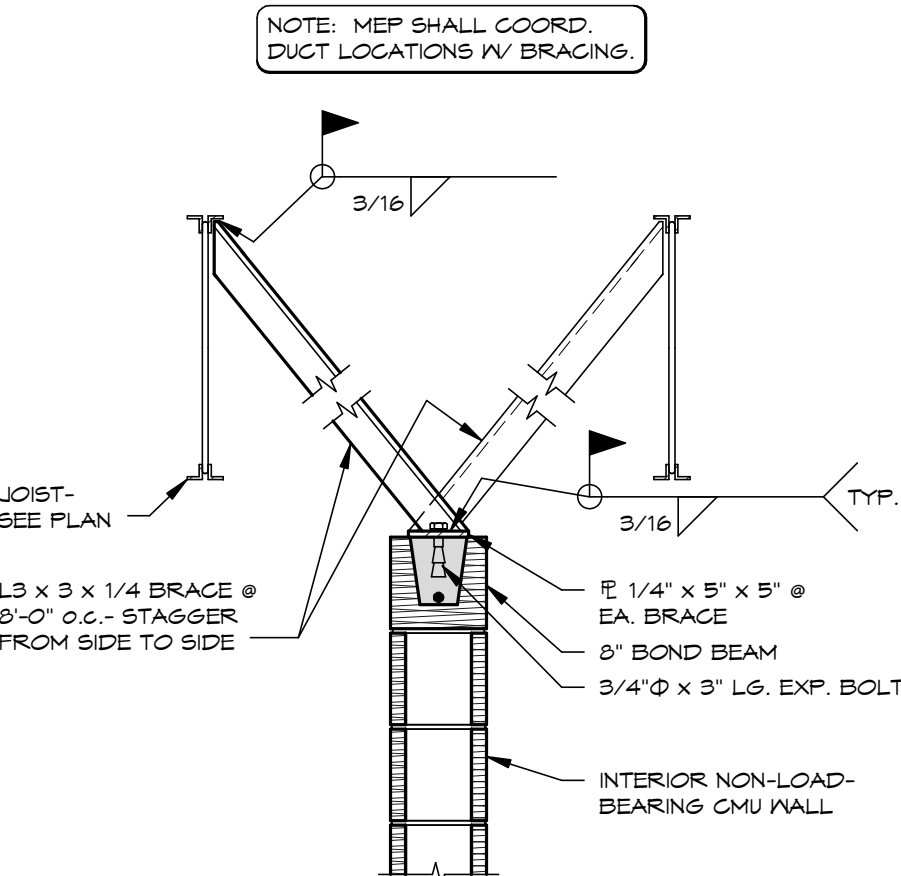


1 DETAIL
3/4" x 1'-0"

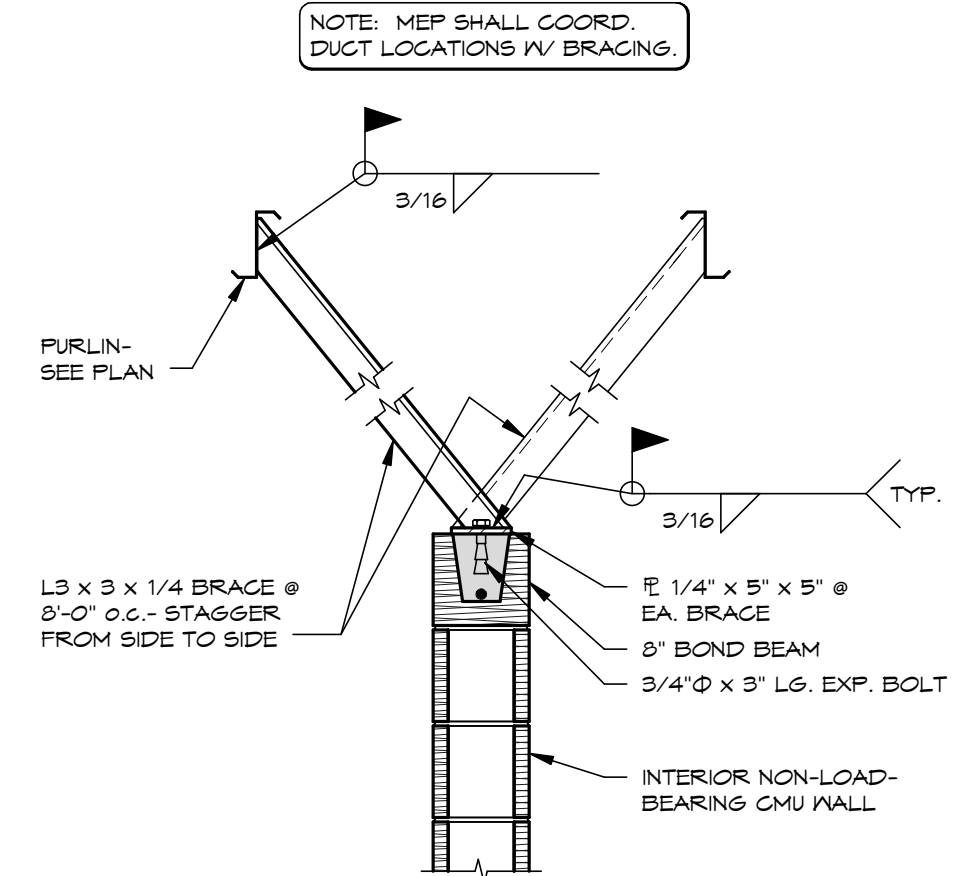
2 DETAIL
3/4" x 1'-0"



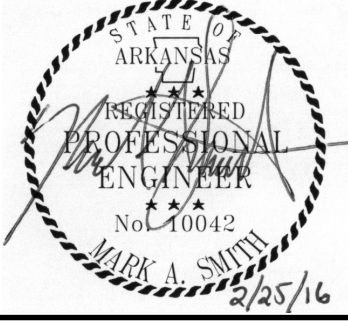
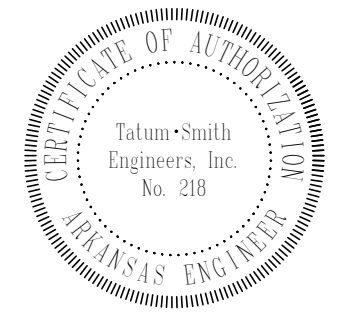
3 TYP. STUD LINTEL DETAILS
3" x 1'-0"



4 TYP. CMU WALL BRACING
NOT TO SCALE



5 TYP. CMU WALL BRACING @ PURLINS
3/4" x 1'-0"



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CAD Technician

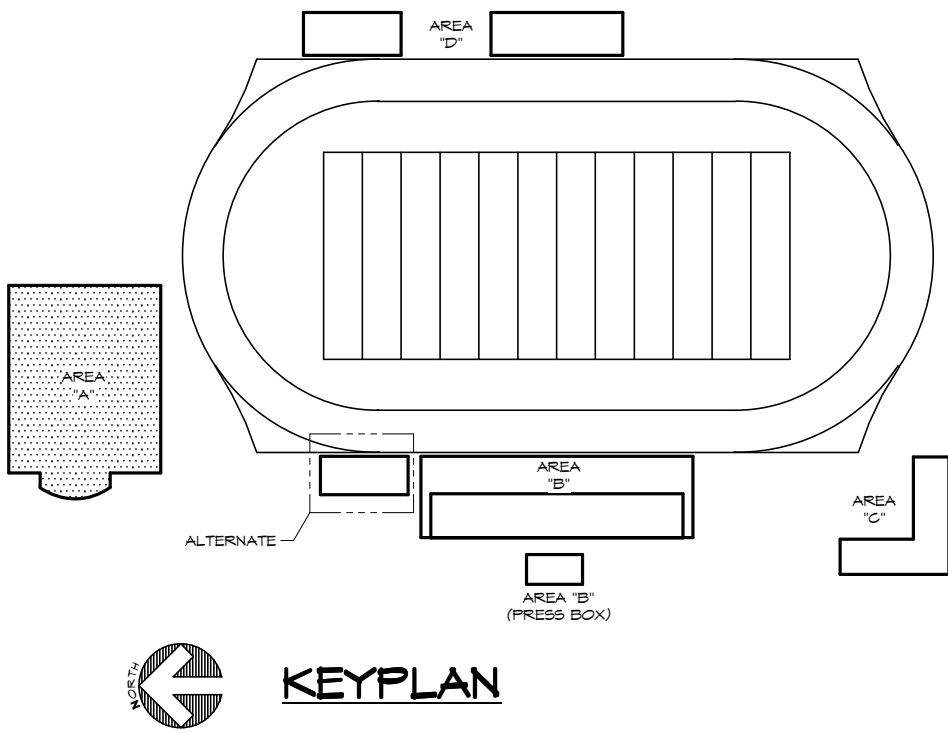
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TAT
Engineer of Record
Project Designer



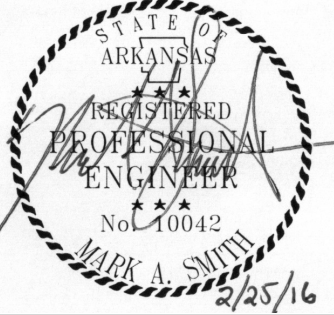
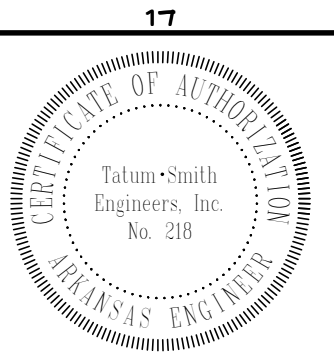
1 LINTEL & MASONRY PLAN - AREA "A"
1/8" = 1'-0"

- LEGEND:**
- L1 DENOTES LINTEL MARK (SEE LINTEL SCHED. ON DWS. S3.0)
 - DENOTES LOAD-BEARING CMU WALL
 - DENOTES NON-LOAD-BEARING CMU WALL
 - DENOTES MASONRY WALL REINF. MARK (SEE MASONRY WALL REINF. SCHED. ON DWS. S3.0)
 - DENOTES LINTEL BEARING PLATE (SEE DTL. 6/S3.0)

- PLAN NOTES:**
- SEE DWGS. S1.1, S1.2, S1.3 & S1.4 FOR GENERAL NOTES & TYP. DETAILS.



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CARTHAGE R-9 SCHOOL DISTRICT
CARTHAGE STADIUM COMPLEX
FIR RD & RIVER ST, CARTHAGE, MO

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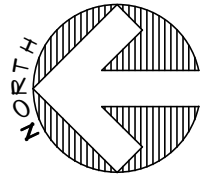
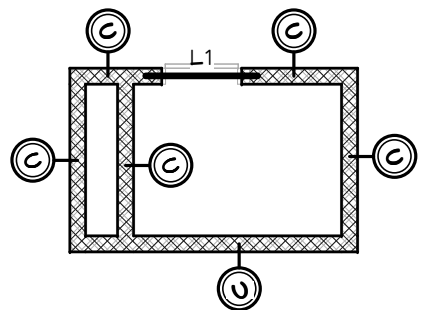
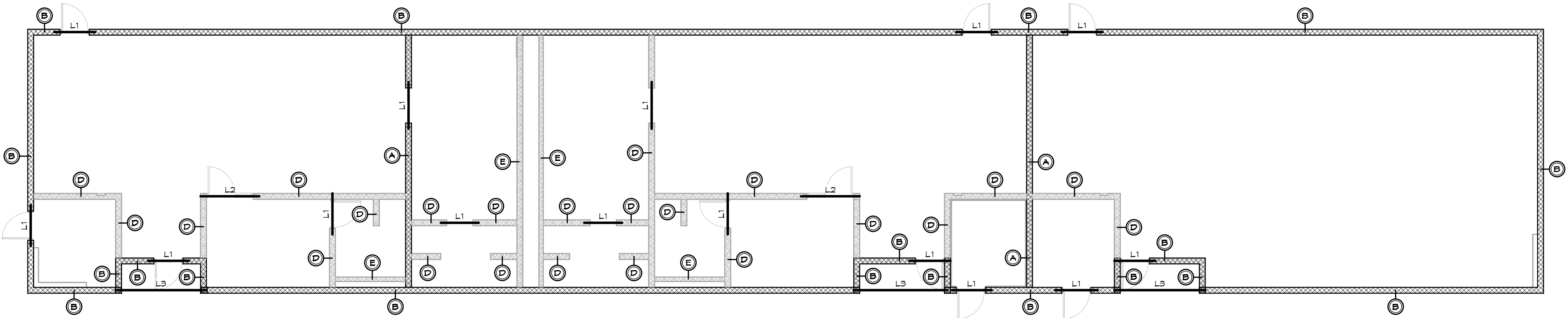
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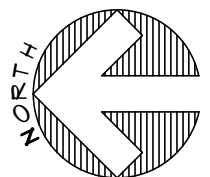
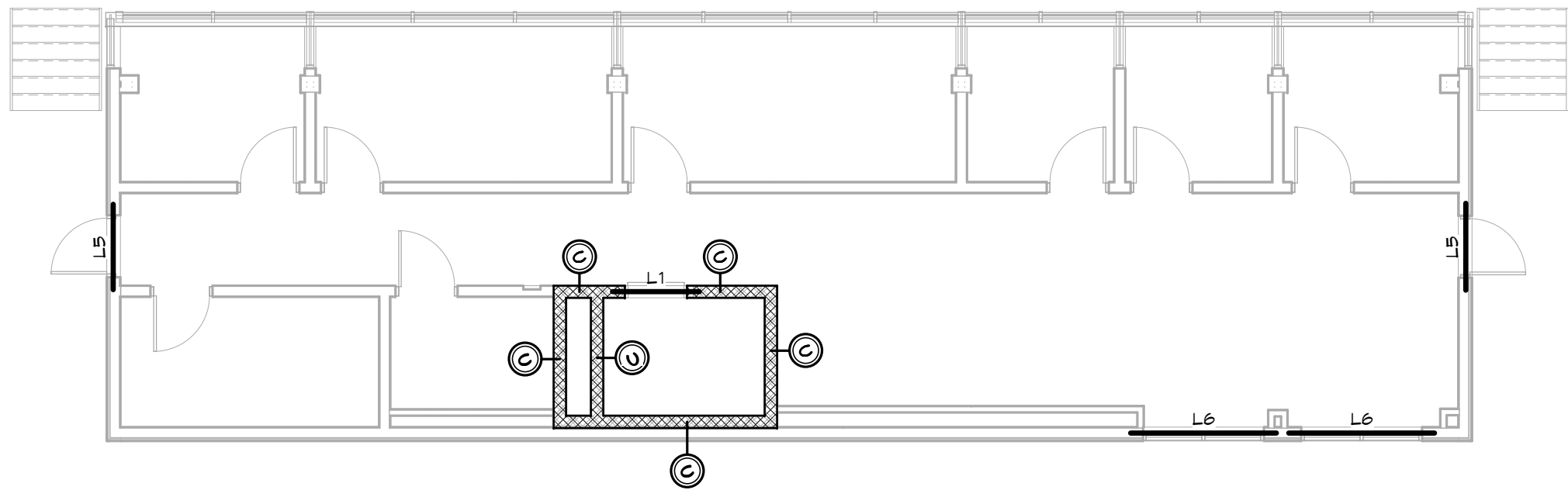
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EEC
CAD Technician

MAS
Engineer of Record
Project Designer
TAT

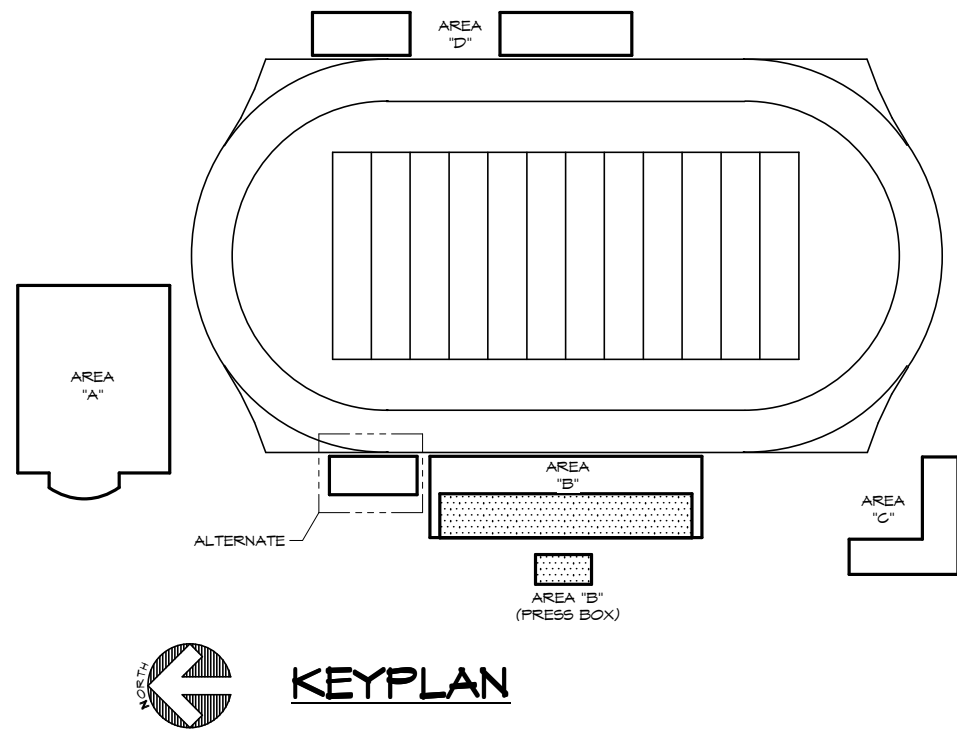


1 FIRST FLOOR LINTEL & MASONRY PLAN - AREA "B"
1/8" = 1'-0"



2 PRESS BOX FLOOR LINTEL & MASONRY PLAN - AREA "B"
1/8" = 1'-0"

NOTE:
SEE DWG. S3.0.1 FOR
LEGEND & PLAN NOTES



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Arkansas
Professional Engineer
No. 40042
Mark A. Smith
2/25/16

Arkansas
Professional Engineer
No. 218
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CARTHAGE STADIUM COMPLEX
FIR RD & RIVER ST., CARTHAGE, MO

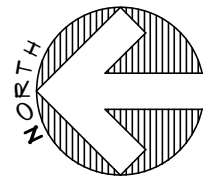
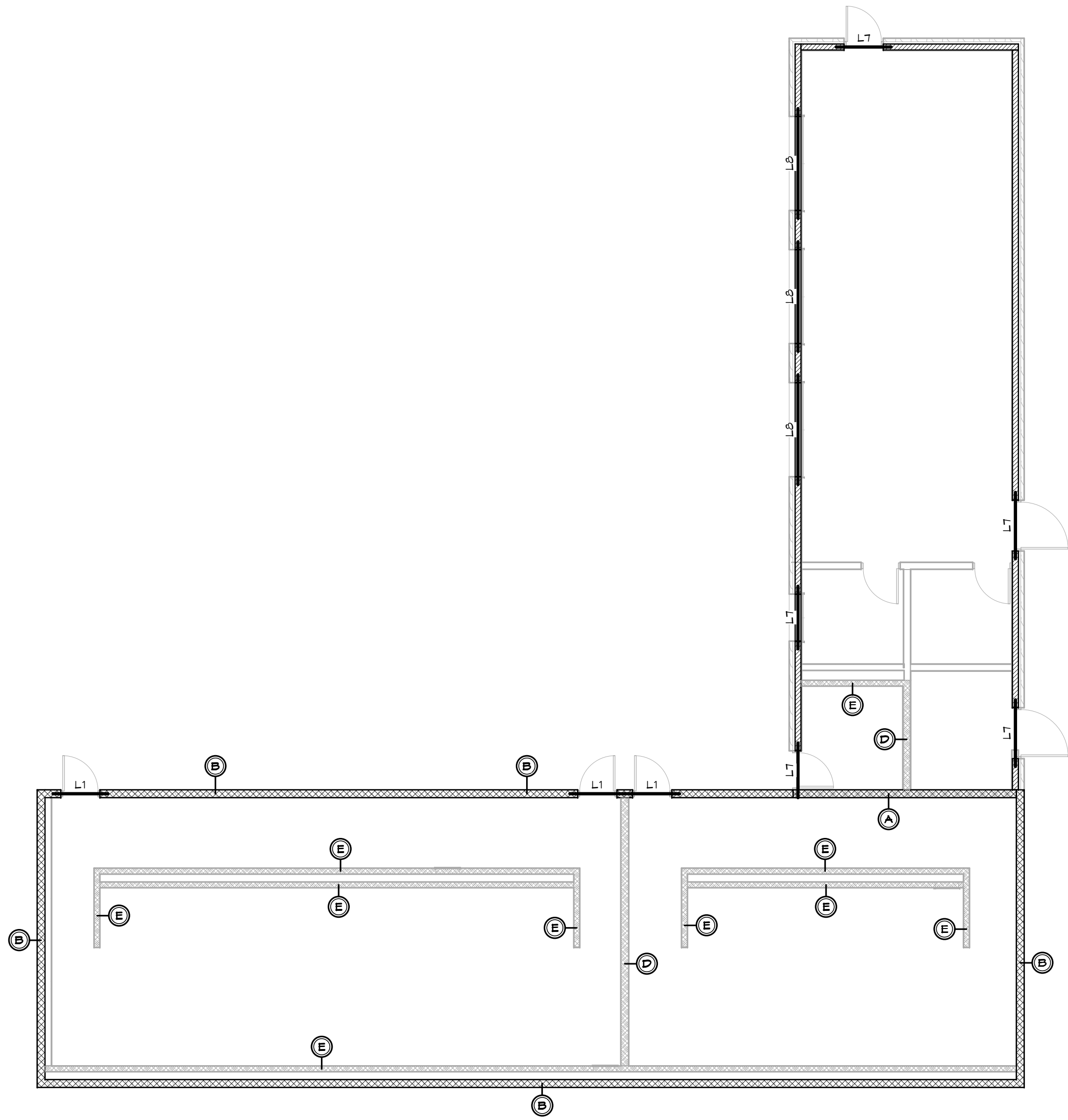
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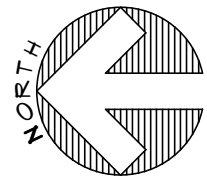
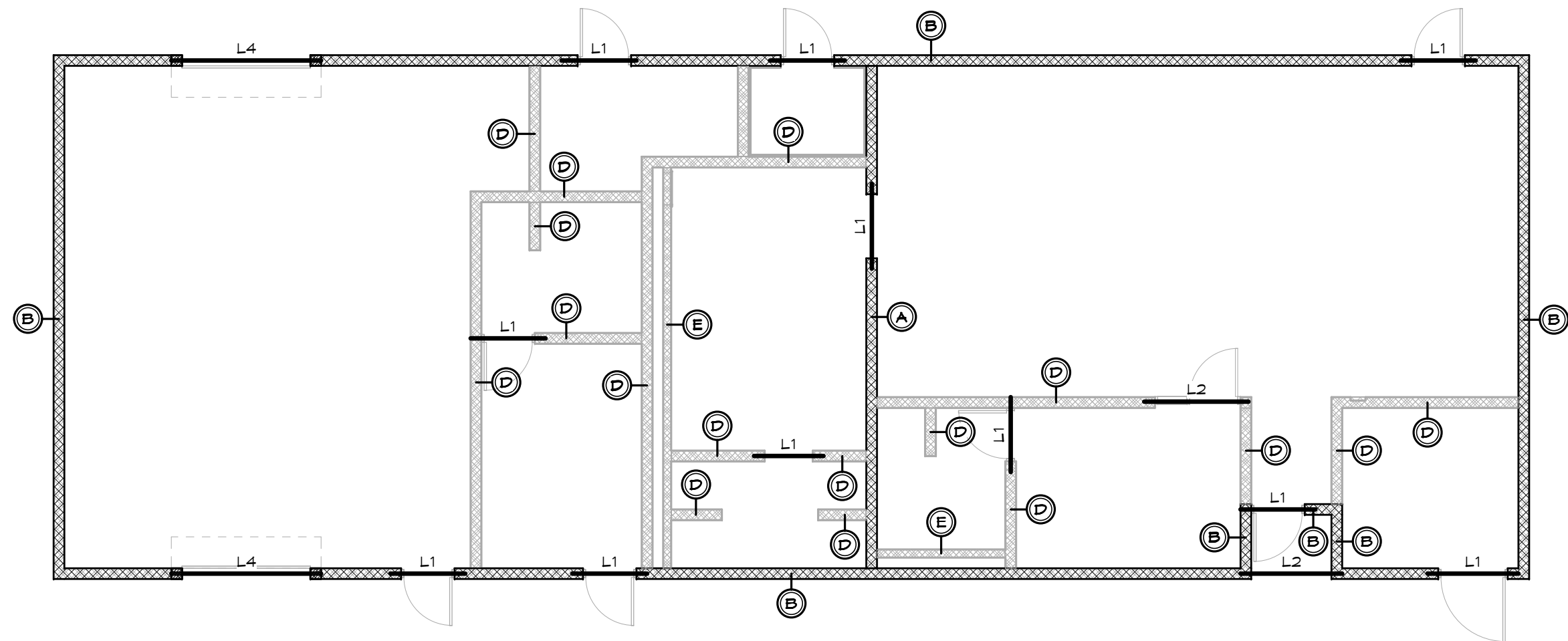
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CAD Technician: EEC
Engineer of Record: MAS
Project Designer: TAT

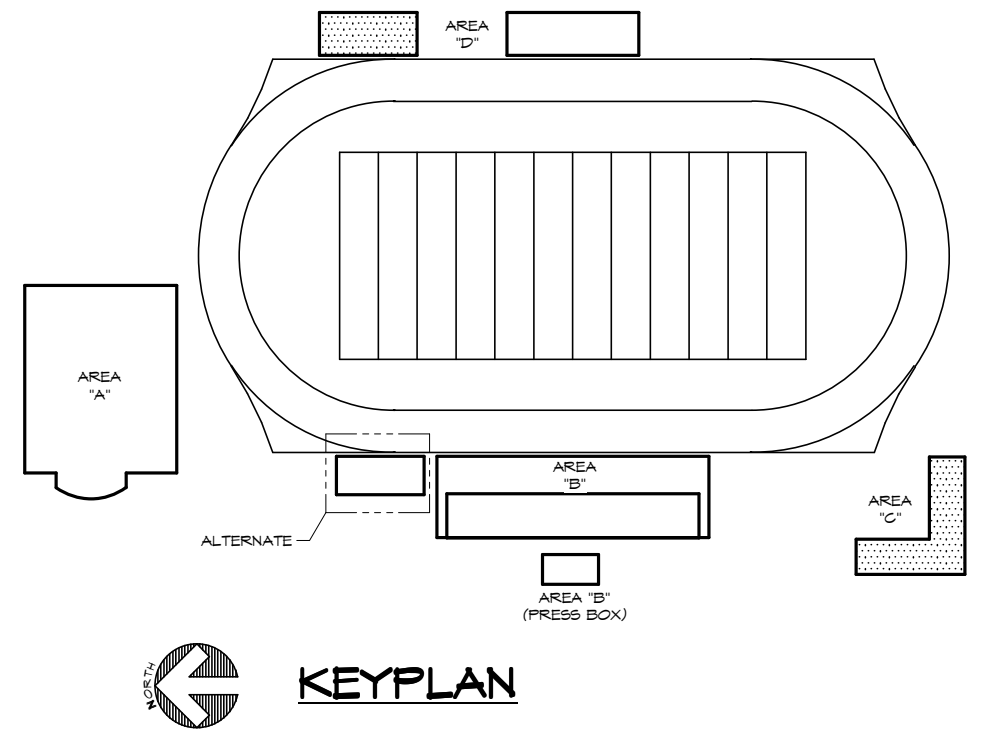


1 LINTEL & MASONRY PLAN - AREA "C"
1/8" = 1'-0"

NOTE:
SEE DMS, S3.0.1 FOR
LEGEND & PLAN NOTES



2 LINTEL & MASONRY PLAN - AREA "D"
1/8" = 1'-0"



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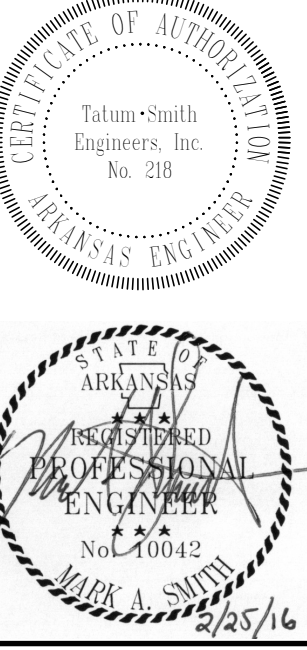
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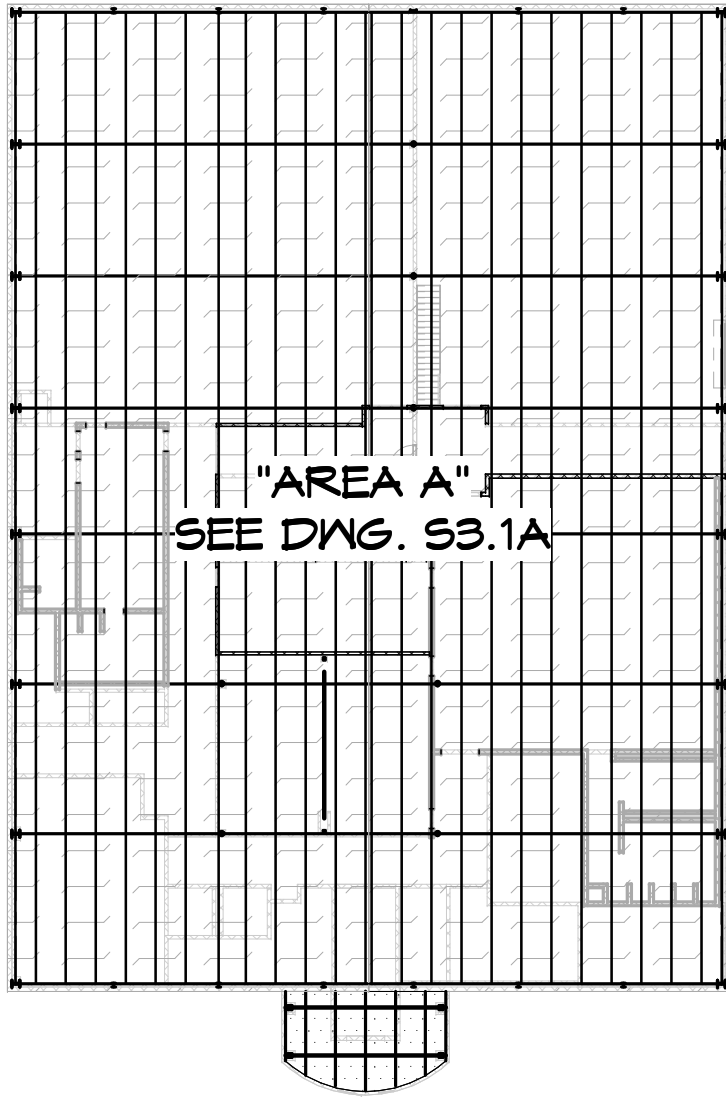
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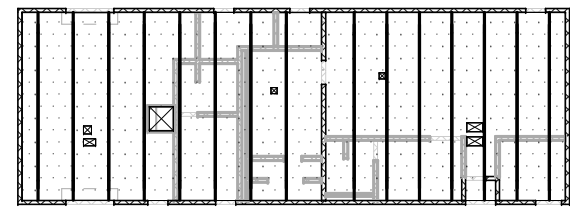
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Engineer of Record:
Project Designer

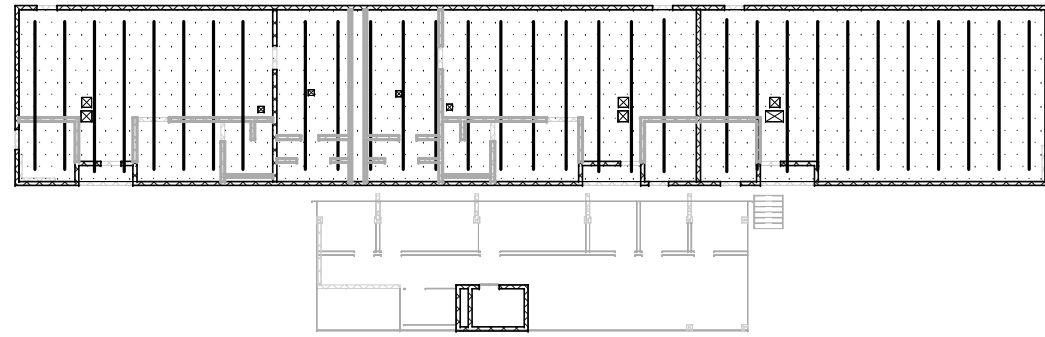
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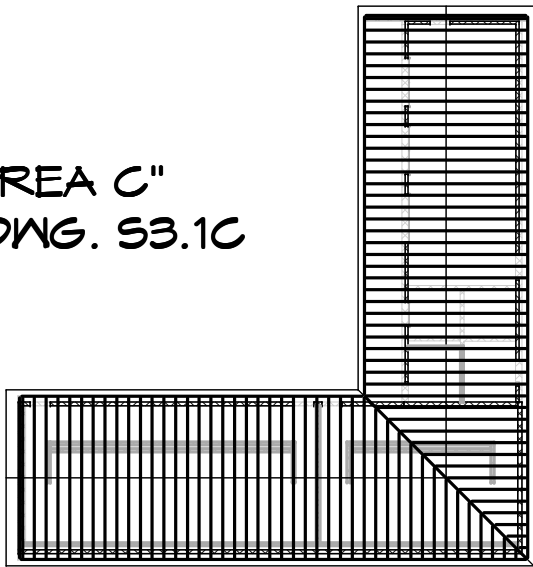
"AREA A"
SEE DWG. S3.1A



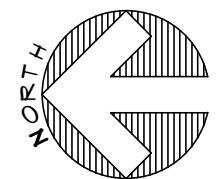
"AREA D"
SEE DWG. S3.1C



"AREA B"
SEE DWG. S3.1B



"AREA C"
SEE DWG. S3.1C



1

OVERALL ROOF FRAMING PLAN

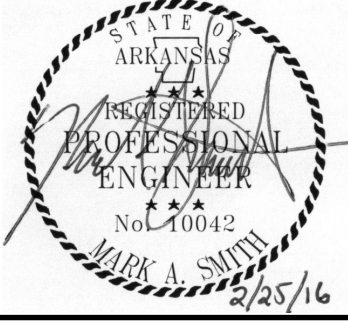
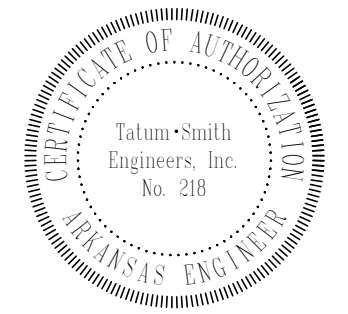
1/32" = 1'-0"

PLAN NOTES:

- THIS DRAWING IS FOR REFERENCE ONLY.
- SEE REFERENCED AREAS FOR ENLARGED PLANS.

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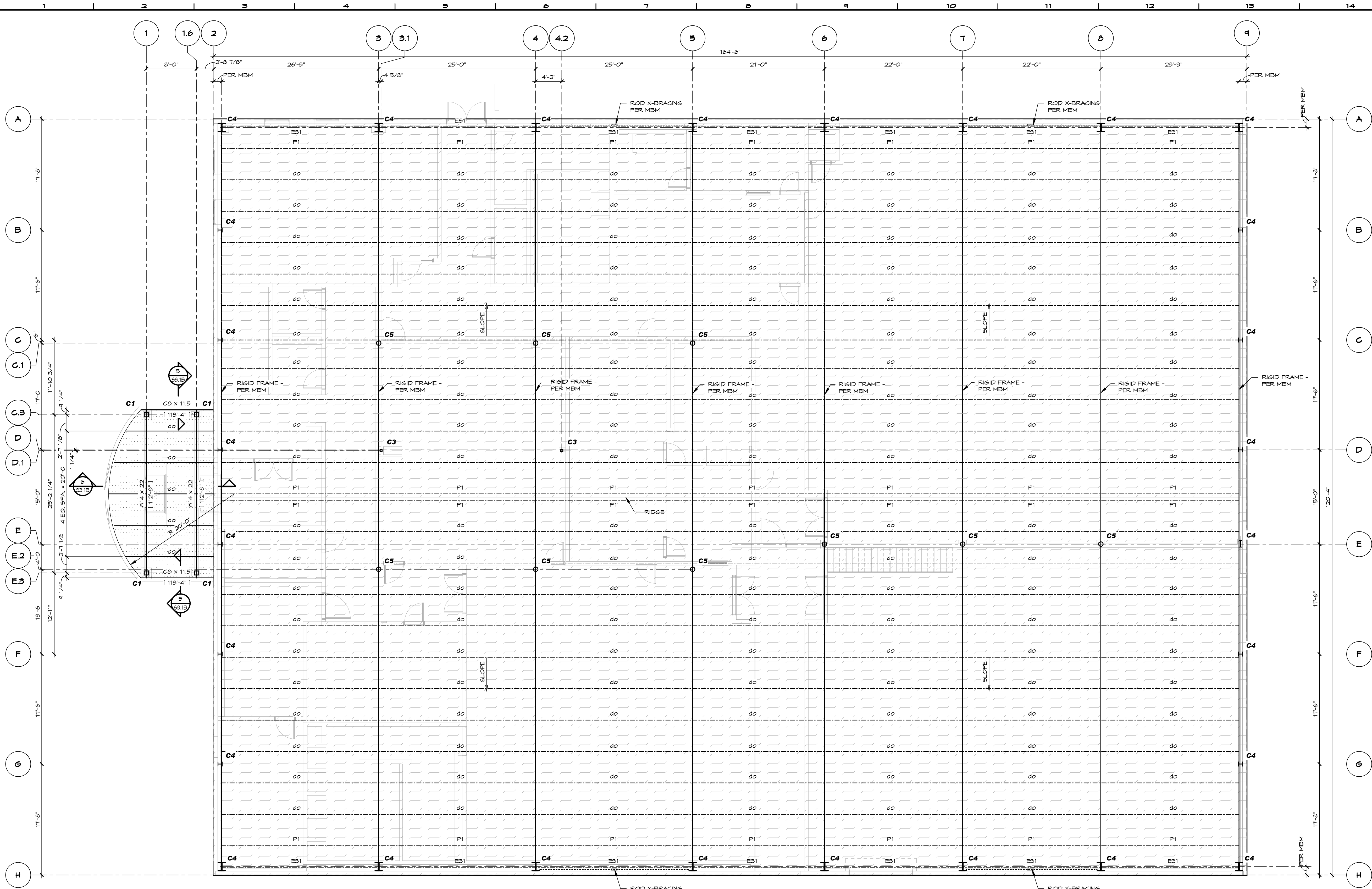
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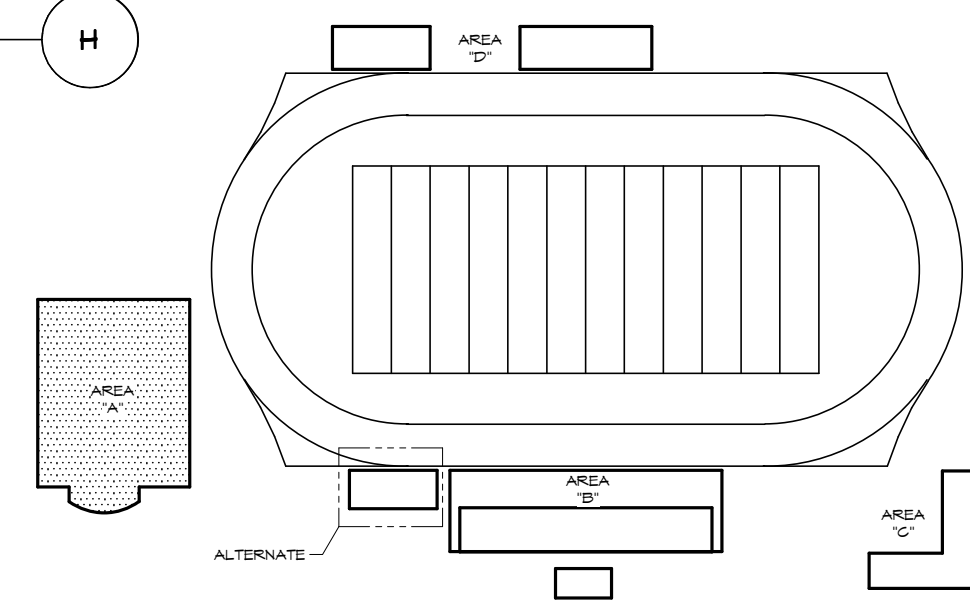
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Engineer of Record
Project Designer
TATUM



1 ROOF FRAMING PLAN - AREA "A"
1/8" = 1'-0"

- LEGEND:**
- C1 DENOTES COLUMN MARK (SEE COL. SCHED. ON DWG. S3.1A)
 - O.F. DENOTES OUTSIDE FACE
 - I.F. DENOTES INSIDE FACE
 - T.O.S. DENOTES TOP OF STEEL ELEVATION
 - J.B.E. DENOTES JOIST BEARING ELEVATION
 - T.B.E. DENOTES TRUSS BEARING ELEVATION
 - T1 DENOTES GABLE TRUSS
 - T2 DENOTES JACK TRUSS
 - P1 DENOTES PURLIN - PER MBM
 - ES1 DENOTES EVE STRUT - PER MBM
 - (2,500#) DENOTES EQUIPMENT WEIGHT IN POUNDS
 - [120'-0"] DENOTES TOP OF BEAM ELEVATION
 - [] DENOTES BEAM BEARING PLATE (SEE DTL. 6/33.0)
 - [] DENOTES LOAD-BEARING STUD WALL
 - [] DENOTES LOAD-BEARING CMU WALL
 - [] DENOTES NON-LOAD-BEARING CMU WALL
 - [] DENOTES LIMITS OF ROOF DECK (SEE PLAN NOTES)
 - [] DENOTES METAL BUILDING ROOF DECK - PER MBM
 - [] DENOTES LIMITS OF ROOF SHEATHING (SEE PLAN NOTES)
 - [] DENOTES LIMITS OF OVERFRAMING (SEE PLAN NOTES)
 - SLOPE DENOTES DIRECTION OF ROOF SLOPE (COORD. PITCH W/ ARCH.)
 - [] DENOTES FRAMED OPENING IN ROOF - COORDINATE SIZE & LOCATION W/ MECH. CONTRACTOR (SEE DTL. 4/51.3)

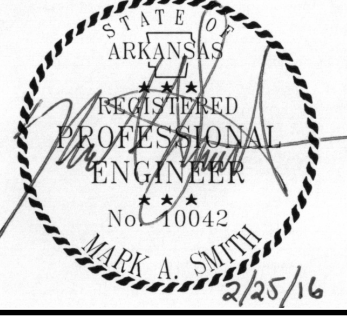
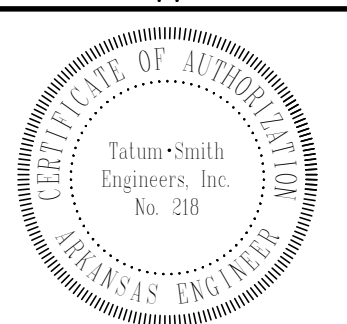
- PLAN NOTES:**
- TYPICAL ROOF DECK: 1 1/2" DEEP, 22 GA. PAINTED WIDE RIB STEEL DECK (SEE STEEL DECK GENERAL NOTES FOR ATTACHMENT REQUIREMENTS).
 - ROOF SHEATHING: 5/8" APA RATED PLYWOOD SHEATHING (40/20) WITH EXPOSURE 1 CLASSIFICATION. ATTACH SHEATHING TO TRUSSES/RAFTERS WITH 10d COMMON NAILS @ 6" O.C. ALONG PANEL EDGES AND 12" O.C. AT INTERMEDIATE TRUSSES/RAFTERS. PROVIDE SHEATHING CLIPS BETWEEN TRUSSES/RAFTERS.
 - OVERFRAMING SHALL BE 2 x 6 RAFTERS @ 24" O.C. SUPPORT EACH RAFTER WITH (2) 2 x 4 POSTS @ 6'-0" O.C. SUPPORTED OFF TRUSSES BELOW. TRUSSES BELOW OVERFRAMING SHALL BE SHEATHED W/ ROOF SHEATHING.
 - TOP OF STEEL DENOTES TOP OF MAIN STEEL.
 - TOP OF STEEL EL. VARIES. SEE PLAN.
 - TOP OF "K" SERIES JOISTS (2 1/2") TYP. (U.N.O.)
 - JOIST BEARING ELEVATION VARIES. SEE PLAN.
 - ALL ELEVATIONS BASED ON FINISH FLOOR EL. 100'-0". ACTUAL FIN. FLR. EL. 1019.00' AS ESTABLISHED BY GRAY ROCK CONSULTING INC.
 - SEE DWGS. S1.2, S1.3, & S1.4 FOR GENERAL NOTES & TYPICAL DETAILS.
 - SEE DWG. S1.4 FOR COLUMN SCHEDULE.
 - SEE ARCH. DWGS. & FOUNDATION PLANS FOR DIMENSIONS NOT SHOWN.
 - SEE DWGS. S3.0.1, S3.0.2 & S3.0.3 FOR LINTEL & MASONRY PLANS.



KEYPLAN

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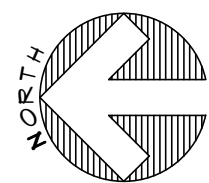
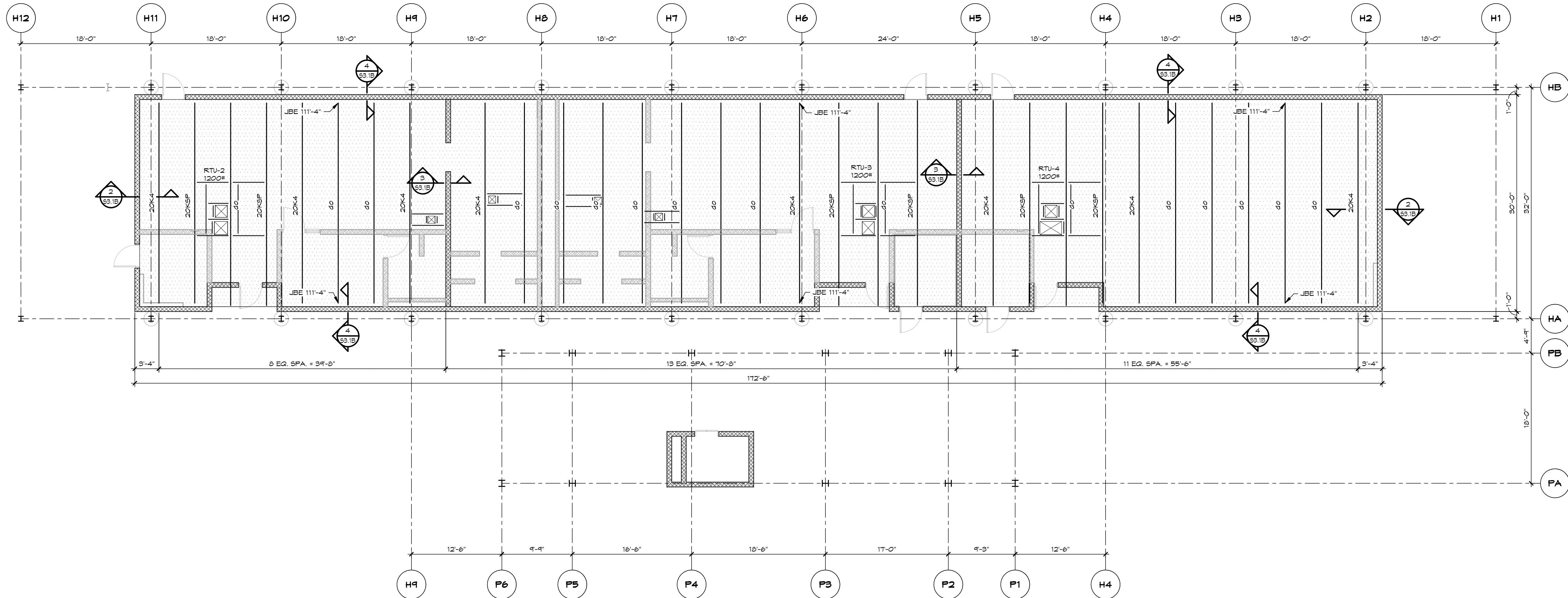
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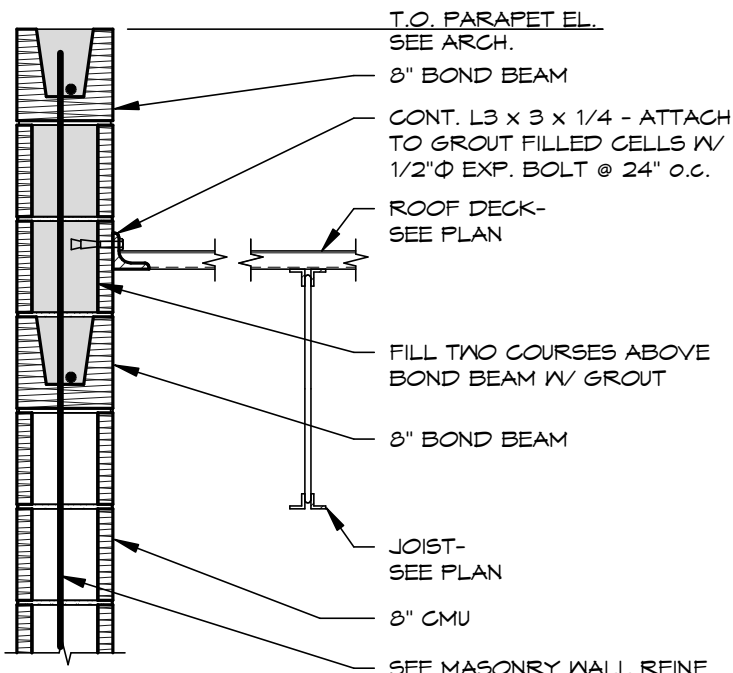
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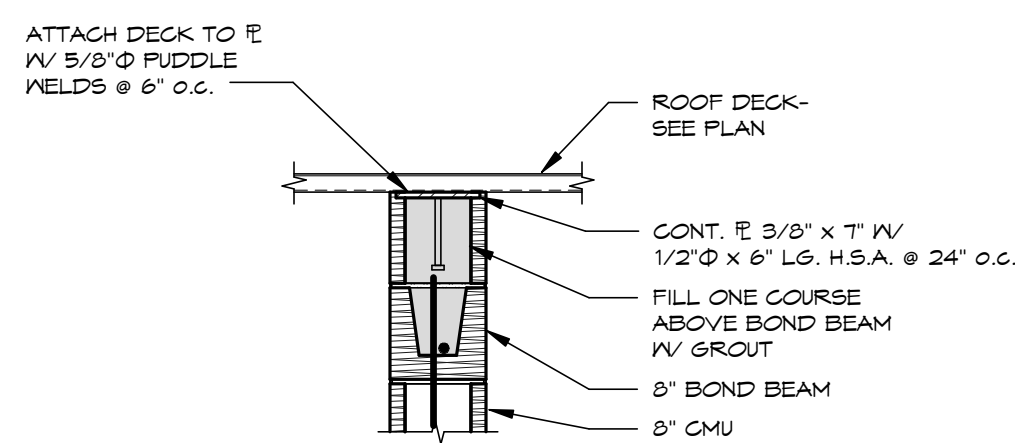


1 ROOF FRAMING PLAN - AREA "B"
1/8" = 1'-0"

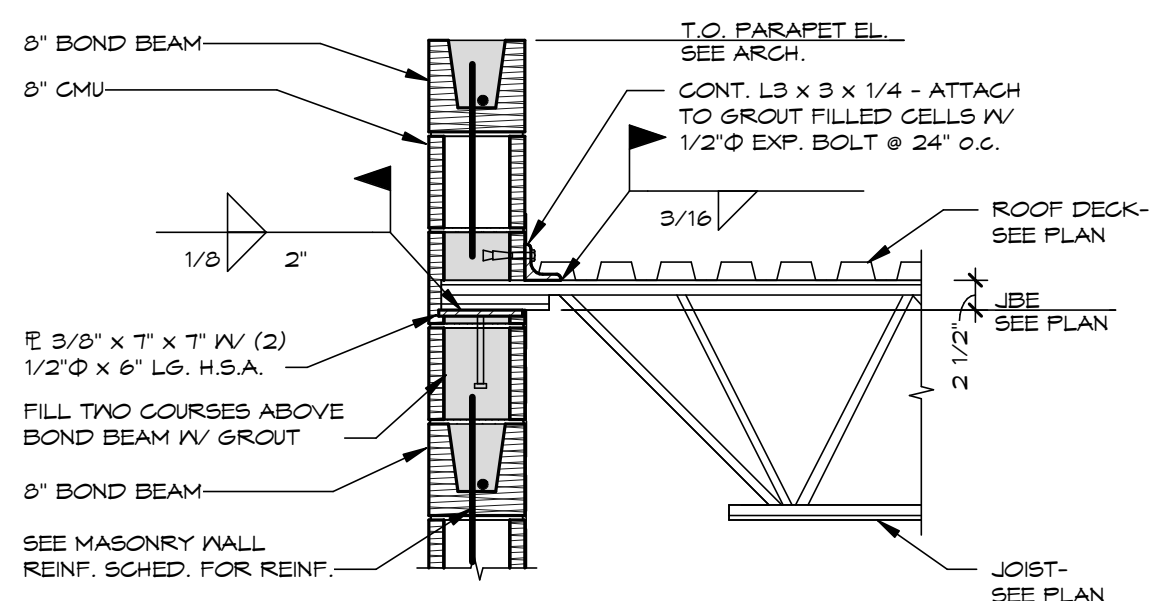
NOTE:
SEE DWG. S3.1A FOR
LEGEND & PLAN NOTES



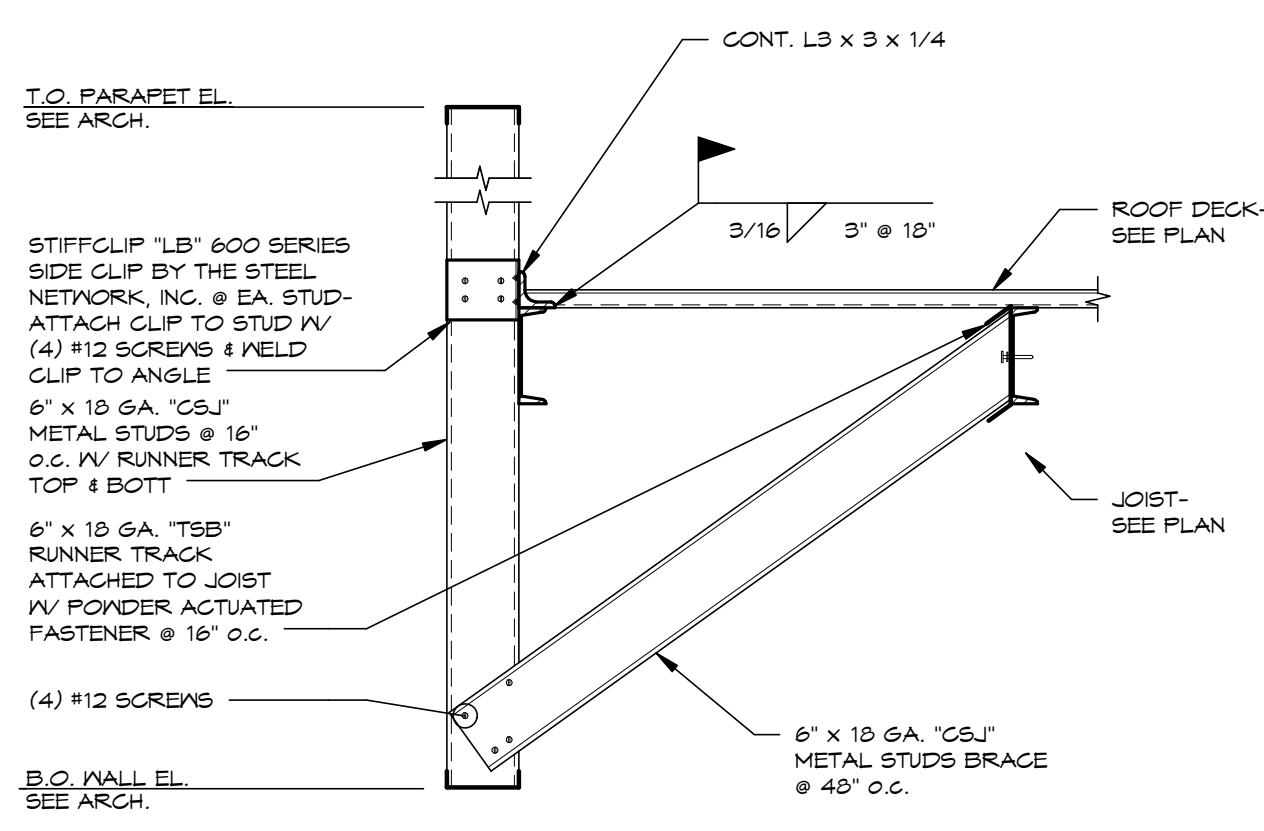
2 SECTION
3/4" = 1'-0"



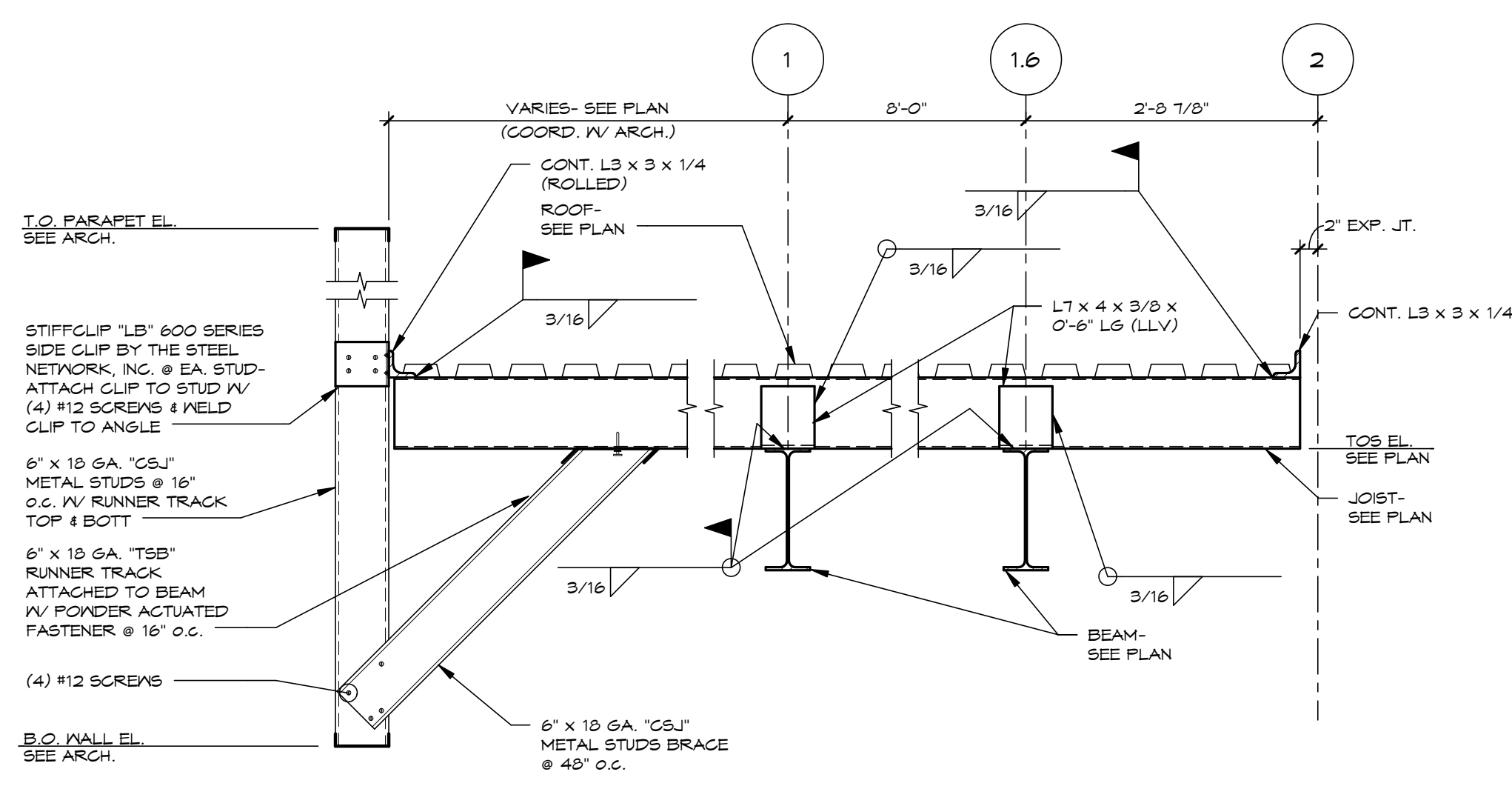
3 SECTION
3/4" = 1'-0"



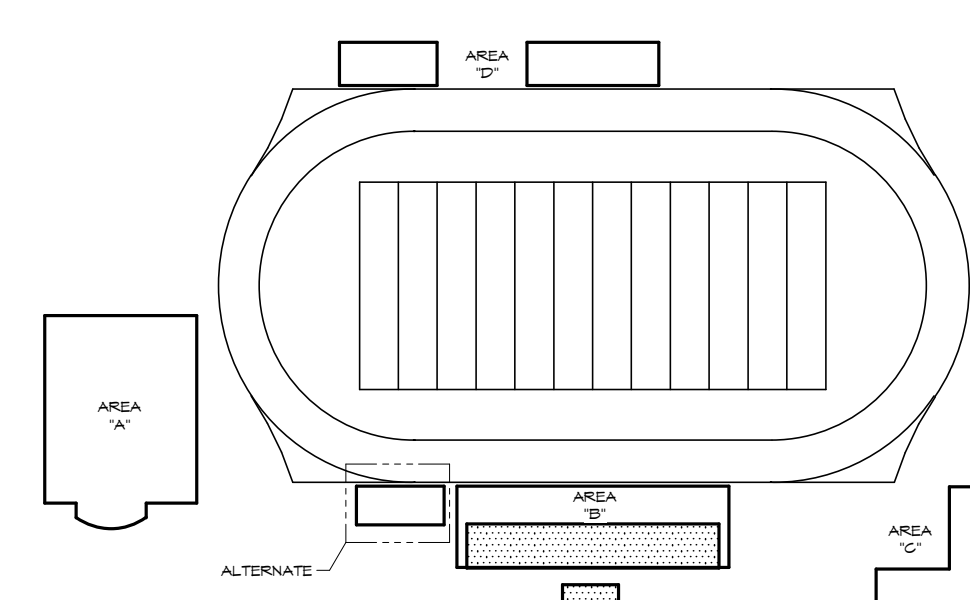
4 SECTION
3/4" = 1'-0"



5 SECTION
3/4" = 1'-0"

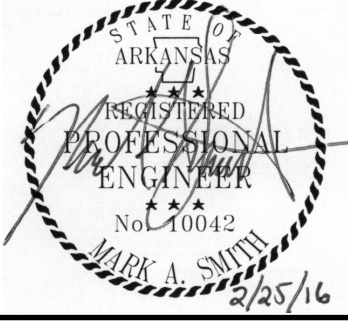
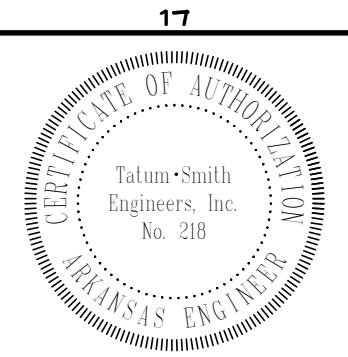


6 SECTION
3/4" = 1'-0"



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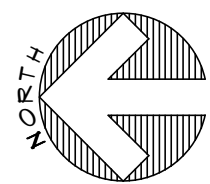
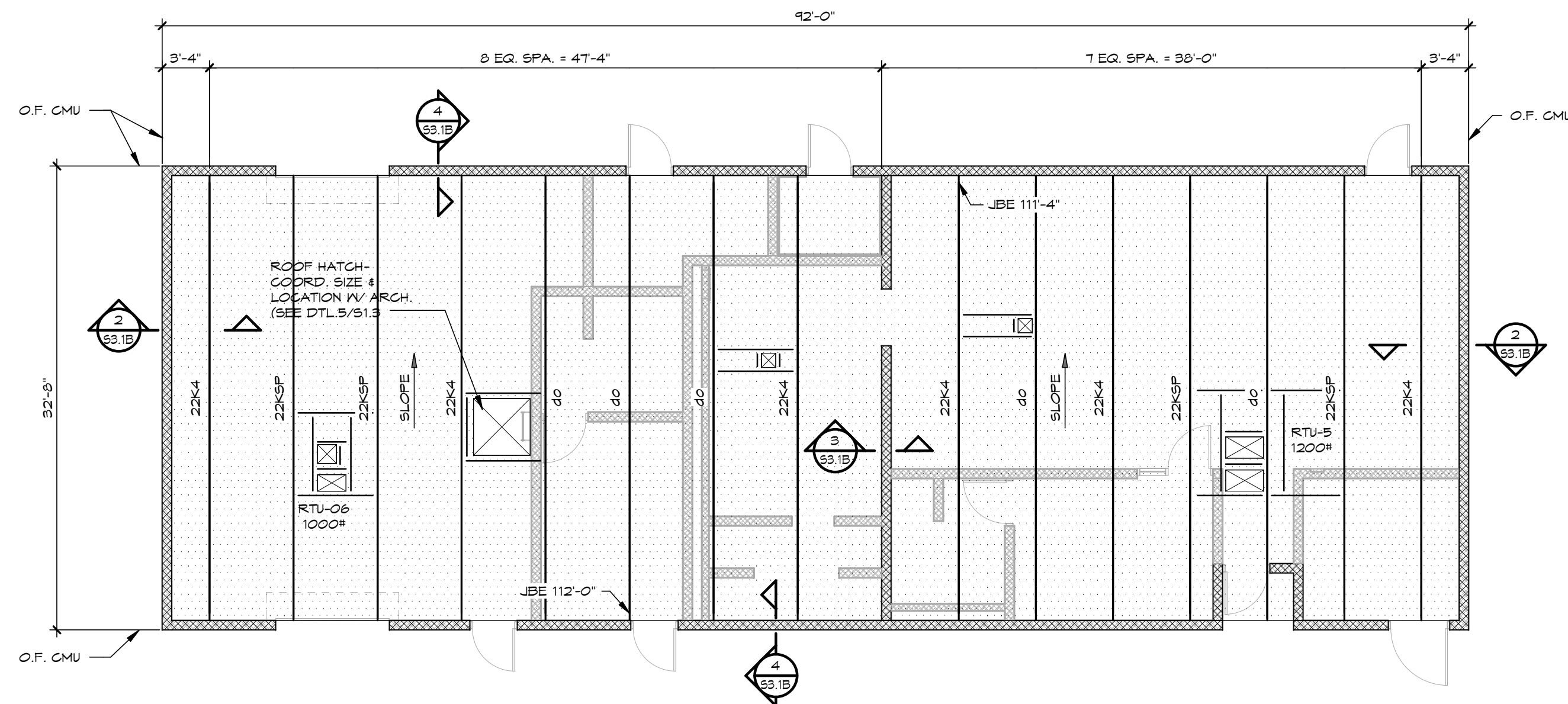
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CARTHAGE STADIUM COMPLEX
FIR RD & RIVER ST, CARTHAGE, MO

DRAWN BY: EEC	CHECK BY: MAS
ISSUE DATE: 02/29/2016	PROJECT NO.: 1365
REVISION DATES	DESCRIPTION
#	
1	AREA "B" (PRESS BOX)
SHEET	S3.1B
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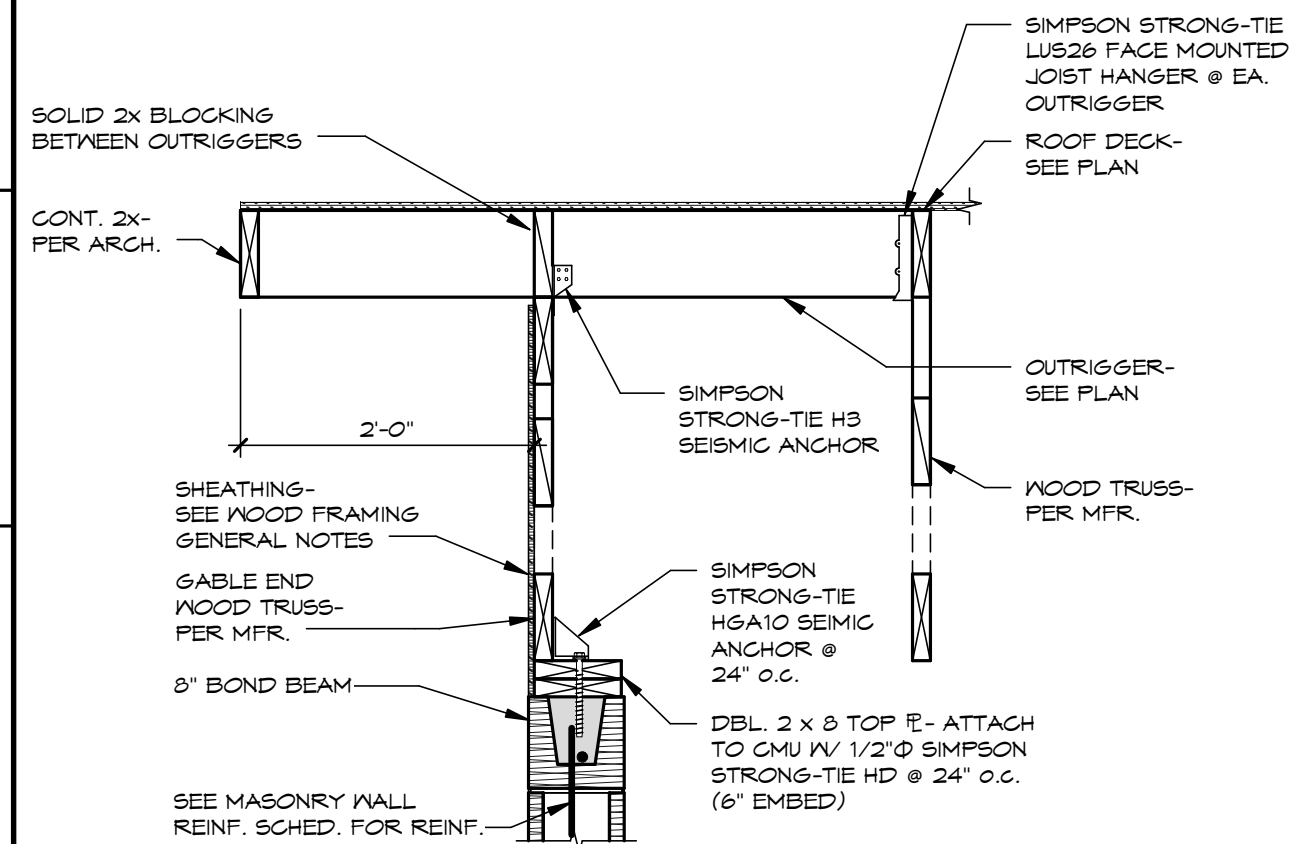
2/25/2016 12:59:51 PM

TS Project No.: 151210
EEC
CAD Technician
MAS
Tatum Smith
Engineers, Inc.
Project Designer

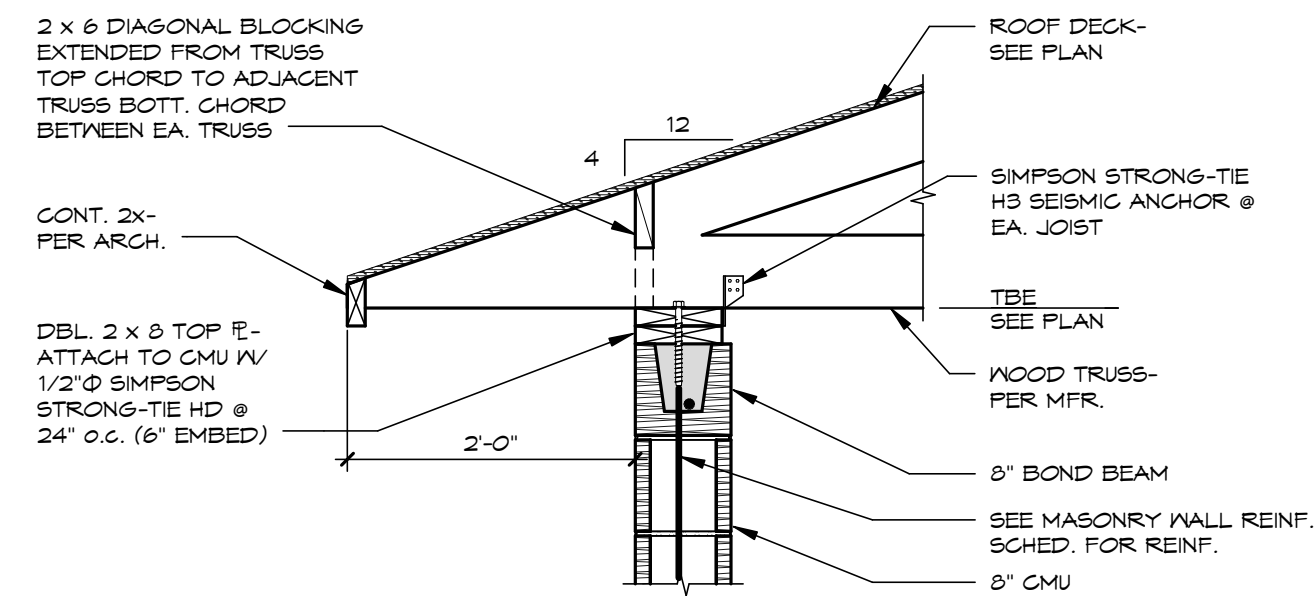


1 ROOF FRAMING PLAN - AREA "D"
1/8" = 1'-0"

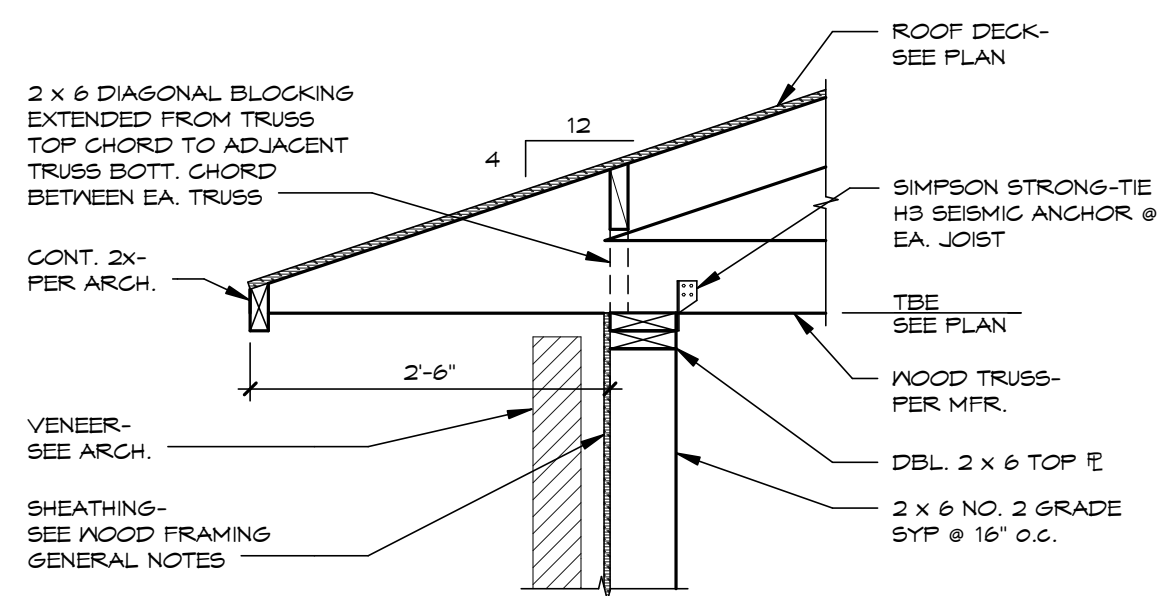
NOTE:
SEE DWS. S3.1A FOR
LEGEND & PLAN NOTES



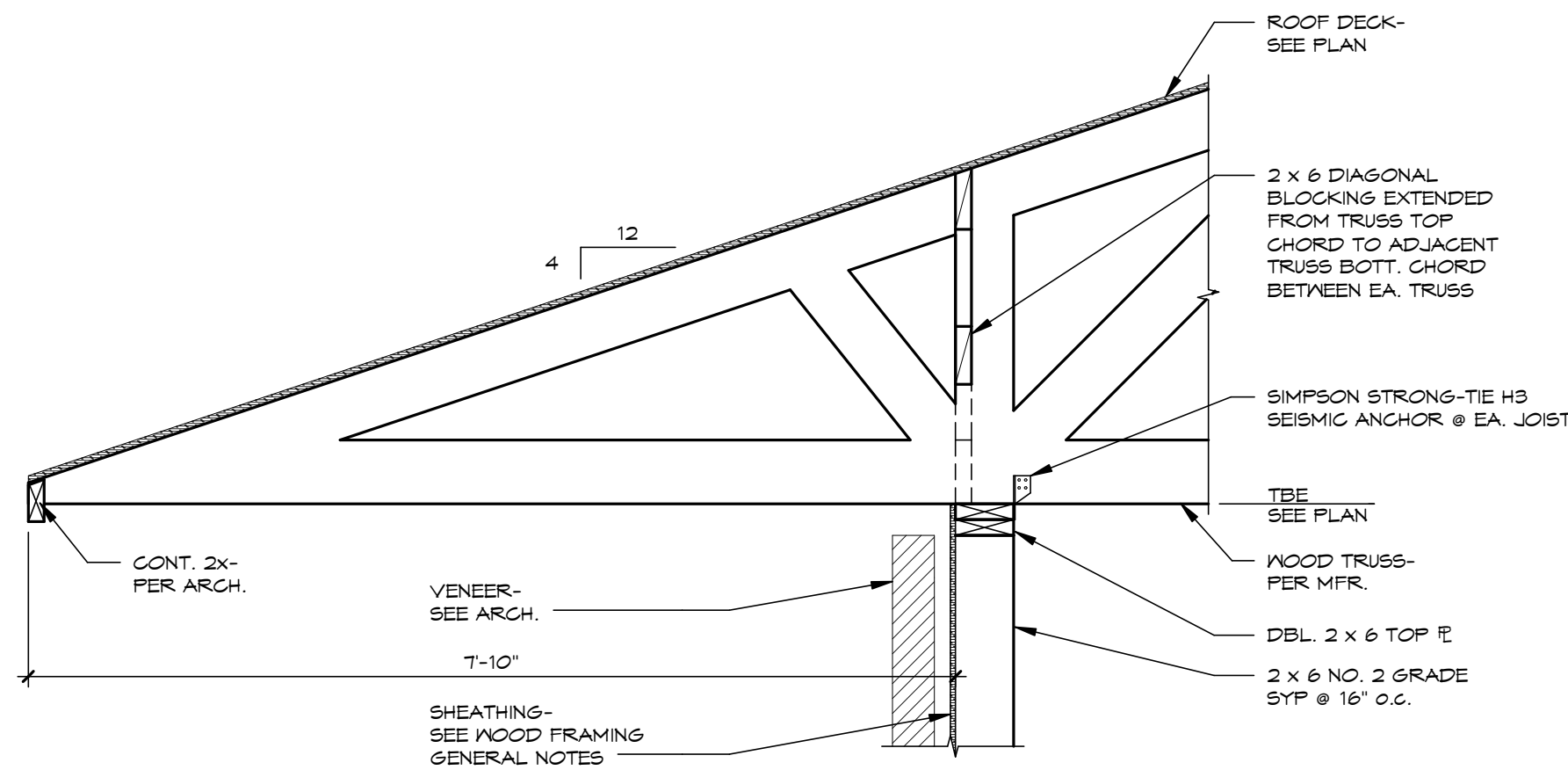
3 SECTION
3/4" = 1'-0"



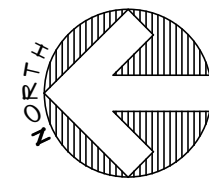
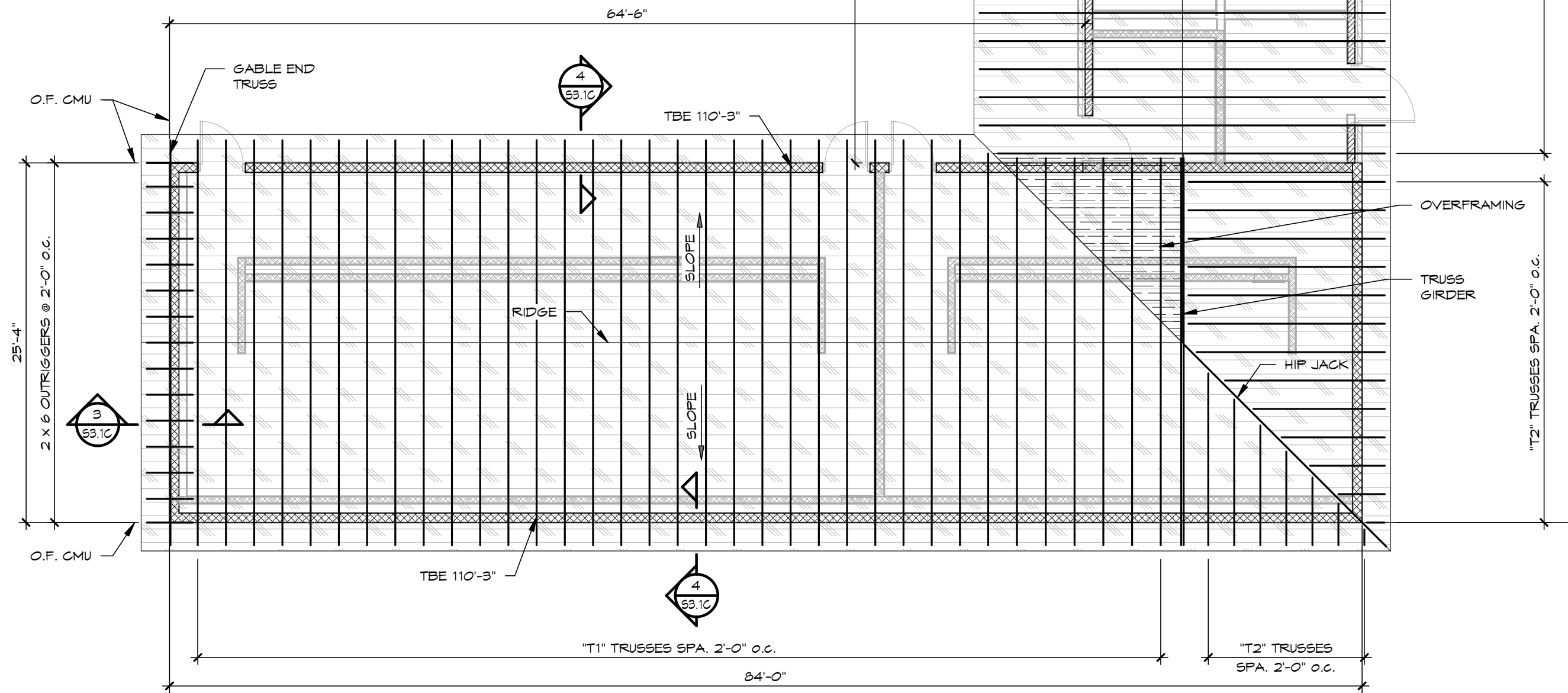
4 SECTION
3/4" = 1'-0"



6 SECTION
3/4" = 1'-0"

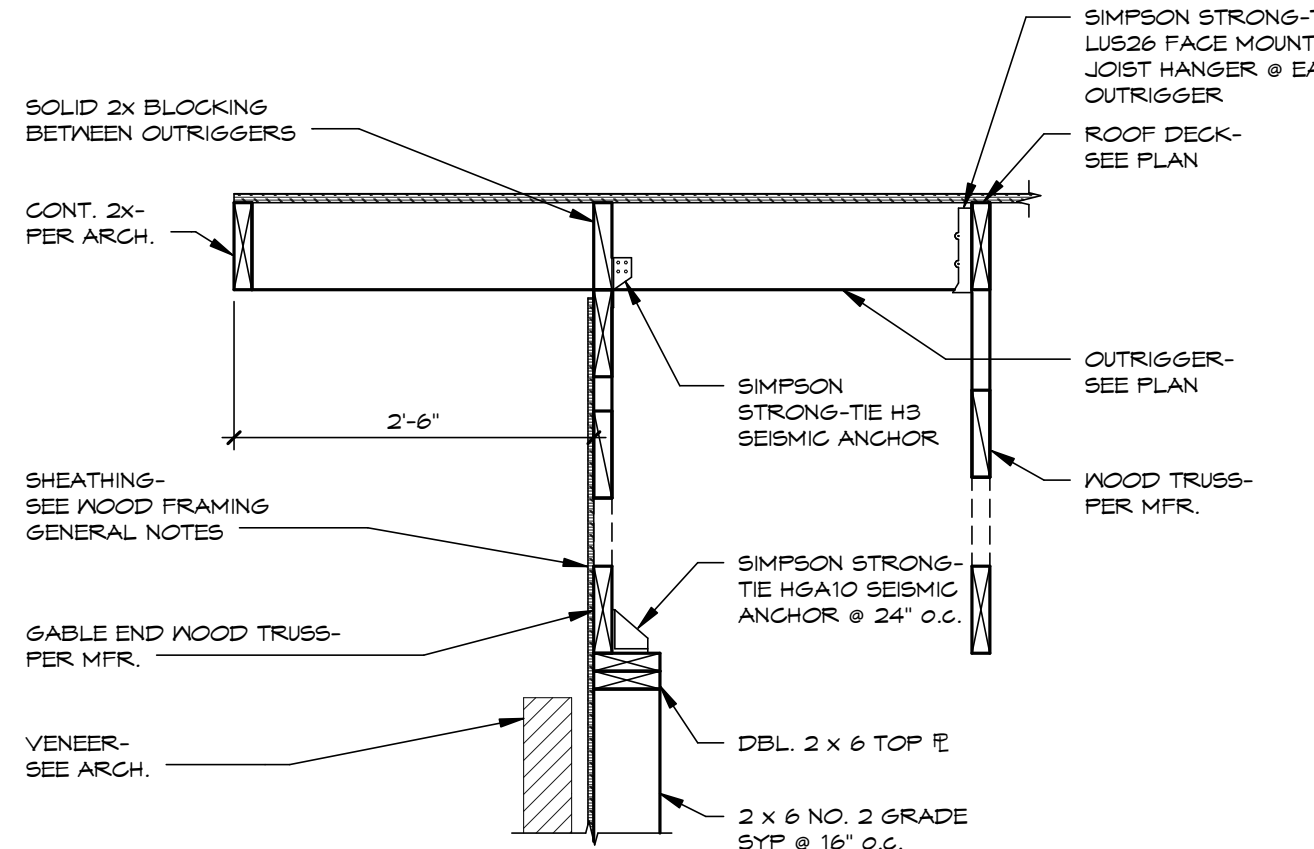


5 SECTION
3/4" = 1'-0"

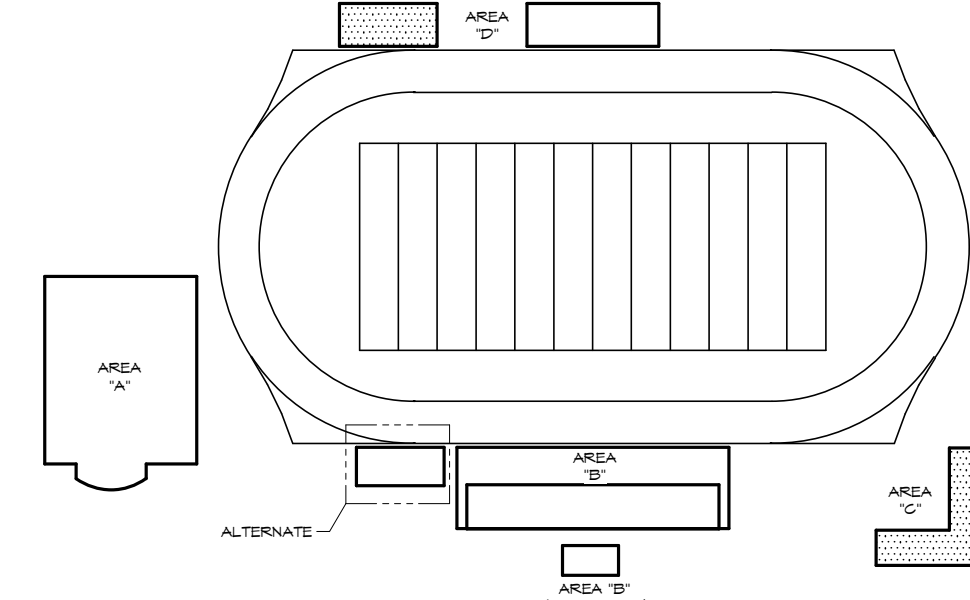


2 ROOF FRAMING PLAN - AREA "C"
1/8" = 1'-0"

NOTE:
SEE DWS. S3.1A FOR
LEGEND & PLAN NOTES



7 SECTION
3/4" = 1'-0"



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CARTHAGE STADIUM COMPLEX

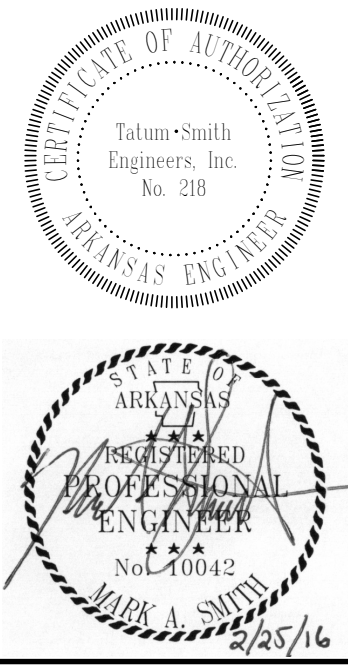
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151210
EEC
CAD Technician

151210
EEC
Project Designer

MAS
TAT

- LEGEND:**
- O.F. DENOTES OUTSIDE FACE
- JBE DENOTES JOIST BEARING ELEVATION
- (2,500#) DENOTES EQUIPMENT WEIGHT IN POUNDS
- [120'-4"] DENOTES TOP OF BEAM ELEVATION
- J1 DENOTES 8" x 14 GA. CSJ STUD JOIST @ 16" o.c.
- J2 DENOTES DBL. 8" x 14 GA. CSJ STUD JOIST @ 16" o.c. ATTACH TOGETHER BACK-TO-BACK W/ (2) #12 TEK SCREWS @ 6" o.c. FULL LENGTH OF JOIST
- [] DENOTES LIMITS OF FIRE-TREATED FLYWOOD FLOOR DECK (SEE PLAN NOTES) (T.O. DECK EL. SEE PLAN)
- DENOTES L4 x 4 x 3/8 DIAGONAL BRACING EXTENDING FROM BEAM BOTTOM FLANGE UP TO ROOF FRAMING (SEE DTL. 6/53.1D).

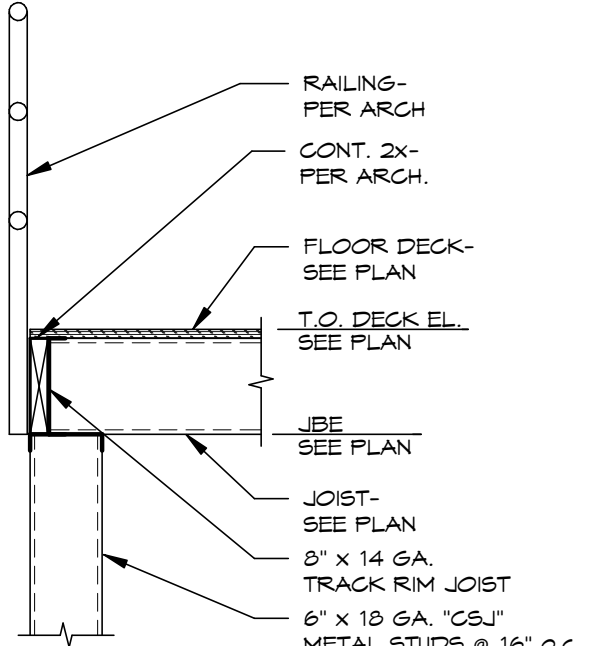
PLAN NOTES:

1. FLYWOOD FLOOR DECK, 1 1/2" STURD-I-FLOOR TONGUE & GROOVE FIRE-TREATED FLYWOOD SHEATHING. ATTACH SHEATHING TO JOIST W/ #10 TEK SCREWS @ 6" o.c. ALONG PANEL EDGES AND @ 12" o.c. AT INTERMEDIATE JOIST.
2. ALL ELEVATIONS BASED ON FINISH FLOOR EL. 100'-0". ACTUAL FIN. FLR. EL. 103.00' AS ESTABLISHED BY GRAY ROCK CONSULTING, INC.
3. SEE DWGS. S1.2, S1.3, & S1.4 FOR GENERAL NOTES & TYPICAL DETAILS.
4. SEE DWG. S1.4 FOR COLUMN SCHEDULE.
5. SEE ARCH. DWGS. & FOUNDATION PLANS FOR DIMENSIONS NOT SHOWN.
6. SEE DWGS. S3.0.1, S3.0.2 & S3.0.3 FOR LINTEL & MASONRY PLANS.
7. PROVIDE HORIZONTAL BLOCKING @ 4'-0" o.c. VERTICALLY FOR FULL HEIGHT OF WALL AT ALL LOAD-BEARING STUD WALLS.

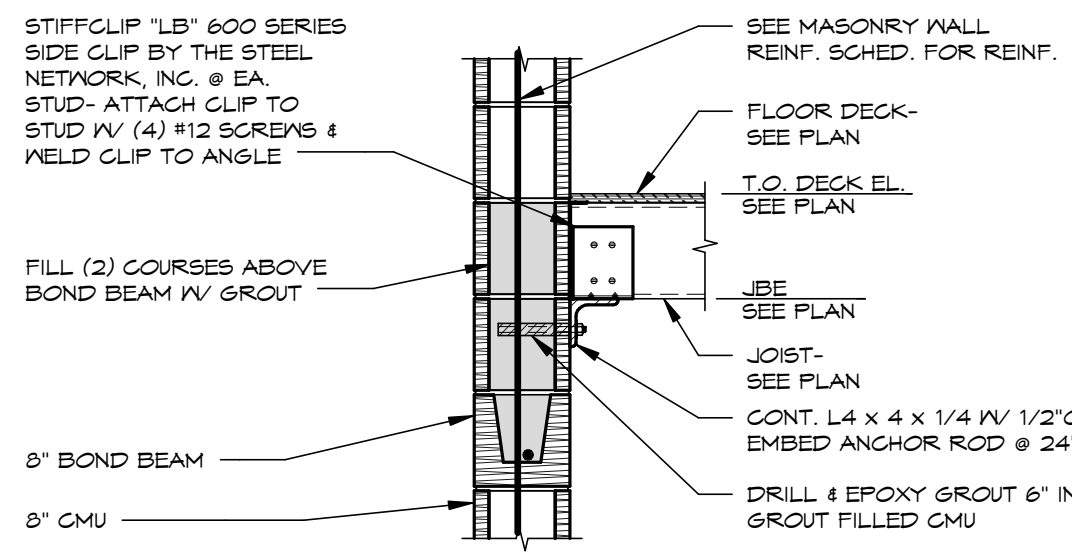
NOTE:
PROVIDE "J2" JOIST BENEATH ALL MECH. UNITS. COORD. EXACT LOCATION & SIZE OF UNIT W/ MECH. CONTRACTOR PRIOR TO MEZZANINE FLOOR CONSTRUCTION.



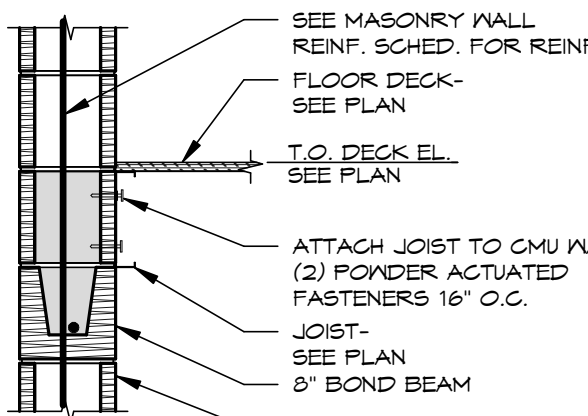
1 MEZZANINE FRAMING PLAN - AREA "A"
1/8" = 1'-0"



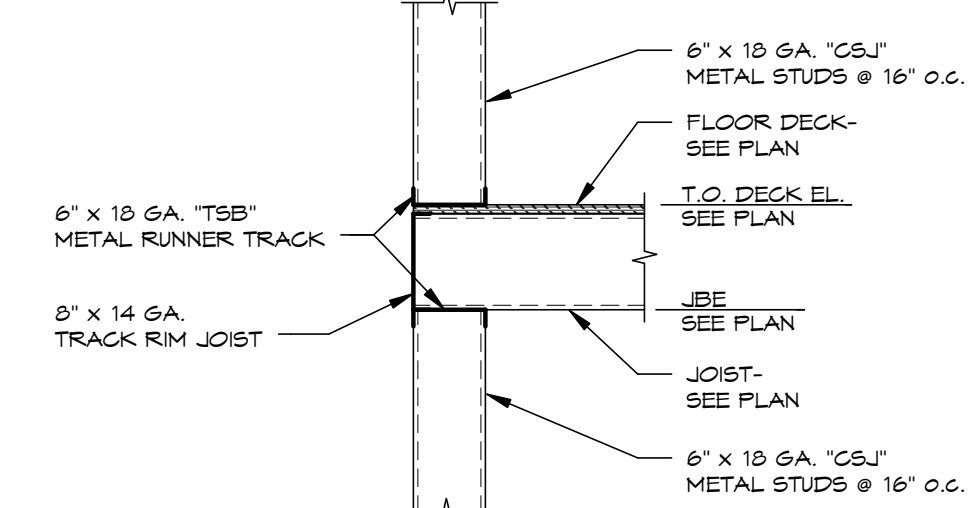
2 SECTION
3/4" = 1'-0"



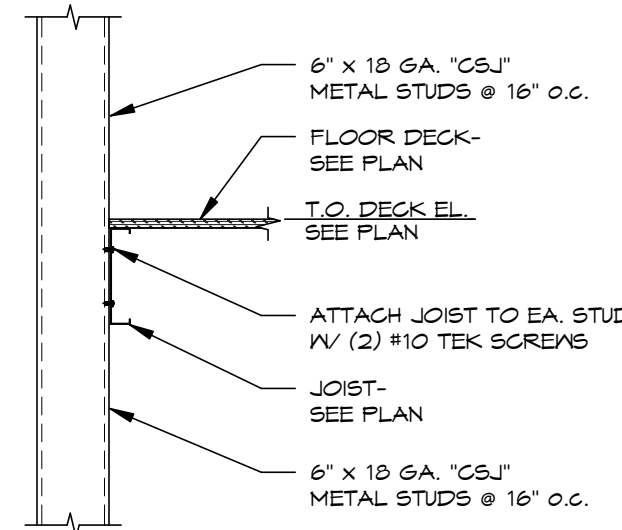
3 SECTION
3/4" = 1'-0"



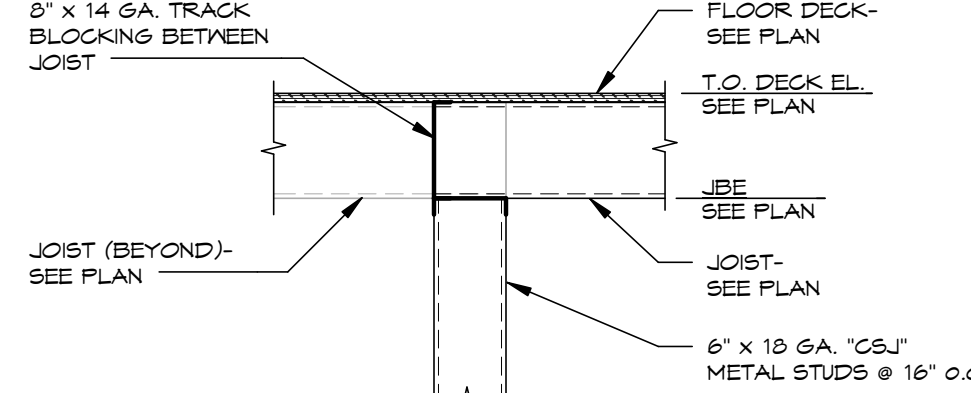
4 SECTION
3/4" = 1'-0"



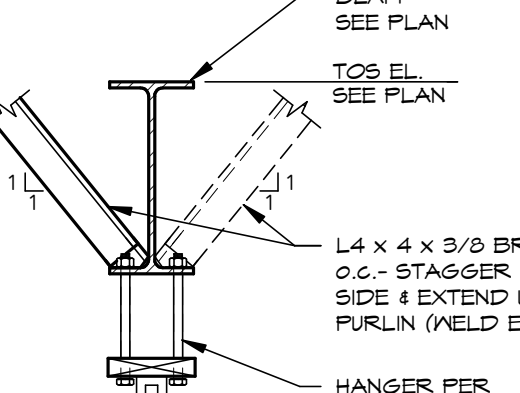
5 SECTION
3/4" = 1'-0"



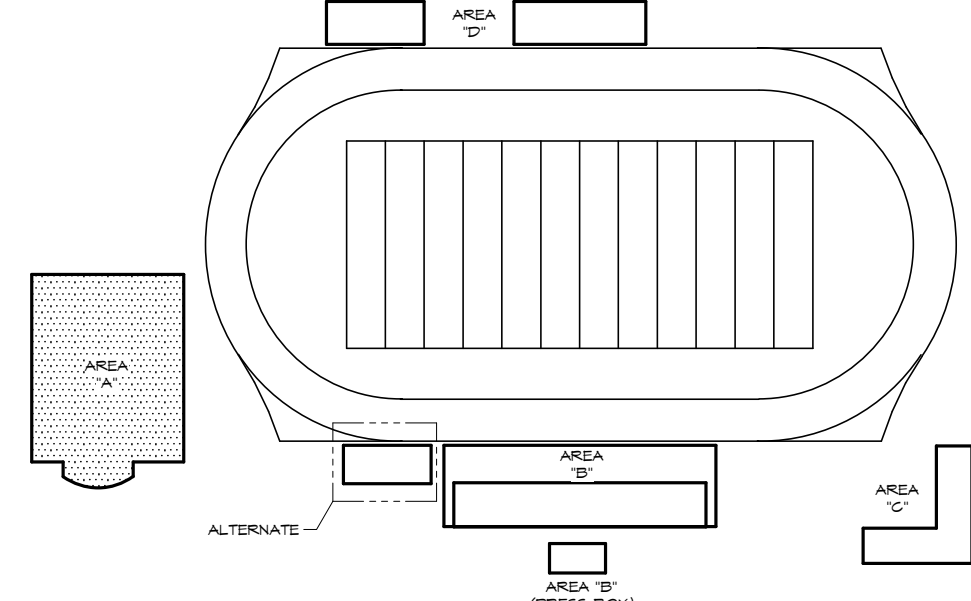
6 SECTION
3/4" = 1'-0"



7 SECTION
3/4" = 1'-0"



8 SECTION
3/4" = 1'-0"



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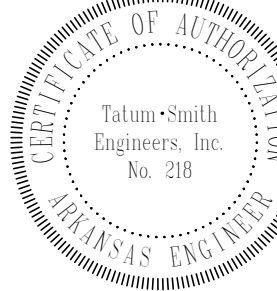
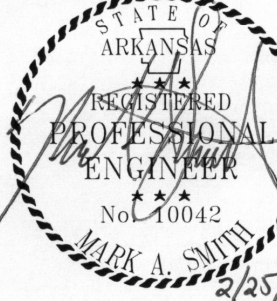
REVISION DATES

DATE DESCRIPTION

SHEET
S3.1D

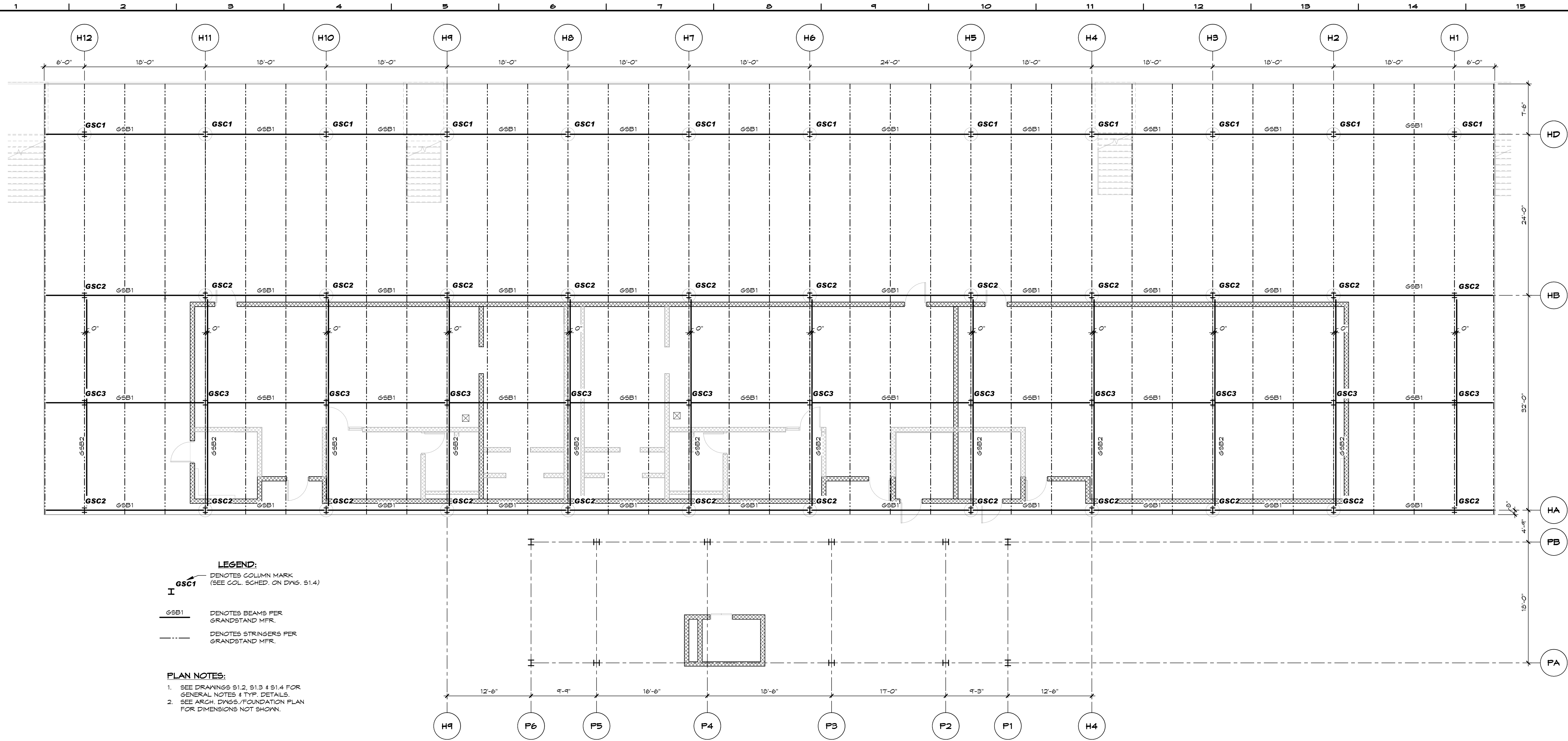
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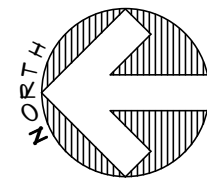


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TS Project No.: 151210
CAD Technician: EEC
Engineer of Record: MAS
Project Designer: TAT

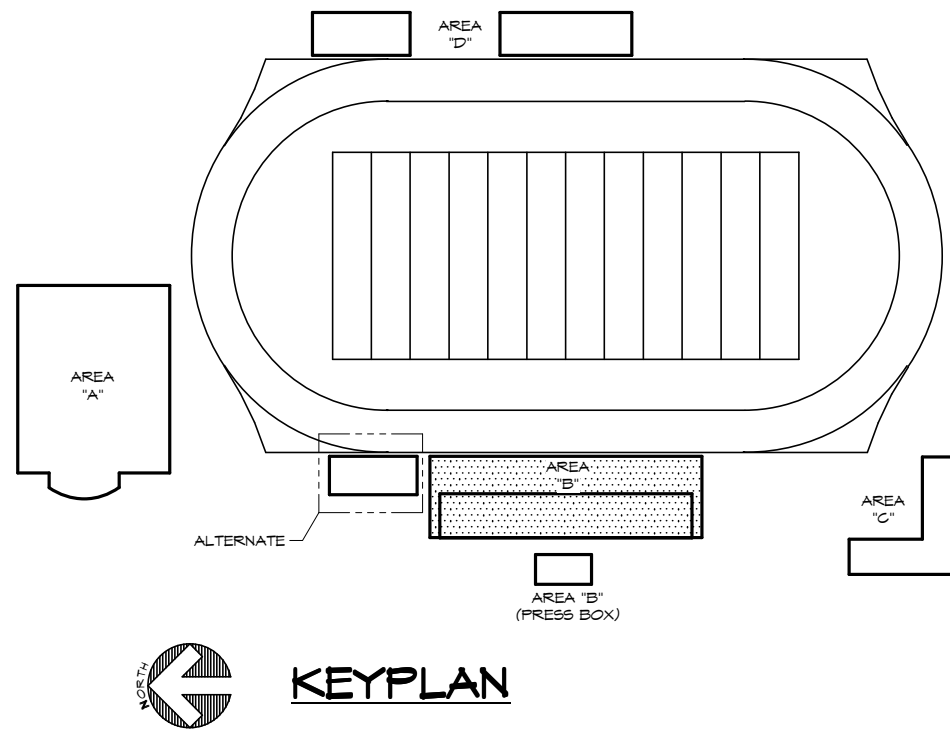


- LEGEND:**
- I** **GSC1** DENOTES COLUMN MARK
(SEE COL. SCHED. ON DNGS. S1.4)
- GSB1** DENOTES BEAMS PER
GRANDSTAND MFR.
- DENOTES STRINGERS PER
GRANDSTAND MFR.
- PLAN NOTES:**
1. SEE DRAWINGS S1.2, S1.3 & S1.4 FOR
GENERAL NOTES & TYP. DETAILS.
 2. SEE ARCH. DNGS./FOUNDATION PLAN
FOR DIMENSIONS NOT SHOWN.



1 GRANDSTAND FRAMING PLAN - AREA "B"
1/8" = 1'-0"

THIS SHEET FOR
REFERENCE ONLY



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Structural Engineering
479.621.6128 Rogers, Arkansas

STATE OF ARKANSAS
Tatum Smith
Engineers, Inc.
No. 218
EXPIRATION DATE 12/31/16

ARKANSAS
REGISTERED
PROFESSIONAL
ENGINEER
No. 10042
WILLIAM A. SMITH
2/25/16

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CARTHAGE STADIUM COMPLEX
FIR RD & RIVER ST., CARTHAGE, MO

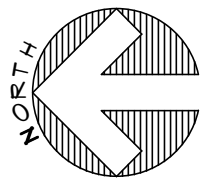
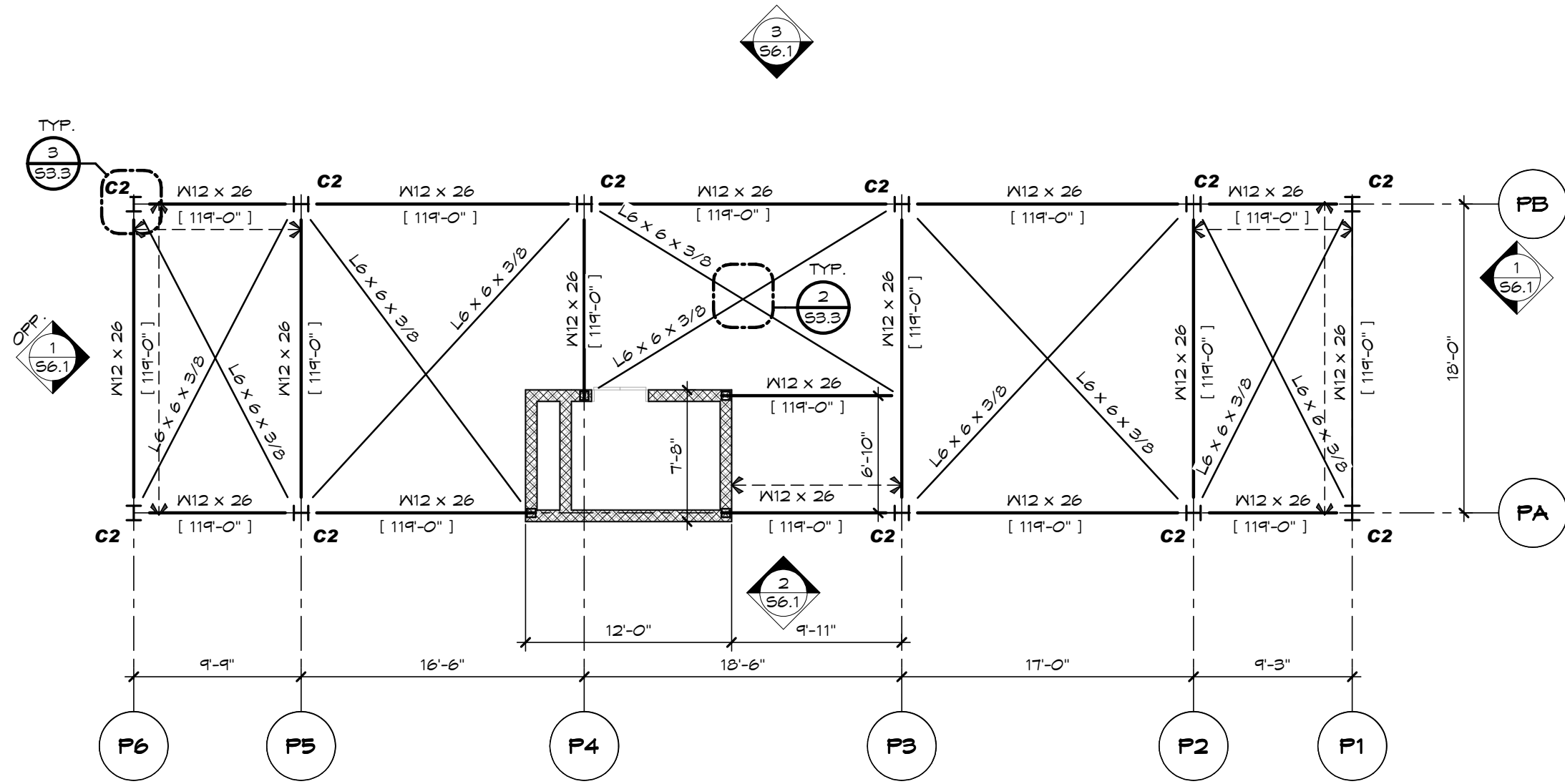
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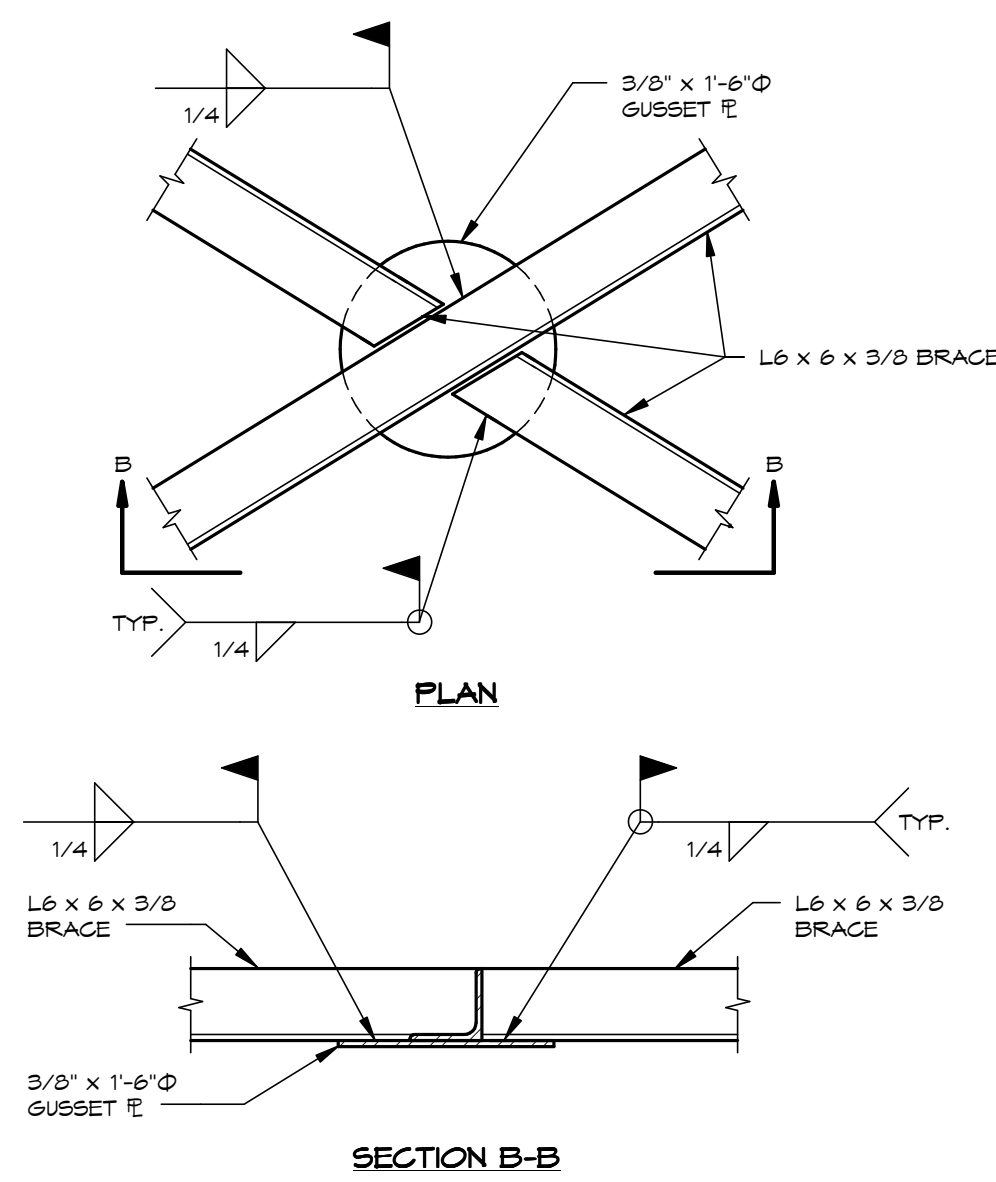
TS Project No.: 151210
CAD Technician: EEC
Engineer of Record: MAS
Project Designer: TAT



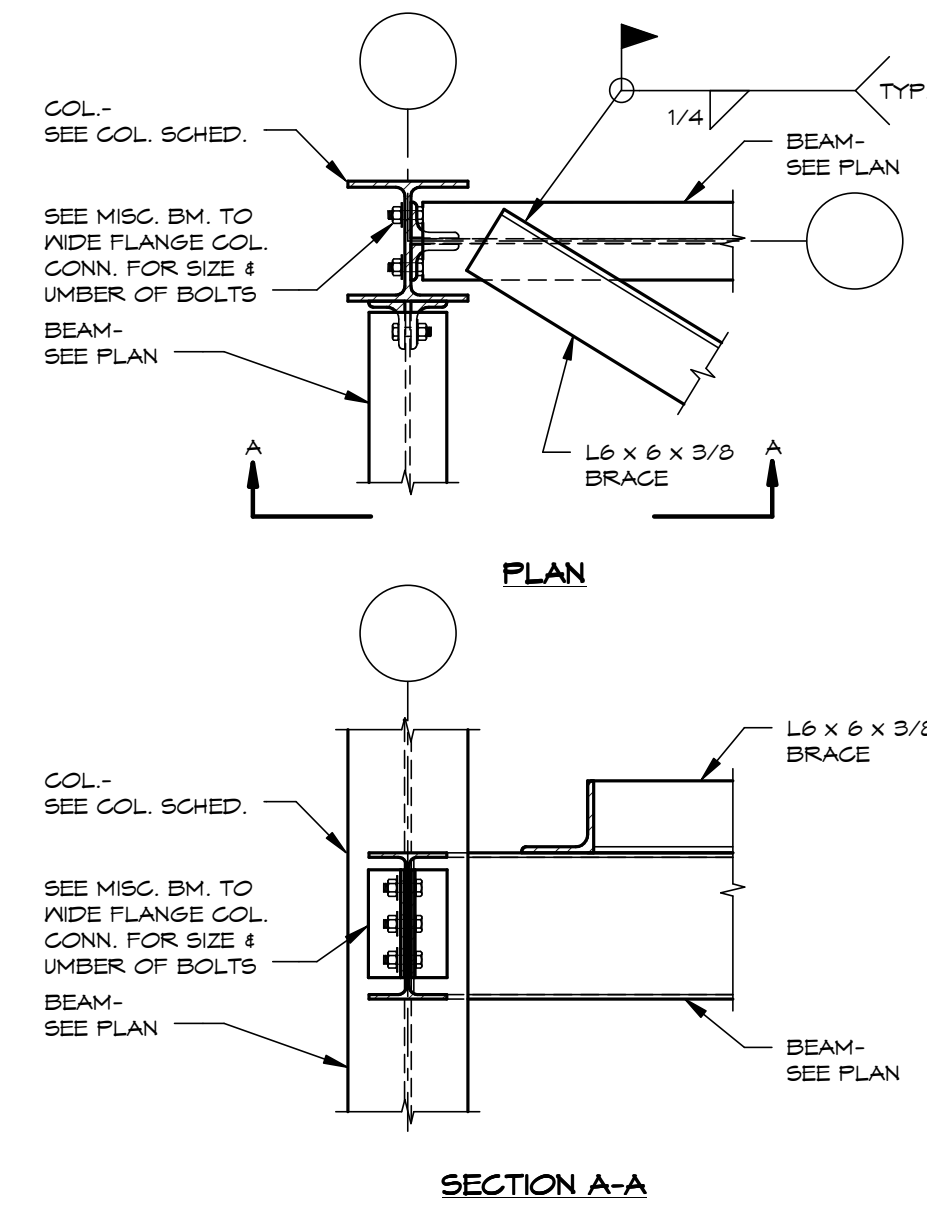
1 PRESS BOX BRACING PLAN 1/8" = 1'-0"

- LEGEND:**
- DENOTES COLUMN MARK (SEE COL. SCHED. ON DWS. 51.4)
 - DENOTES TOP OF BEAM ELEVATION
 - DENOTES LOAD-BEARING CMU WALL
 - DENOTES BEAM BEARING PLATE (SEE DTL. 6/53.0)
 - DENOTES DIAGONAL BRACING (SEE FRMS. ELEVATIONS ON 56.1 FOR LOCATION & SIZE)
 - DENOTES DETAIL REFERENCE ON CORRESPONDING DWS. (SOLID HATCH DENOTES DIRECTIONAL VIEW)

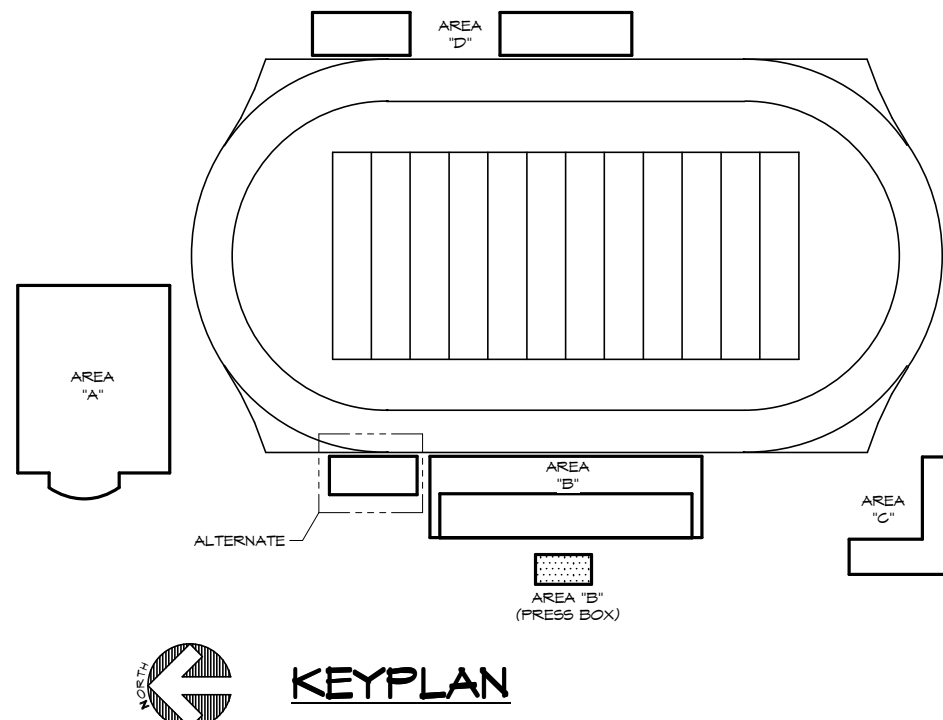
- PLAN NOTES:**
- TOP OF STEEL DENOTES TOP OF MAIN STEEL.
 - TOP OF STEEL EL. VARIES. SEE PLAN.
 - ALL ELEVATIONS BASED ON FINISH FLOOR EL. 100'-0". ACTUAL FIN. FLR. EL. 1073.00' AS ESTABLISHED BY GRAY ROCK CONSULTING INC..
 - SEE DWS. 51.2, 51.3, & 51.4 FOR GENERAL NOTES & TYPICAL DETAILS
 - SEE DWS. 51.4 FOR COLUMN SCHEDULE.
 - SEE ARCH. DWS. & FOUNDATION PLANS FOR DIMENSIONS NOT SHOWN.
 - SEE DWS. 53.0.1, 53.0.2 & 53.0.3 FOR LINTEL & MASONRY PLANS.



2 TYP. BRACING PLAN DETAIL 3/4" = 1'-0"



3 TYP. BRACING PLAN DETAIL 3/4" = 1'-0"



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CARTHAGE R-9 SCHOOL DISTRICT CARTHAGE STADIUM COMPLEX FIR RD & RIVER ST, CARTHAGE, MO

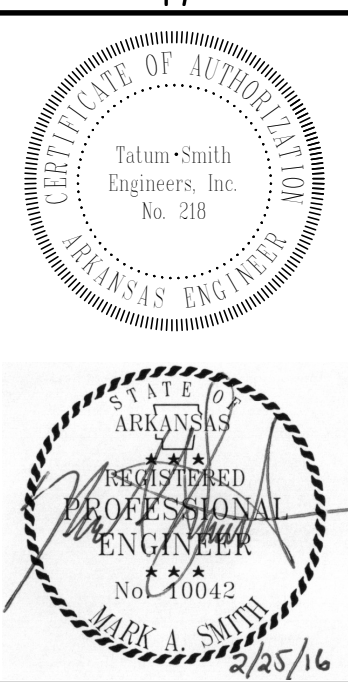
DRAWN BY: EEC
CHECK BY: MAS
ISSUE DATE: 02/29/2016

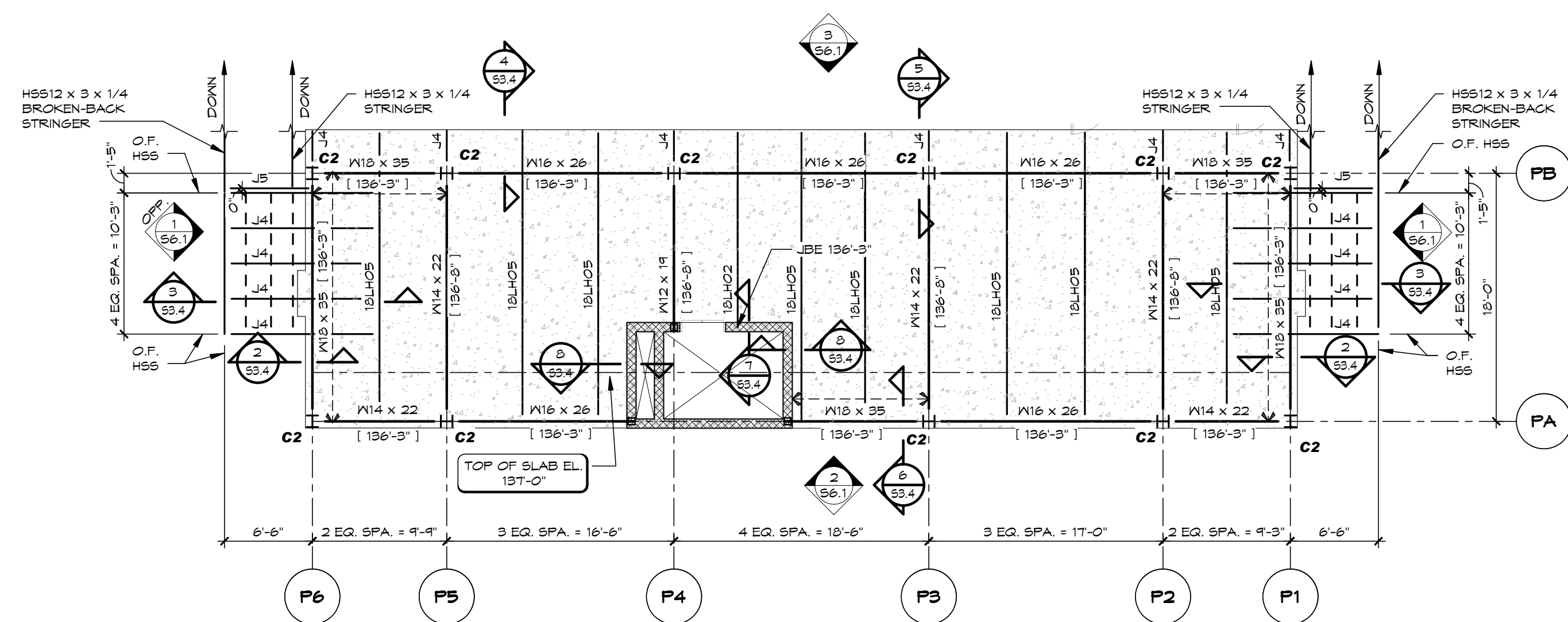
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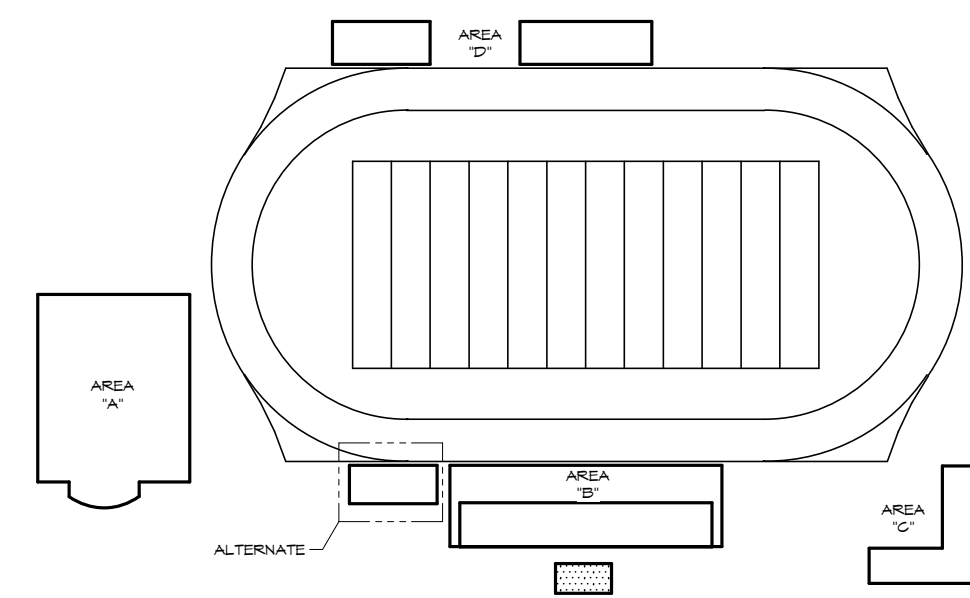
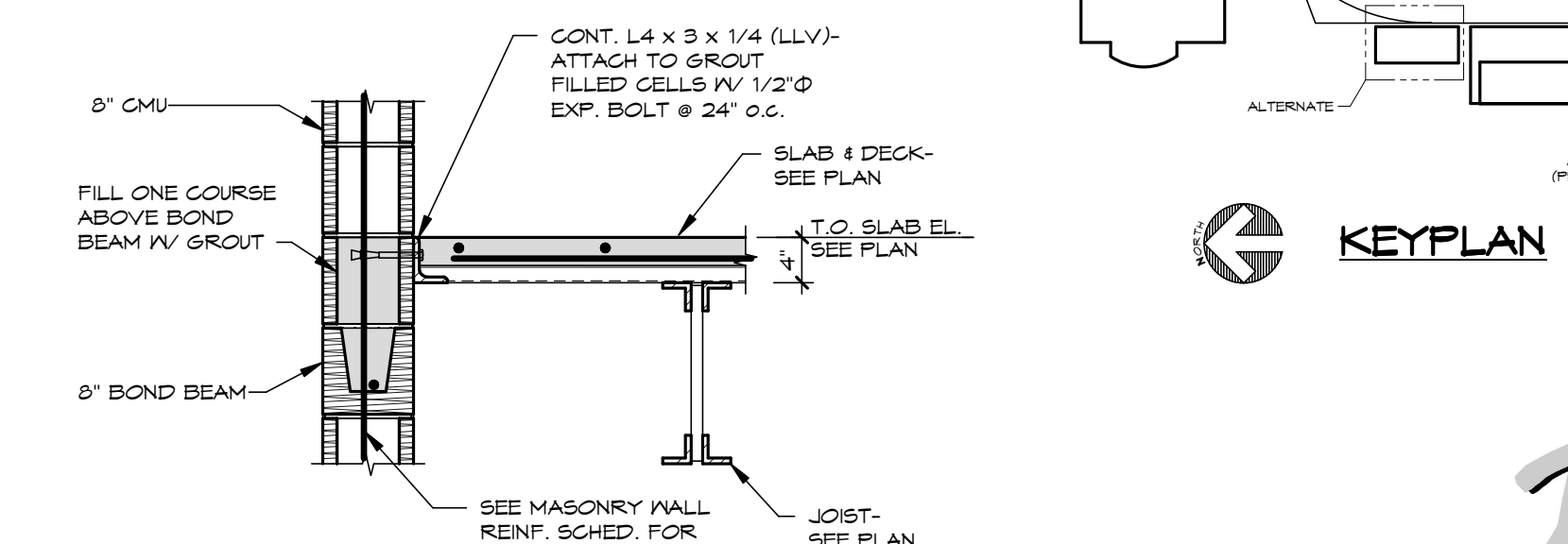
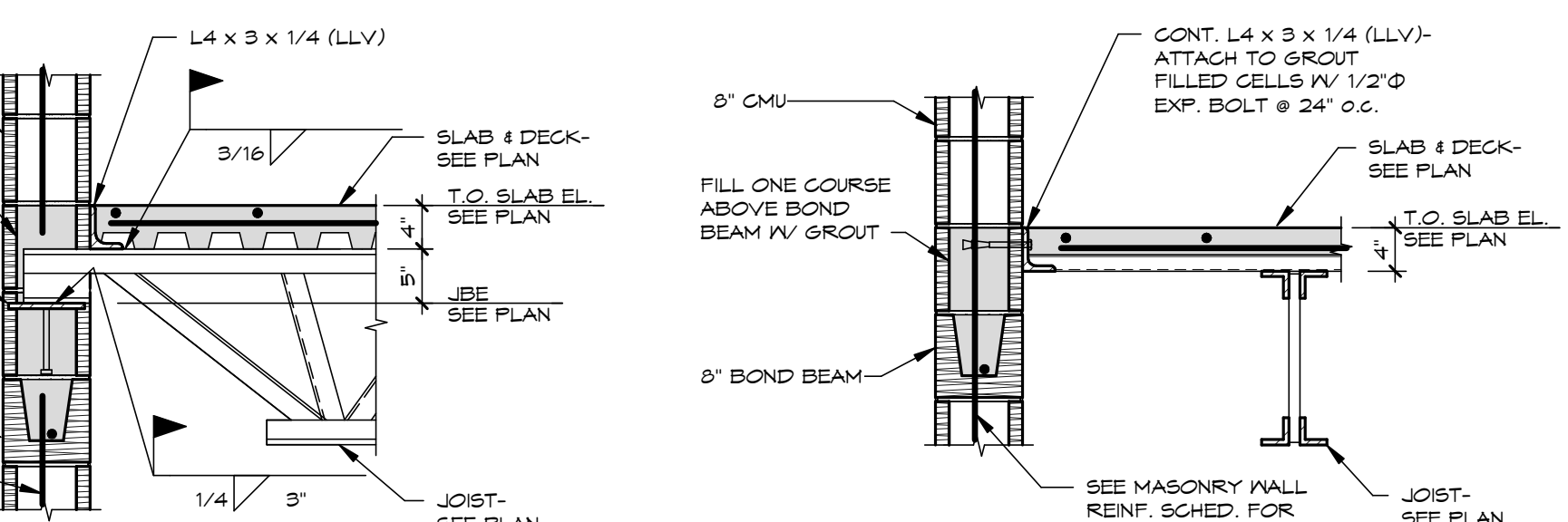
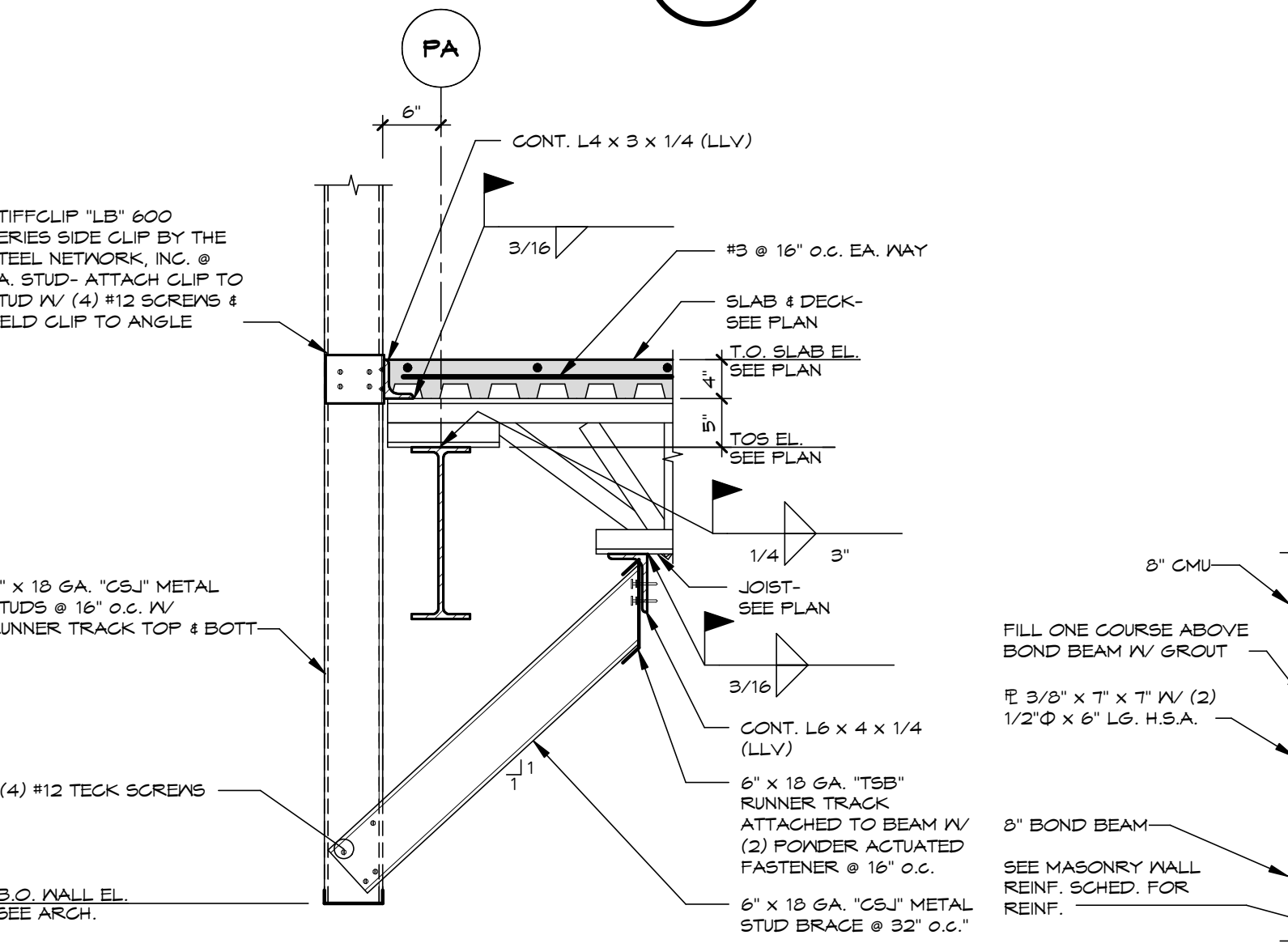
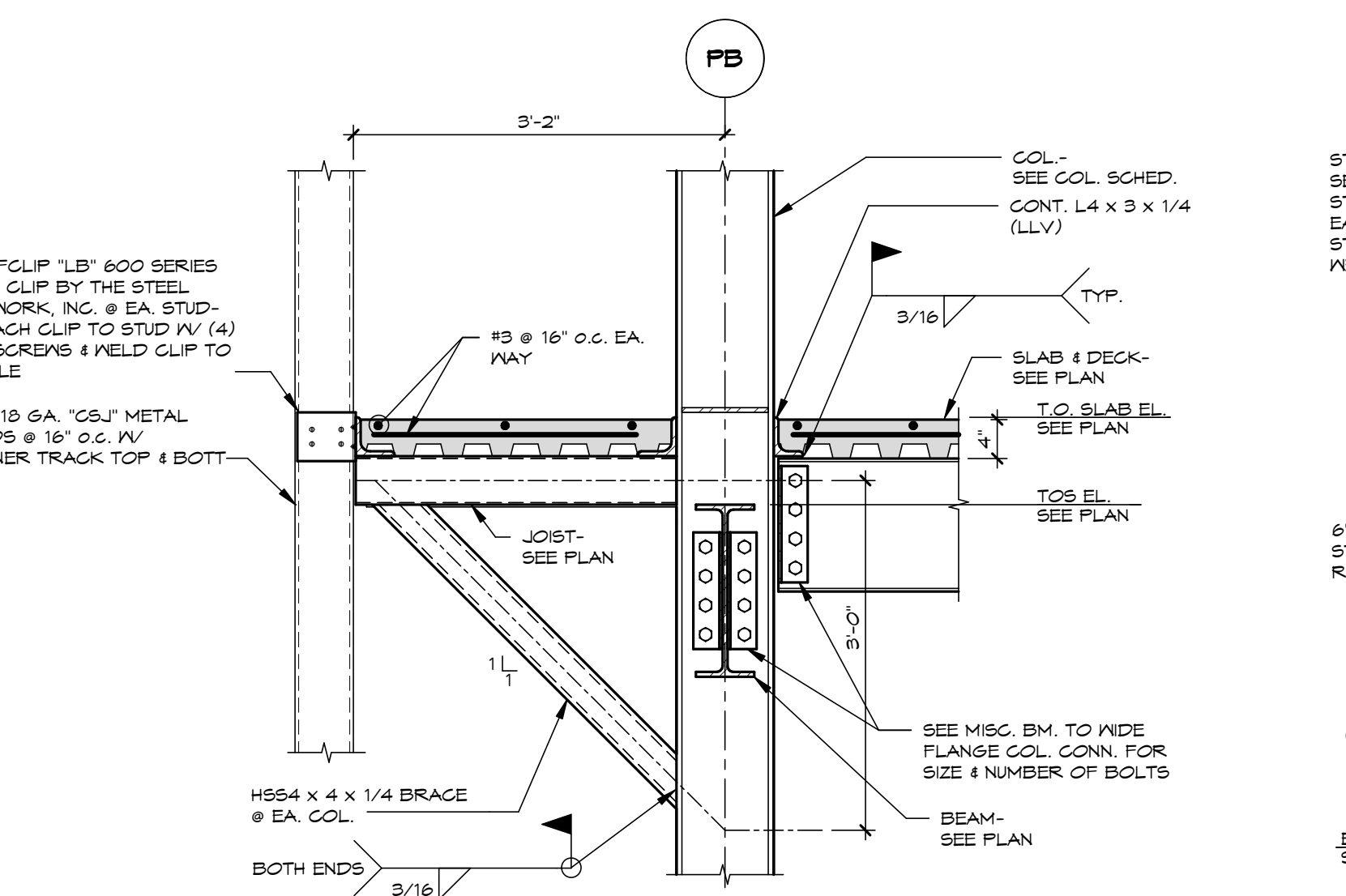
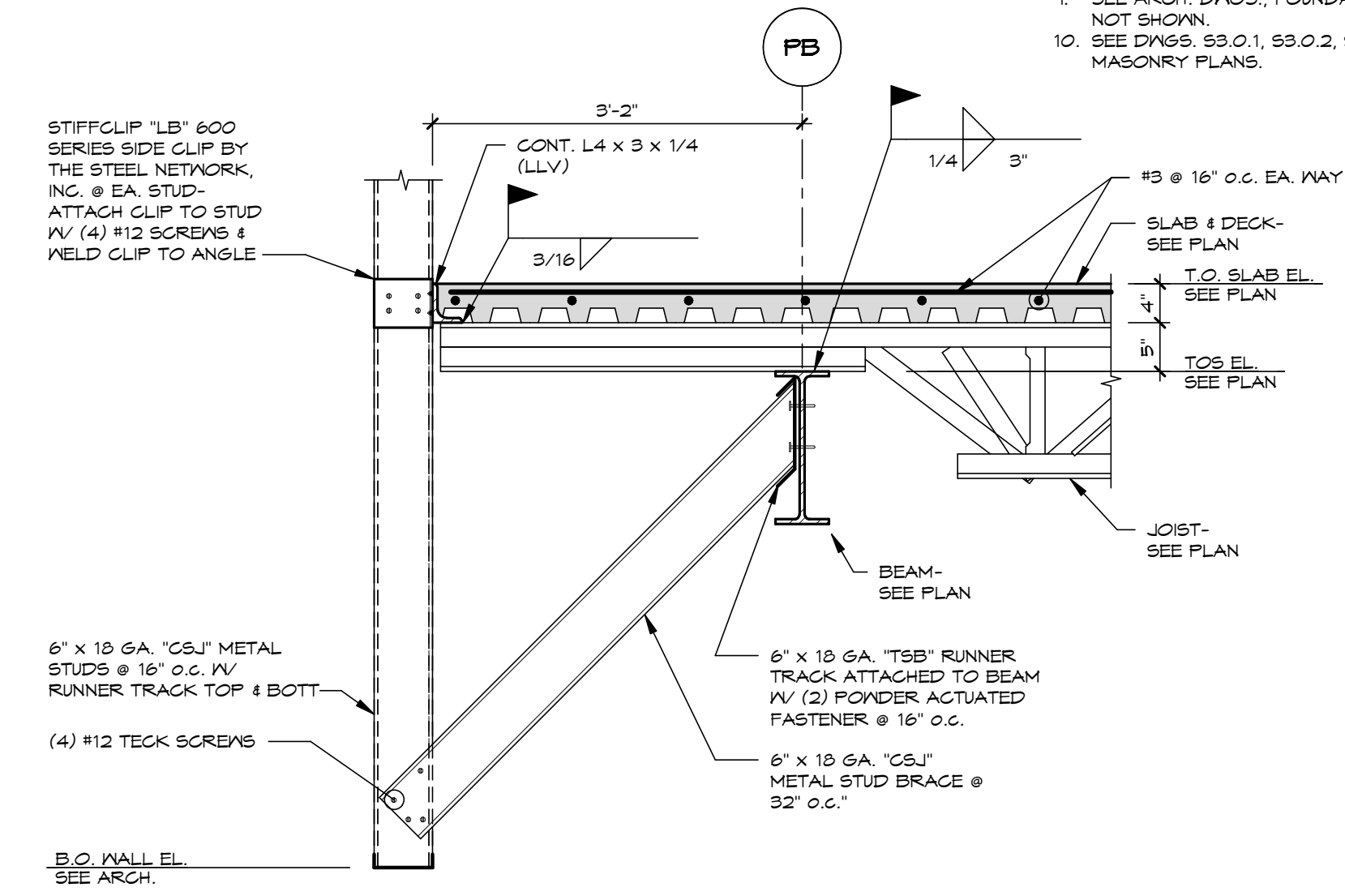
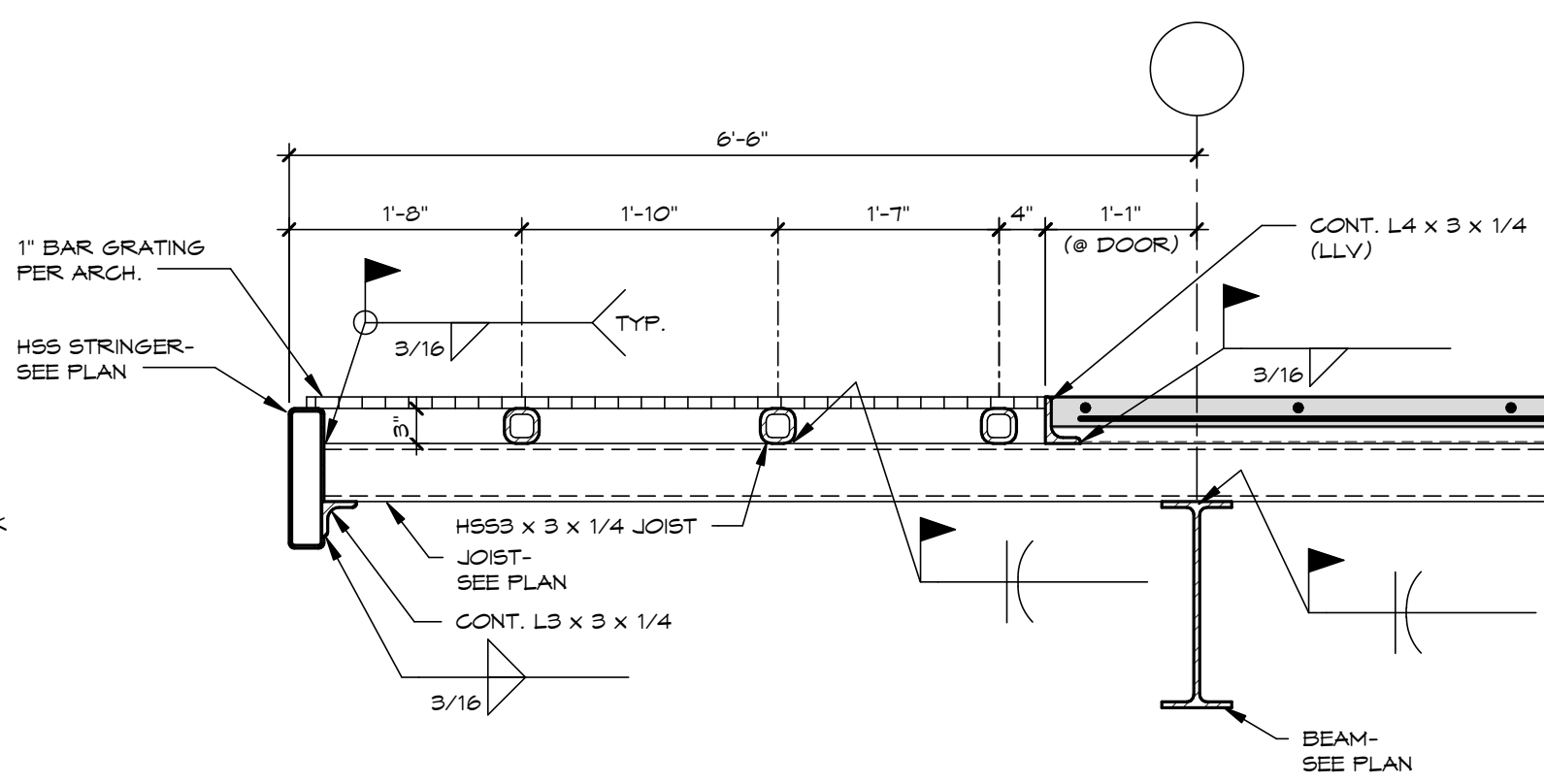
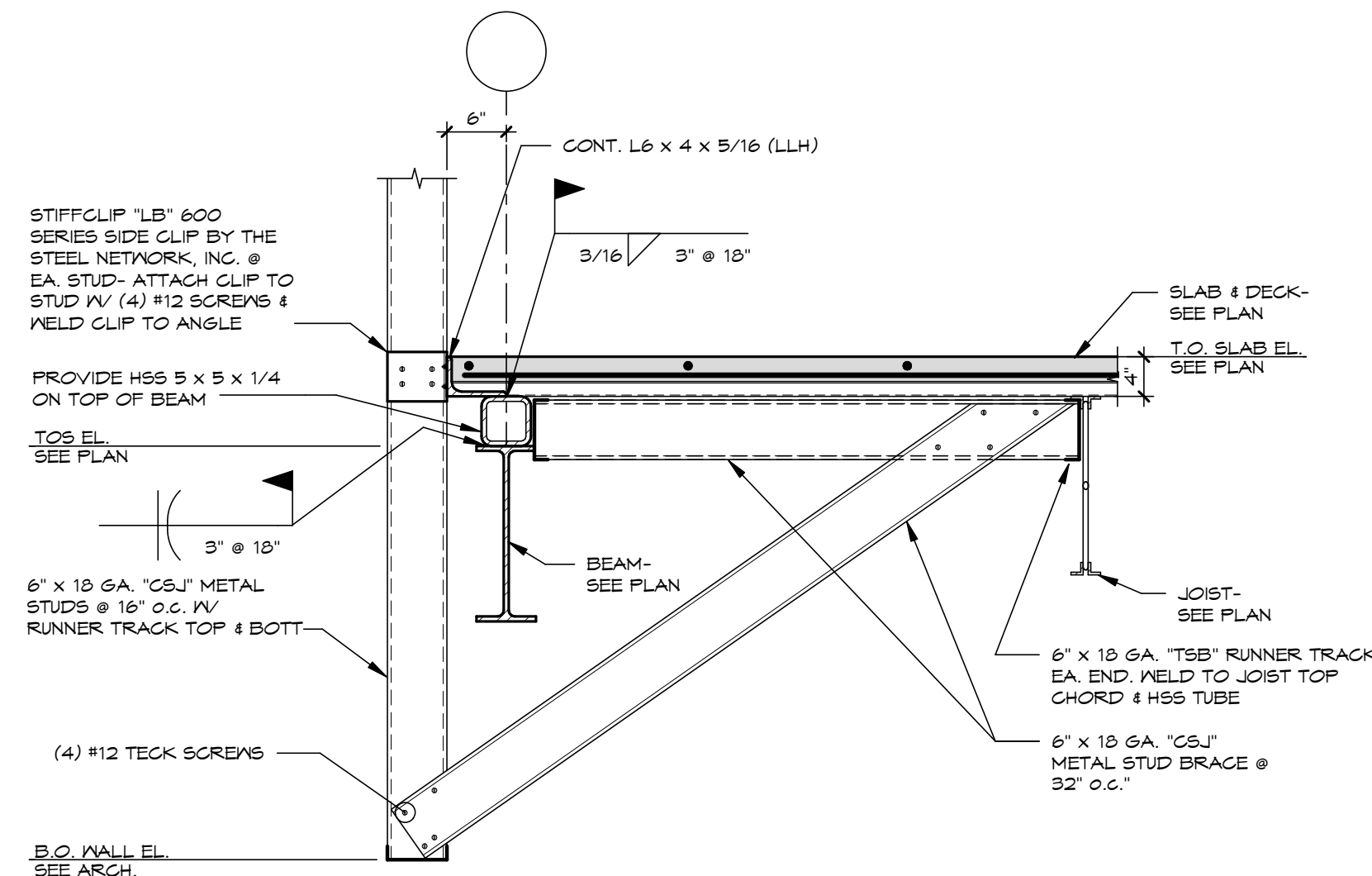
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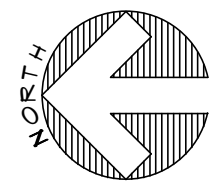
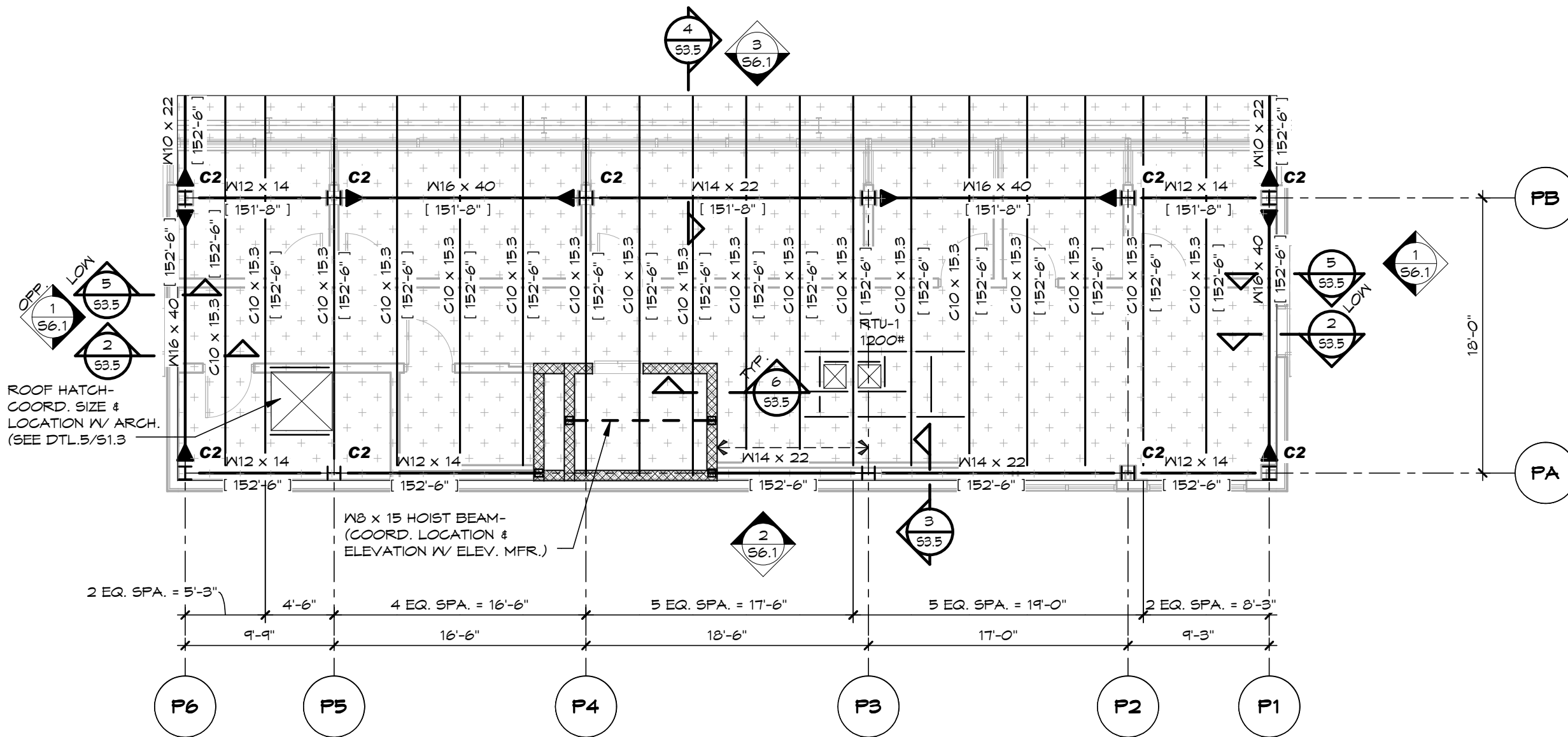
1 PRESS BOX FLOOR FRAMING PLAN
1/8" = 1'-0"

- LEGEND:**
- 1 C1 DENOTES COLUMN MARK (SEE COL. SCHED. ON DWS. 51.4)
 - [135'-0"] DENOTES TOP OF BEAM ELEVATION
 - [118'-0"] DENOTES TOP OF SLAB EL.
 - 118'-0" DENOTES DETAIL REFERENCE ON CORRESPONDING DWS. (SOLID HATCH DENOTES DIRECTIONAL VIEW)
 - 118'-0" DENOTES LIMITS OF ELEVATED 4" CONG. SLAB (SEE PLAN NOTES) (T.O. SLAB EL. SEE PLAN)
 - 118'-0" DENOTES LIMITS OF FLOOR OPENING (SEE DTL. 3/51.3)
 - 118'-0" DENOTES DIAGONAL BRACING (SEE FRMS. ELEVATIONS ON 56.1 FOR LOCATION & SIZE)
 - 118'-0" DENOTES HSS5 x 3 x 1/4 JOIST (TOS EL. 136'-11")
 - J4 DENOTES HSS5 x 5 x 1/4 JOIST (TOS EL. 136'-8")
 - J5 DENOTES DBL. HSS5 x 5 x 1/4 JOIST (TOS EL. 136'-8")
- PLAN NOTES:**
- ELEVATED SLAB CONSTRUCTION: 4" NORMAL WEIGHT CONCRETE SLAB REINFORCED W/ #3 @ 16" O.C. EACH WAY ON 1 1/2" DEEP, 22 GA. GALVANIZED VUL. COMPOSITE STEEL DECK. PLACE REINFORCING 1 1/2" CLEAR FROM TOP OF SLAB. (SEE STEEL DECK GENERAL NOTES FOR ATTACHMENT REQUIREMENTS.)
 - TOP OF STEEL EL. VARIES, SEE PLAN.
 - TOP OF ELEVATED SLAB EL. SEE PLAN.
 - TOP OF "LH" JOIST EL. (18")
 - CONTRACTOR SHALL SAWCUT 1/8" x 1" DEEP JOINTS @ 16'-0" O.C. MAX. CONTRACTOR SHALL SUBMIT JOINT PLAN TO ARCHITECT FOR APPROVAL PRIOR TO THE ELEVATED SLAB CONSTRUCTION.
 - ALL ELEVATIONS BASED ON FINISH FLOOR EL. 100'-0". ACTUAL FIN. FLR. EL. 1073.00' AS ESTABLISHED BY GRAY ROCK CONSULTING INC.
 - SEE DWS. 51.4 FOR COLUMN SCHEDULE.
 - SEE ARCH. DWGS., FOUNDATION PLANS FOR DIMENSIONS NOT SHOWN.
 - SEE DWS. 53.0.1, 53.0.2, 53.0.3 FOR LINTEL & MASONRY PLANS.

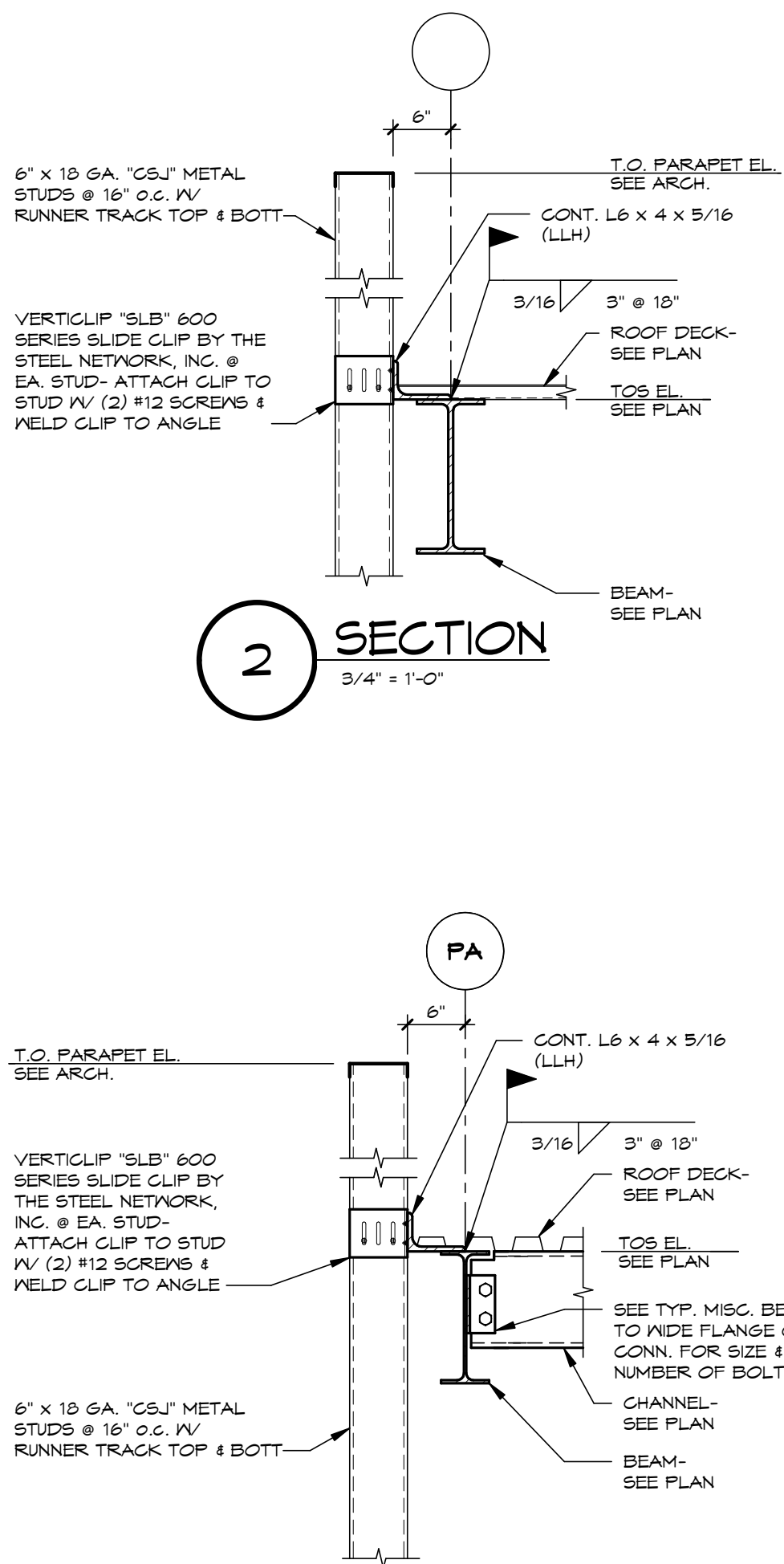


KEYPLAN

Tatum Smith
Engineers, Inc.
Structural Engineering
479.621.6128
Rogers, Arkansas

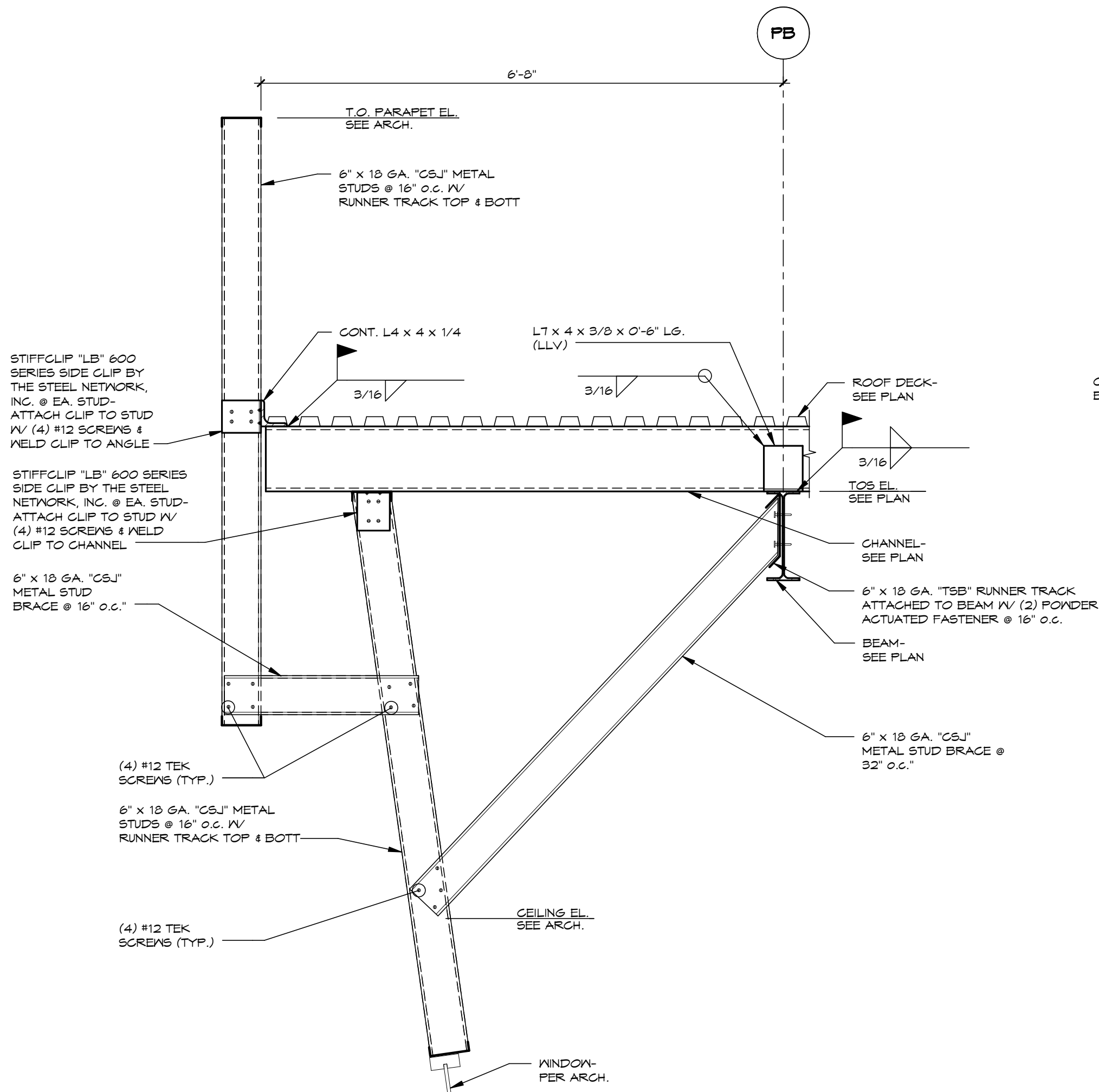


1 PRESS BOX ROOF FRAMING PLAN
1/8" = 1'-0"

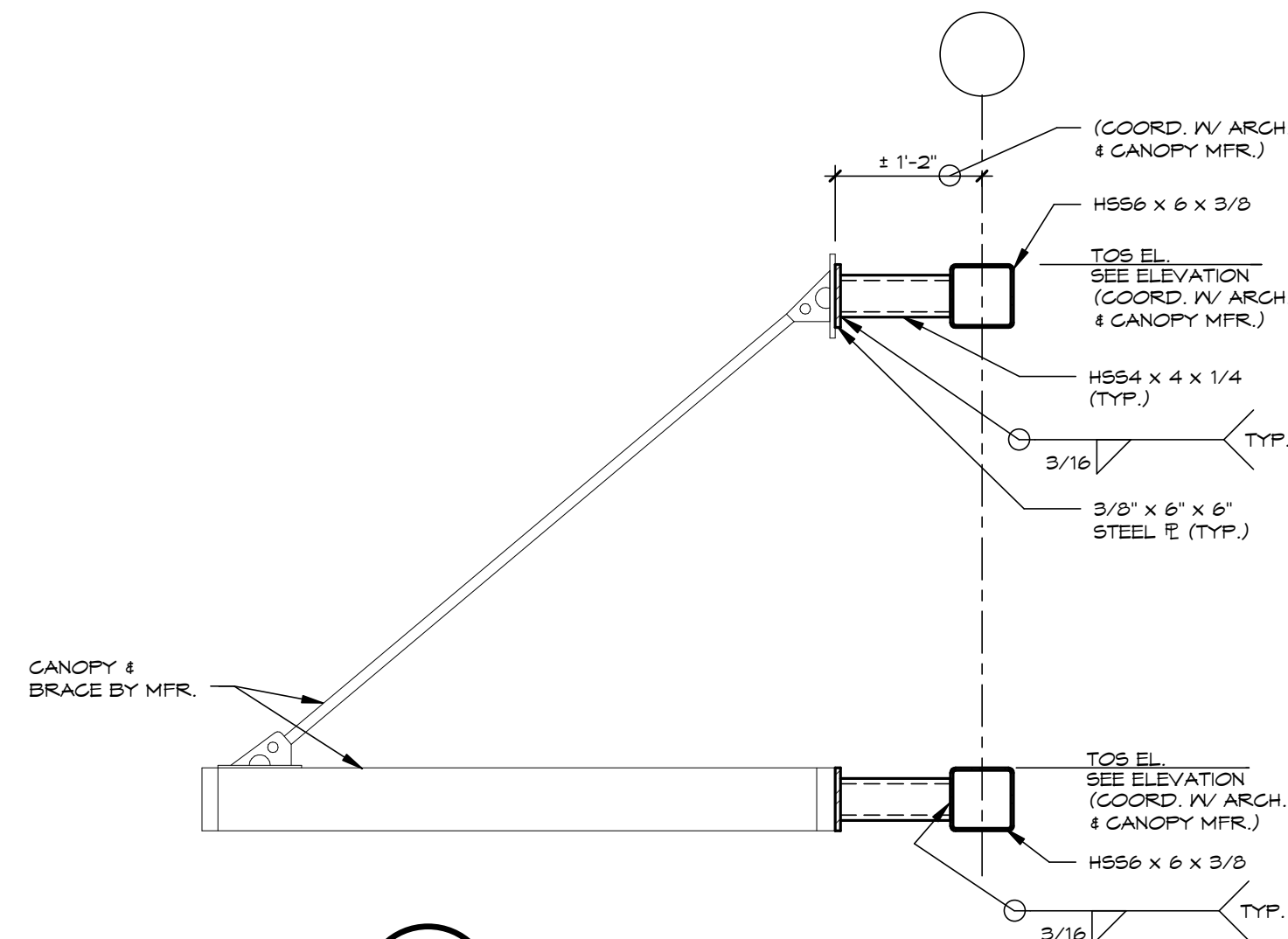


2 SECTION
3/4" = 1'-0"

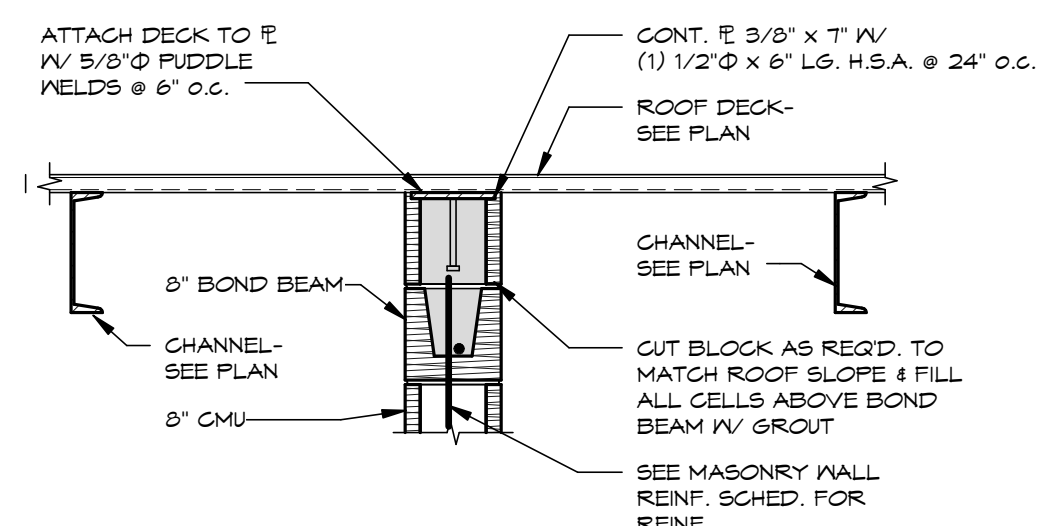
3 SECTION
3/4" = 1'-0"



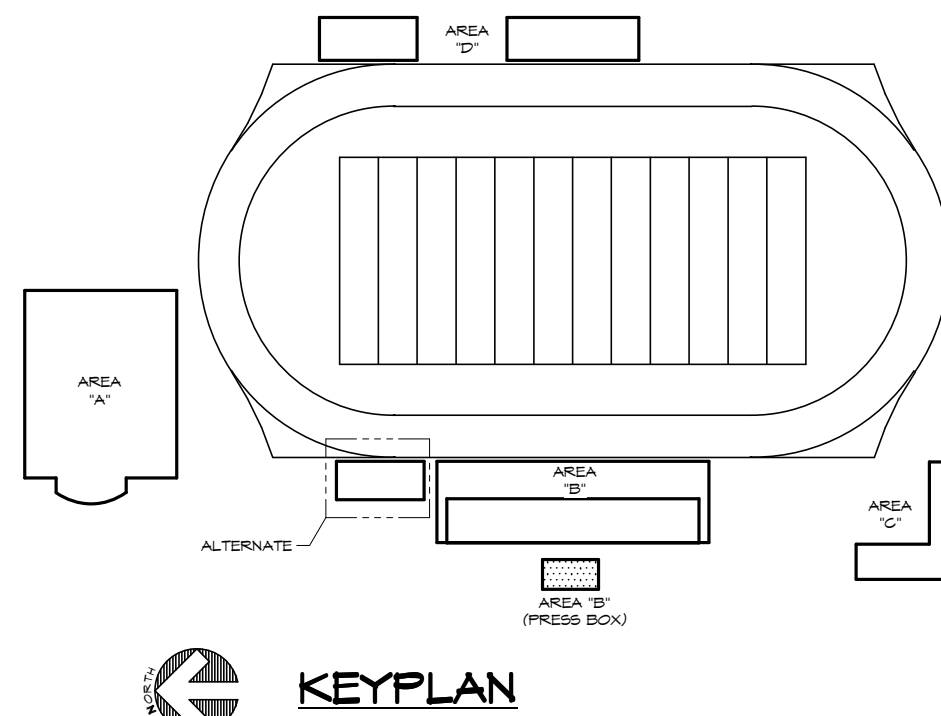
4 SECTION
3/4" = 1'-0"



5 SECTION
3/4" = 1'-0"



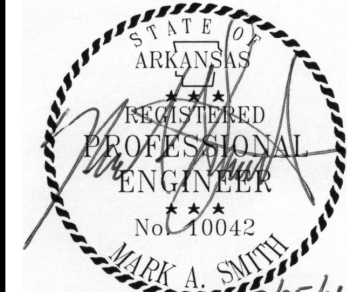
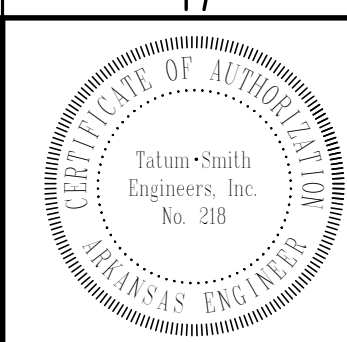
6 SECTION
3/4" = 1'-0"



KEYPLAN

- LEGEND:**
- 1 C1 DENOTES COLUMN MARK (SEE COL. SCHED. ON DWS. S1.4)
 - [148'-2"] DENOTES TOP OF BEAM ELEVATION
 - [152'-6"] DENOTES LOAD-BEARING CMU WALL
 - [151'-8"] DENOTES BEAM BEARING PLATE (SEE DTL. 6/S3.0)
 - [152'-6"] DENOTES LIMITS OF 1 1/2", 18 GA., ROOF DECK (SEE PLAN NOTES)
 - [152'-6"] DENOTES MOMENT CONNECTION (SEE DTL. 8/S1.3)
 - [152'-6"] DENOTES DIAGONAL BRACING (SEE FRAM. ELEVATIONS ON S6.1 FOR LOCATION & SIZE)
 - [152'-6"] DENOTES DETAIL REFERENCE ON CORRESPONDING DWS. (SOLID HATCH DENOTES DIRECTIONAL VIEW)
 - [152'-6"] DENOTES FRAMED OPENING IN ROOF. COORDINATE SIZE & LOCATION IV MECH. CONTRACTOR (SEE DTL. 4/S1.3)

- PLAN NOTES:**
1. PRESS BOX ROOF DECK: 1 1/2" DEEP, 18 GA. PAINTED WIDE RIB STEEL DECK (SEE STEEL DECK GENERAL NOTES FOR ATTACHMENT REQUIREMENTS).
 2. TOP OF STEEL DENOTES TOP OF MAIN STEEL.
 3. TOP OF STEEL EL. VARIES, SEE PLAN.
 4. JOIST BEARING ELEVATION VARIES, SEE PLAN.
 5. ALL ELEVATIONS BASED ON FINISH FLOOR EL. 100'-0". ACTUAL FIN. FLR. EL. 101.00' AS ESTABLISHED BY GRAY ROCK CONSULTING INC.
 6. SEE DWS. S1.2, S1.3, & S1.4 FOR GENERAL NOTES & TYPICAL DETAILS.
 7. SEE DWS. S1.4 FOR COLUMN SCHEDULE.
 8. SEE ARCH. DWS. & FOUNDATION PLANS FOR DIMENSIONS NOT SHOWN.
 9. SEE DWS. S3.0.1, S3.0.2 & S3.0.3 FOR LINTEL & MASONRY PLANS.



Hight Jackson ASSOCIATES
5201 Village Parkway, Suite 300 • Rogers, Arkansas 72758 • (479) 464-4865 • www.hjarch.com

CARTHAGE R-9 SCHOOL DISTRICT
CARTHAGE STADIUM COMPLEX
FIR RD & RIVER ST., CARTHAGE, MO

DRAWN BY:
EEC
CHECK BY:
MAS
ISSUE DATE:
02/29/2016

PROJECT NO.
1365

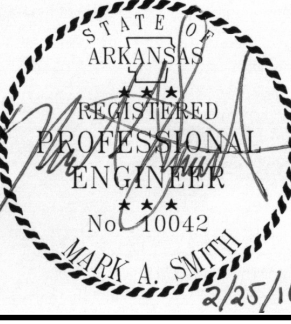
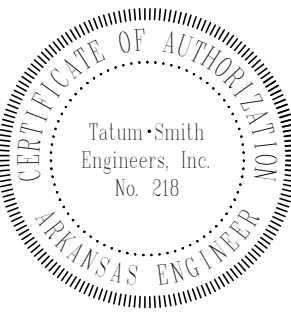
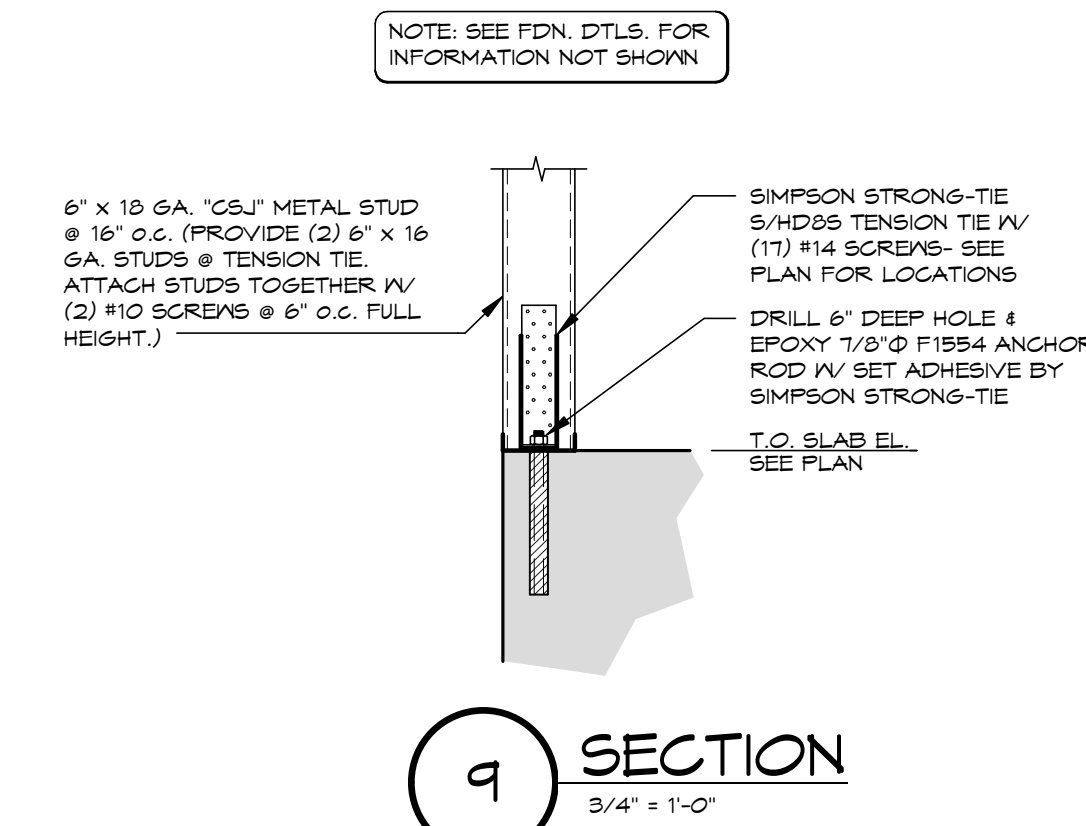
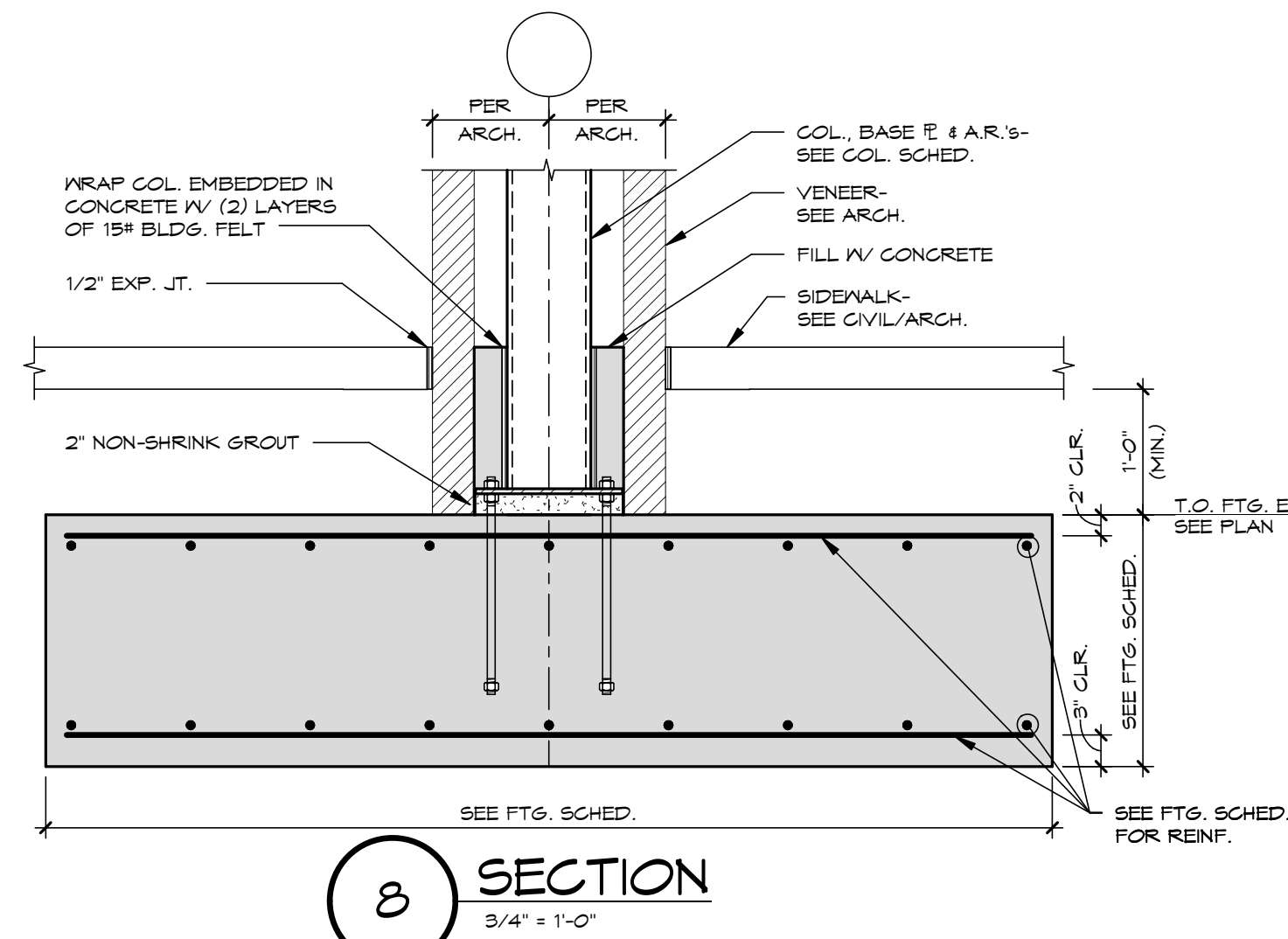
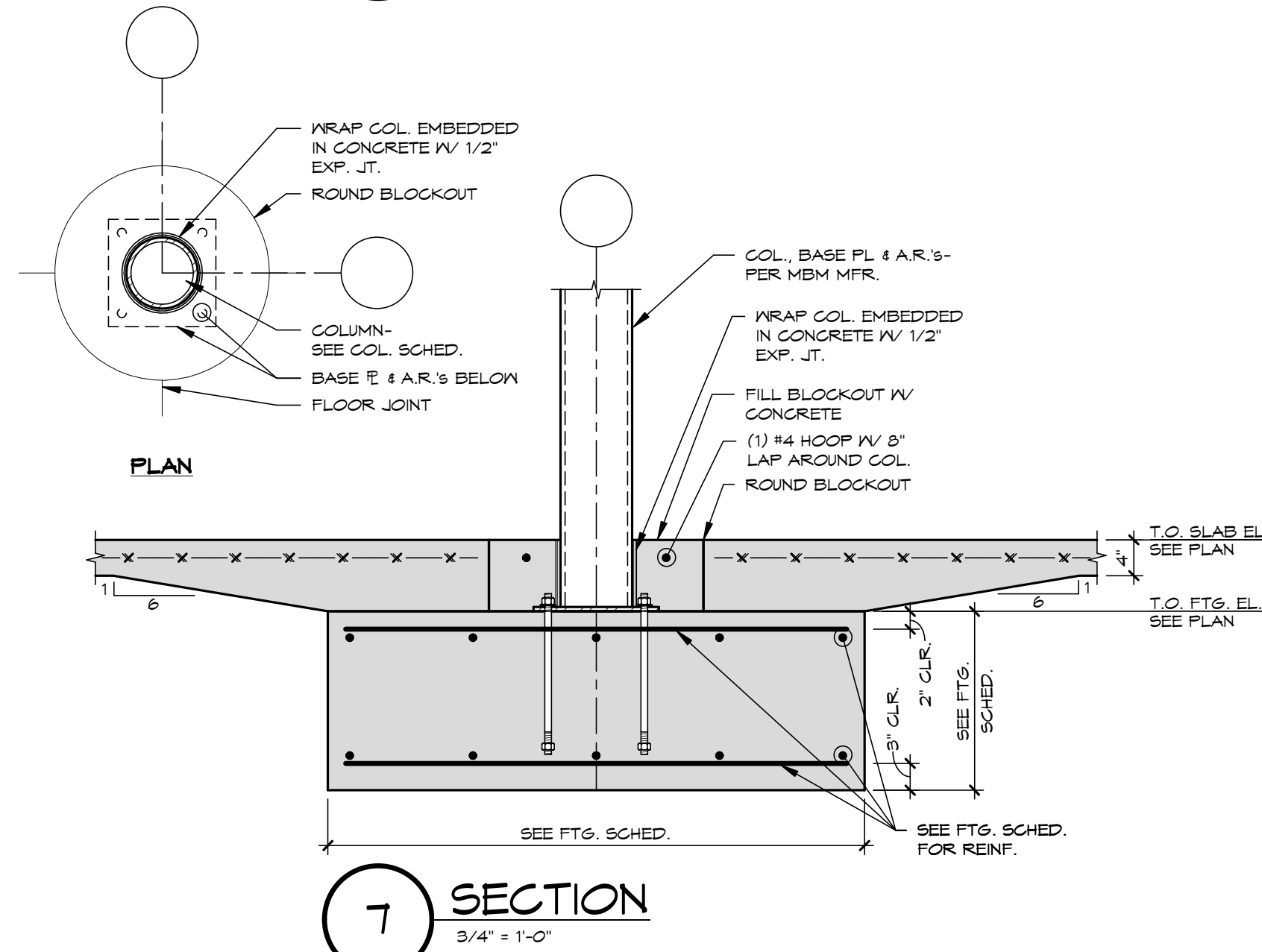
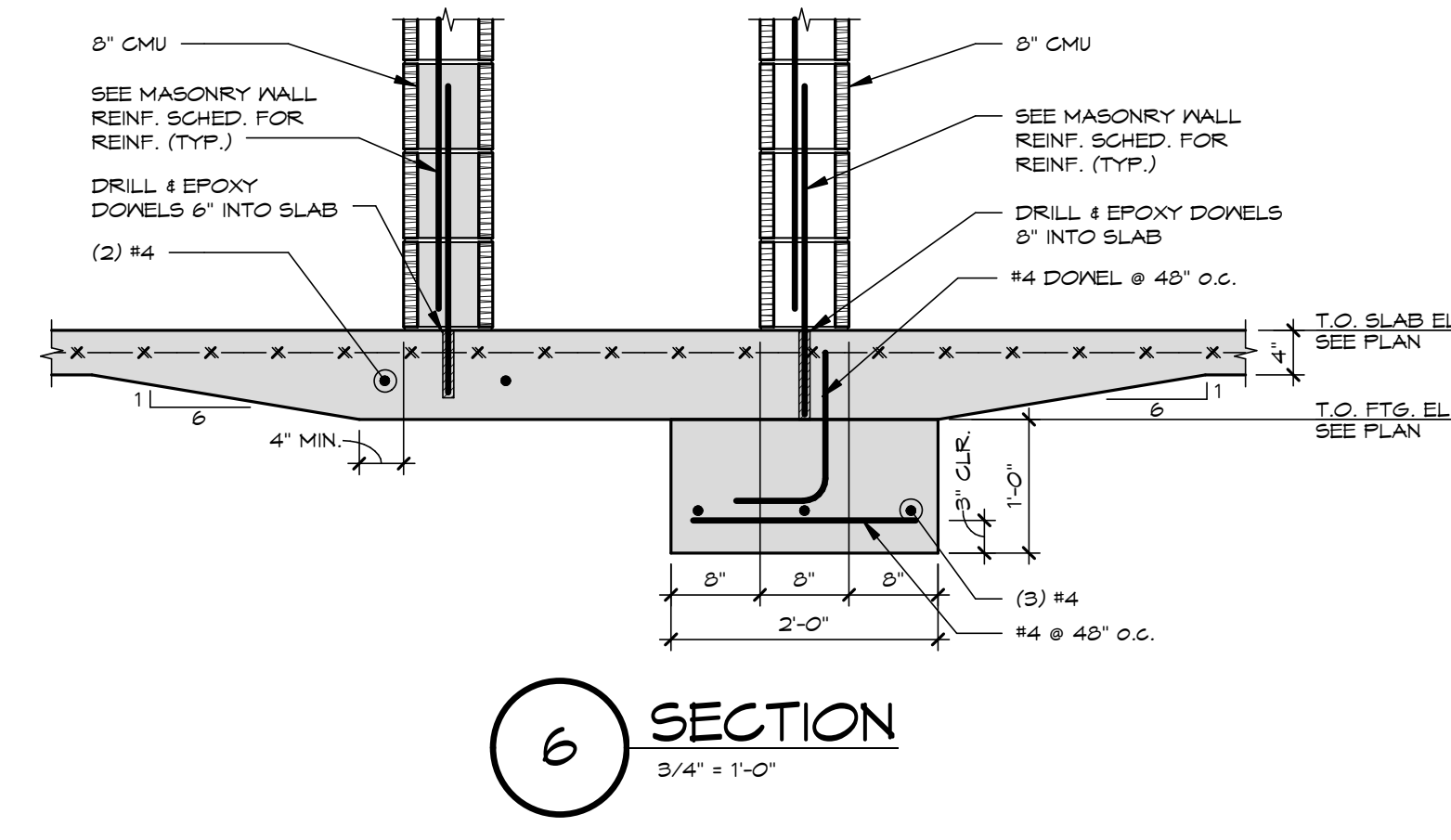
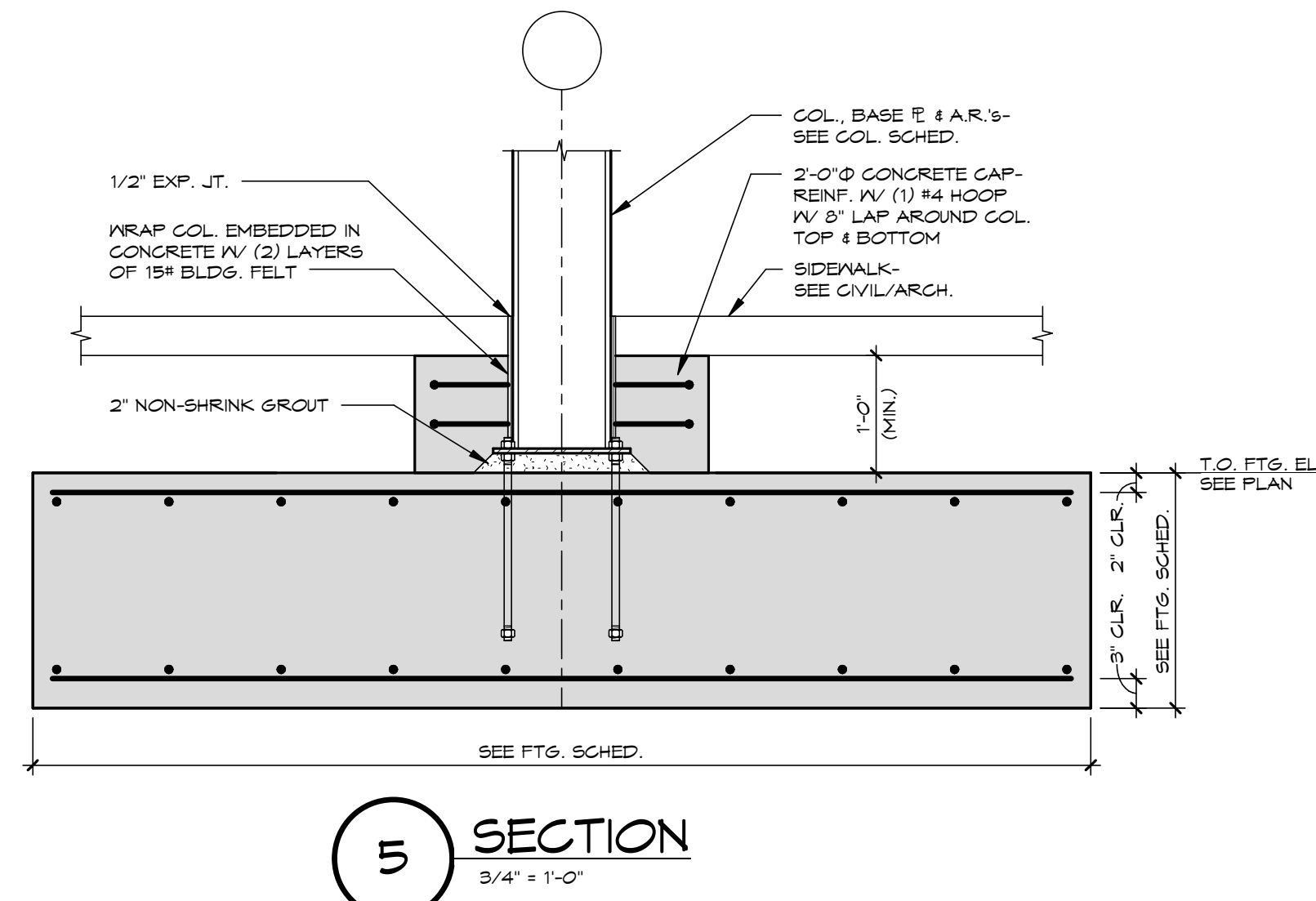
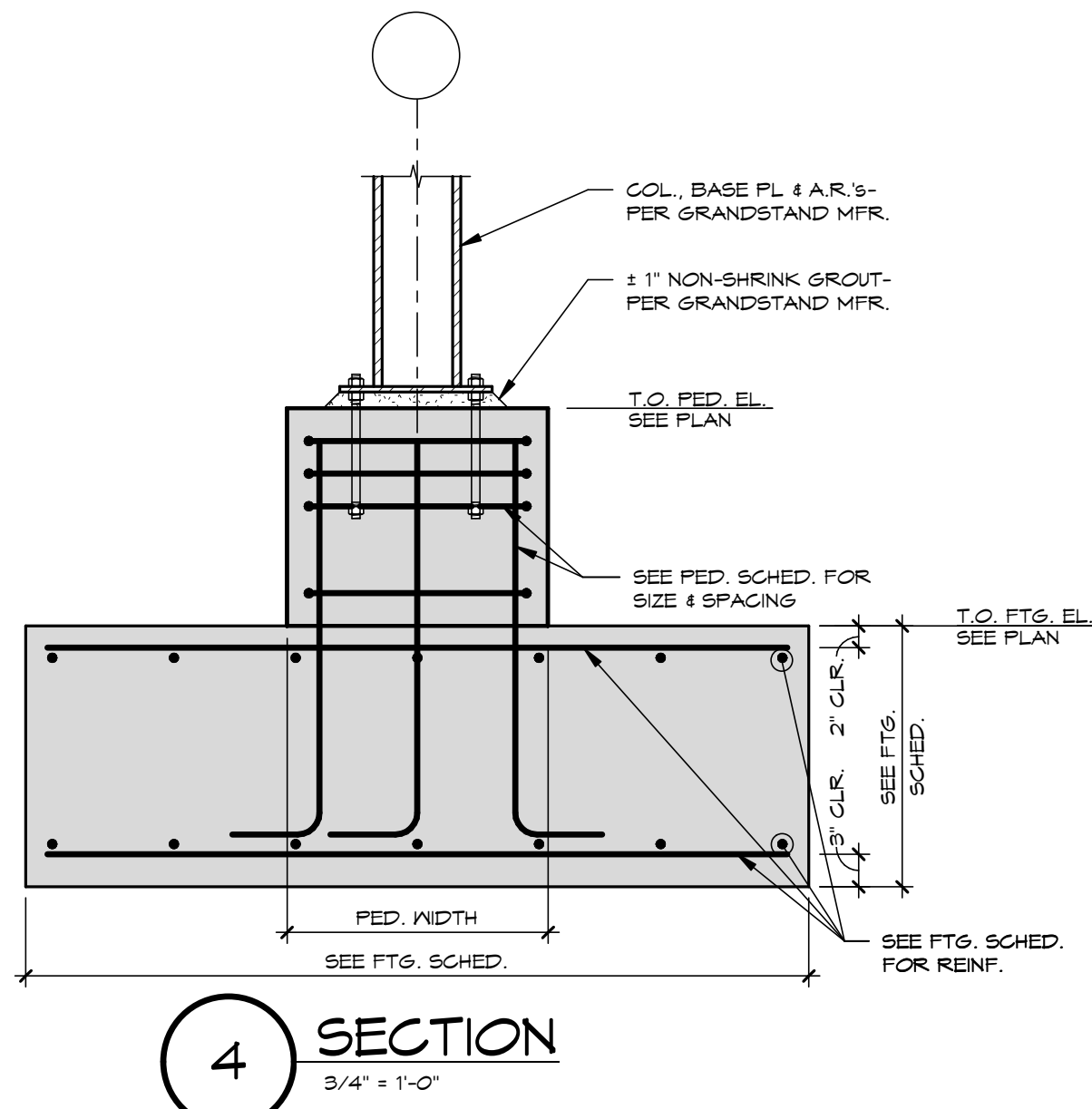
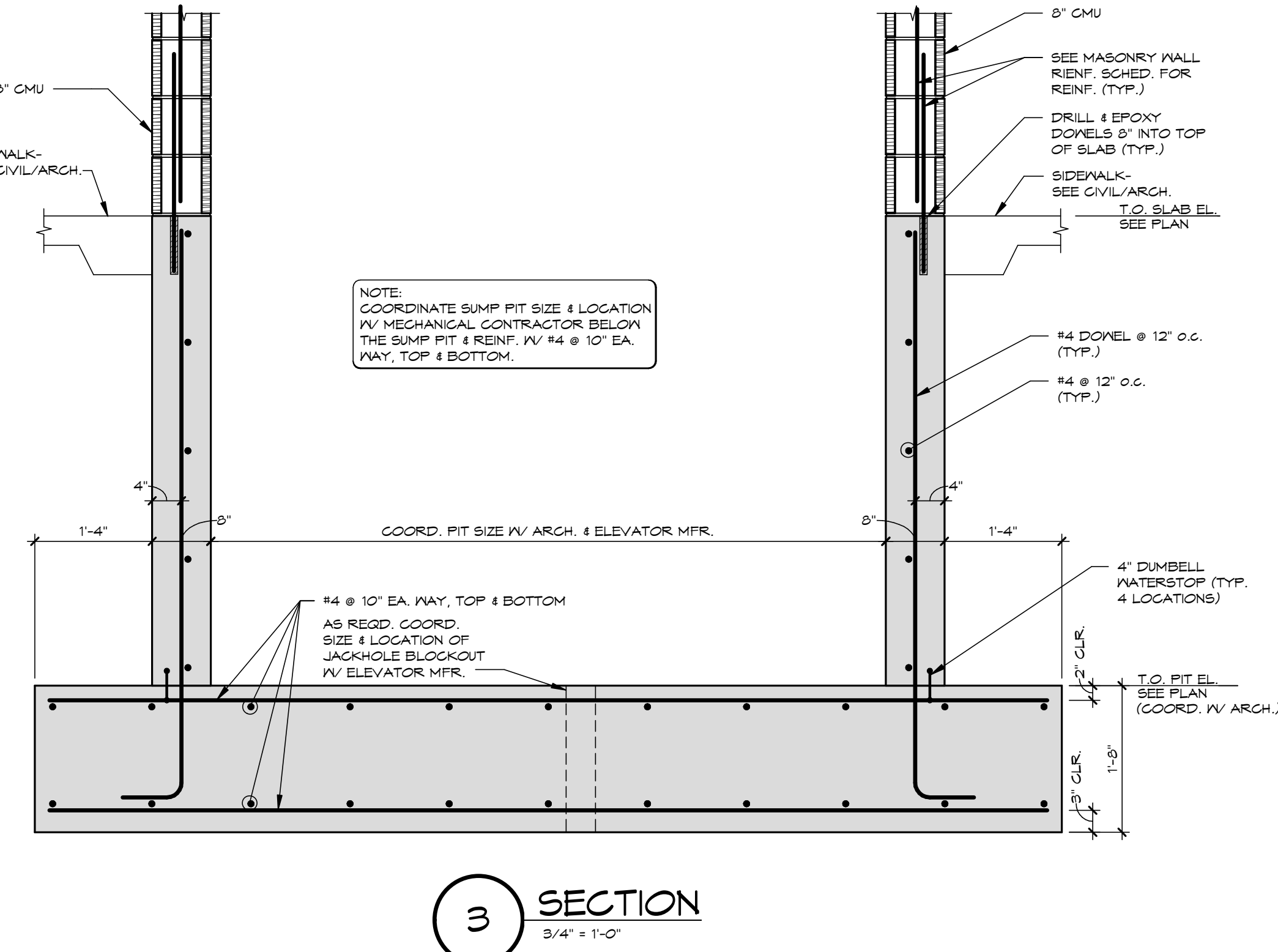
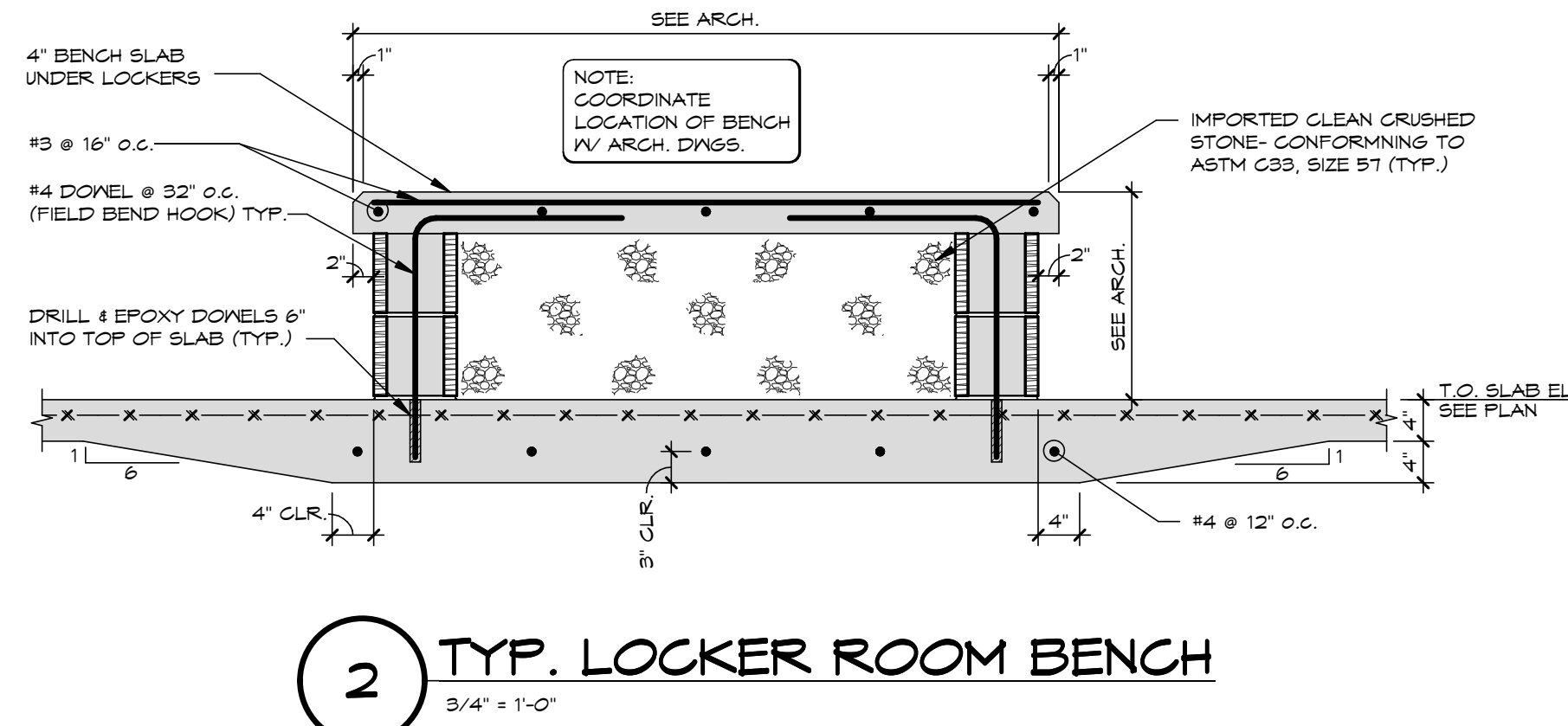
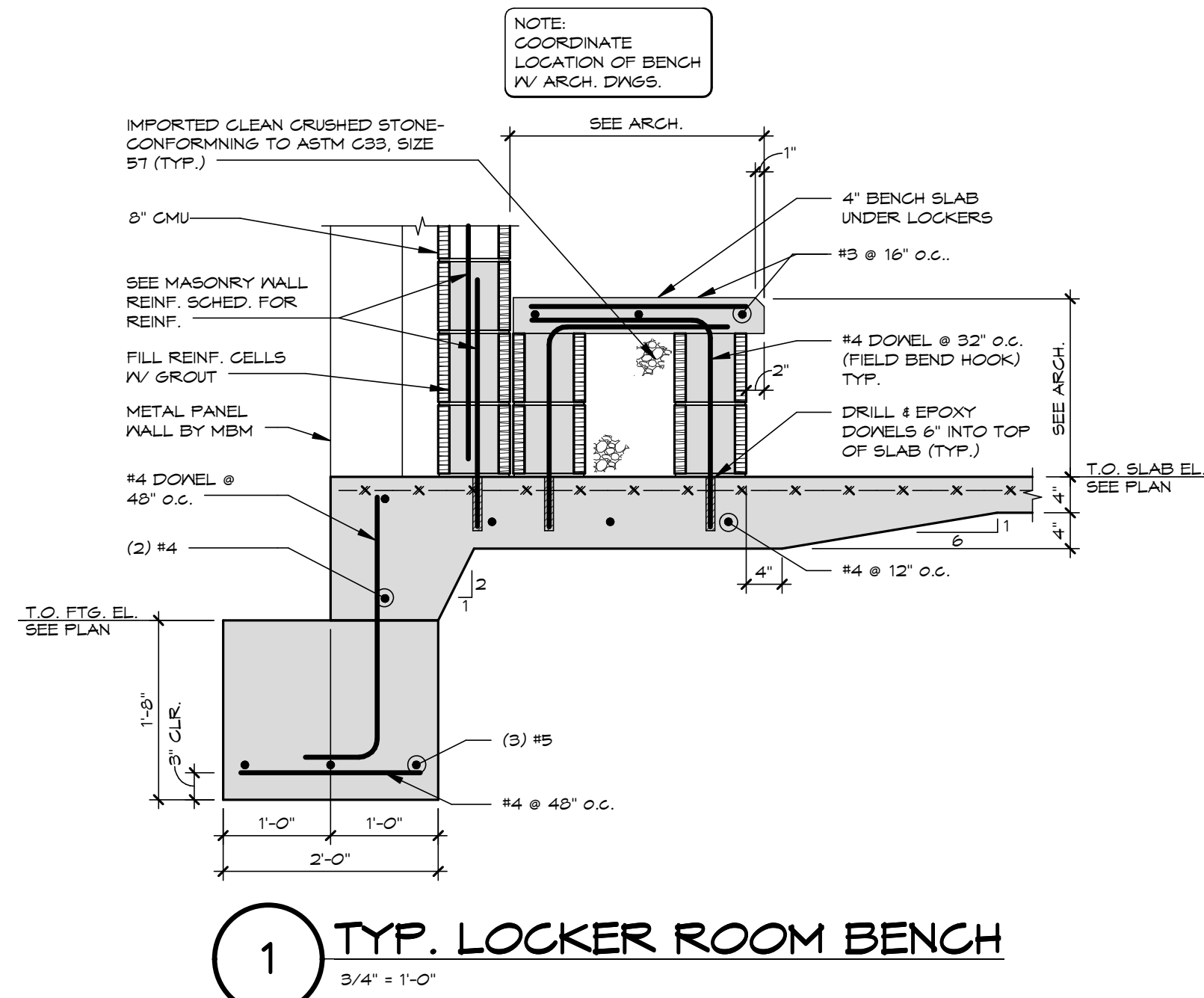
REVISION DATES
DATE DESCRIPTION

SHEET
S3.5
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Tatum Smith
Engineers, Inc.
Structural Engineering
479.621.6128
Rogers, Arkansas

2/25/2016 12:59:57 PM

191210
EEG
TAT
Tatum-Smith
Engineers, Inc.
Project Designer



Hight Jackson
ASSOCIATES
5201 Village Parkway, Suite 300 • Rogers, Arkansas 72758 • (479) 664-4665 • www.hjarch.com

CARTHAGE R-9 SCHOOL DISTRICT
CARTHAGE STADIUM COMPLEX
FIR RD & RIVER ST, CARTHAGE, MO

DRAWN BY:
EEG
CHECK BY:
MAS
ISSUE DATE:
02/29/2016

PROJECT NO:
1365

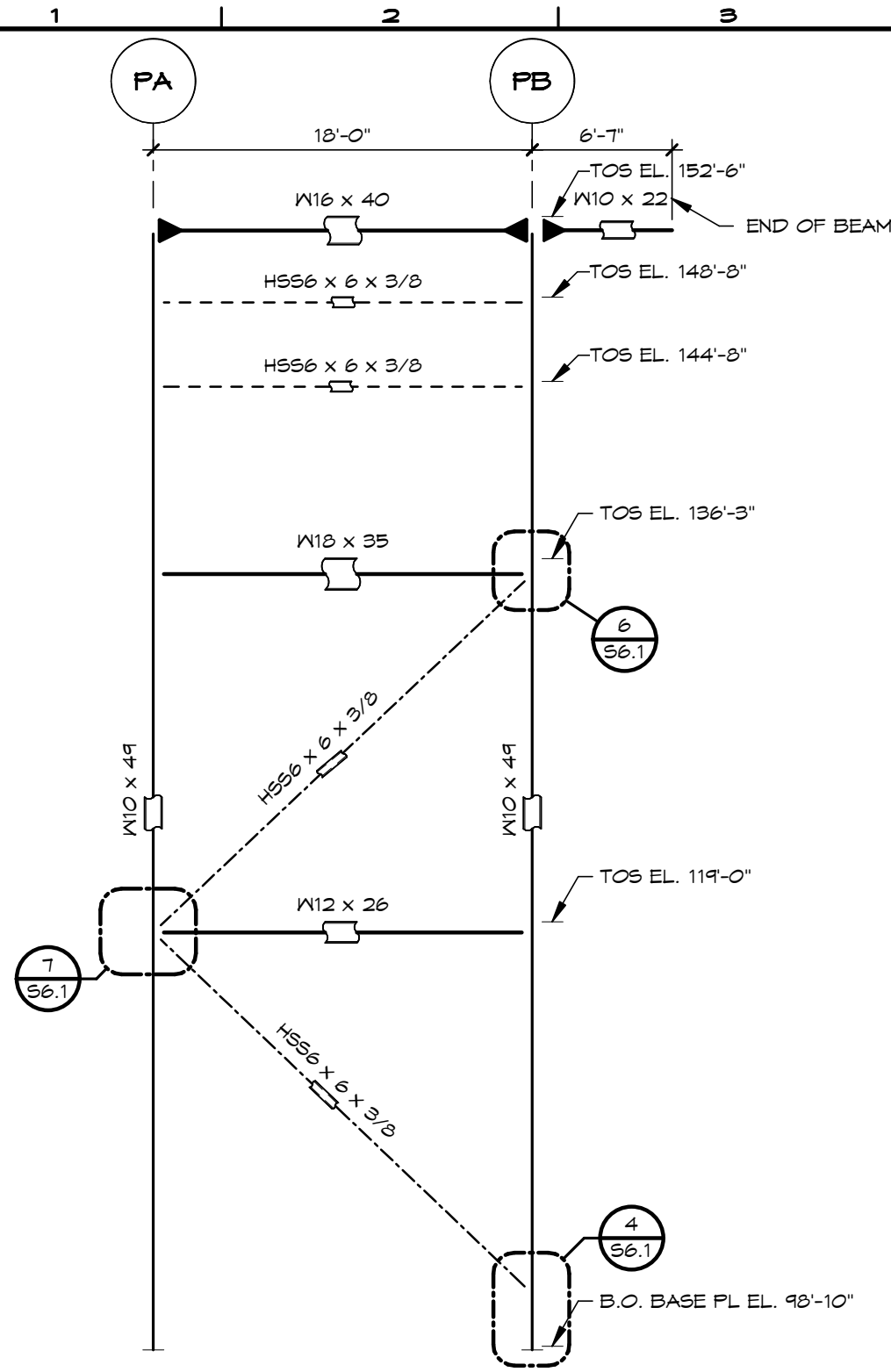
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DATE DESCRIPTION

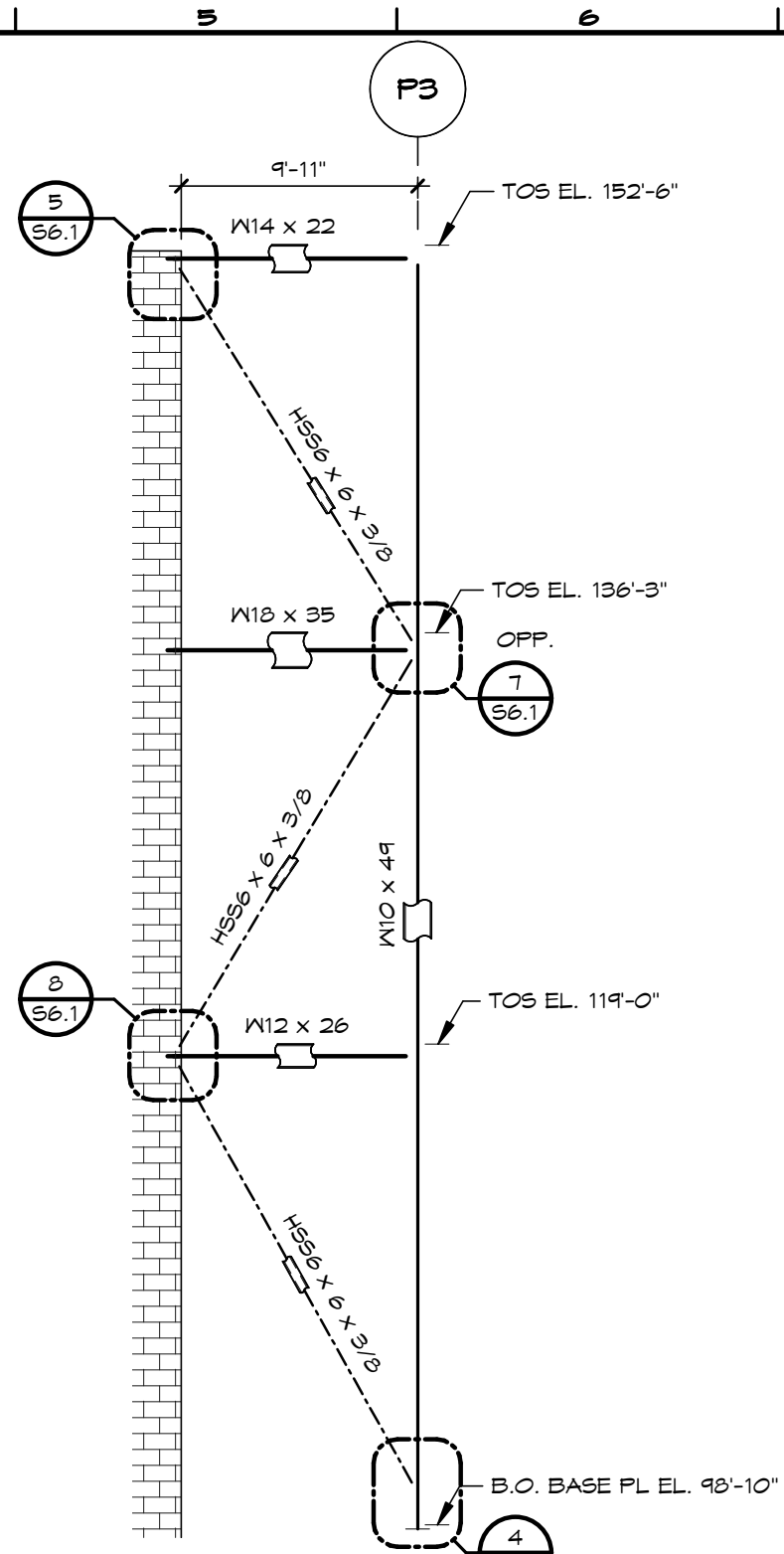
SHEET
S4.2

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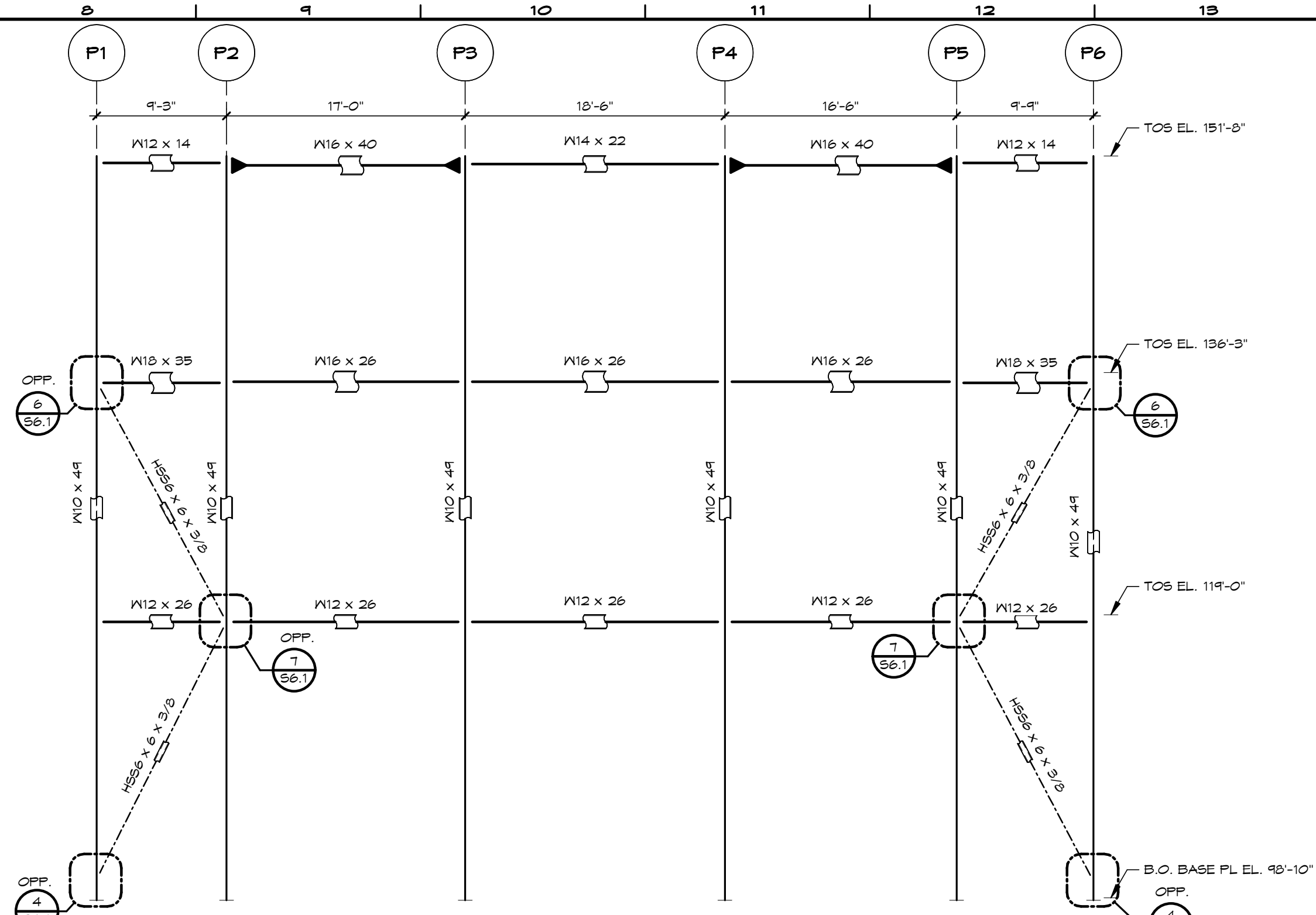
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Engineers, Inc.
Structural Engineering
479.621.6128 Rogers, Arkansas



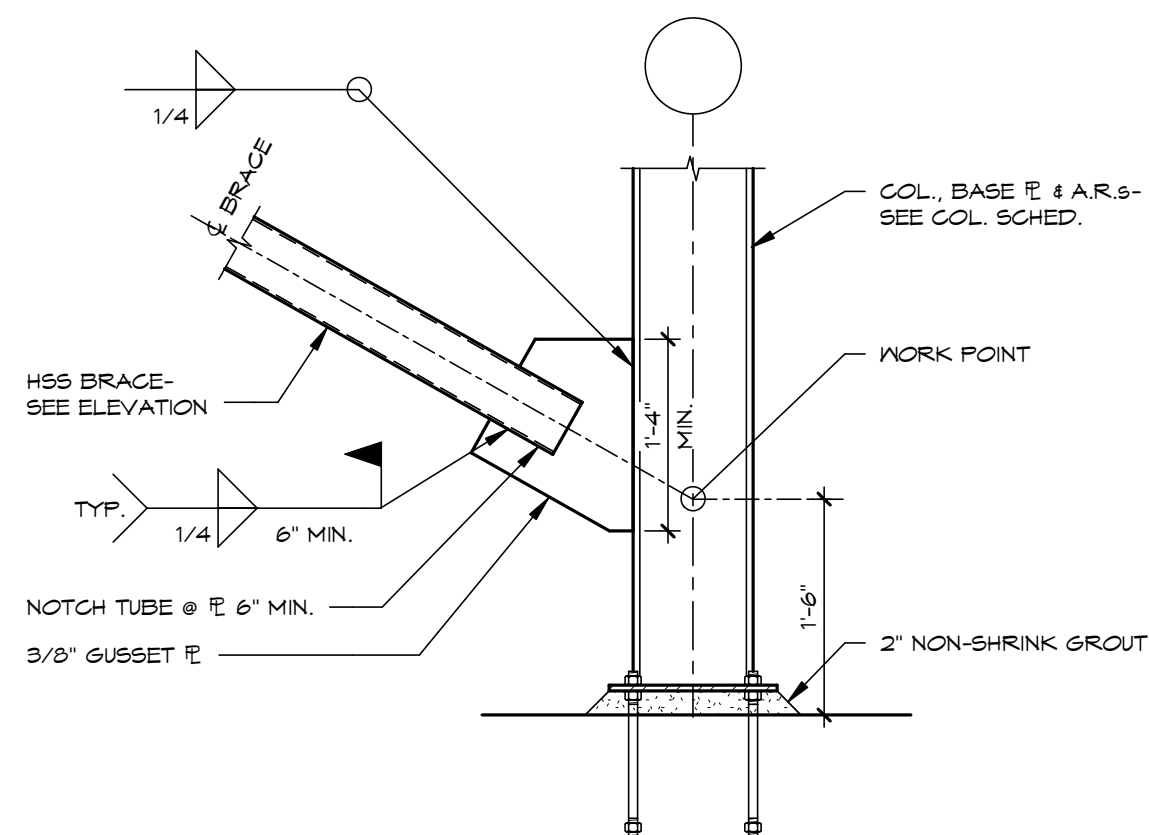
1 PARTIAL ELEVATION
1/8" = 1'-0"



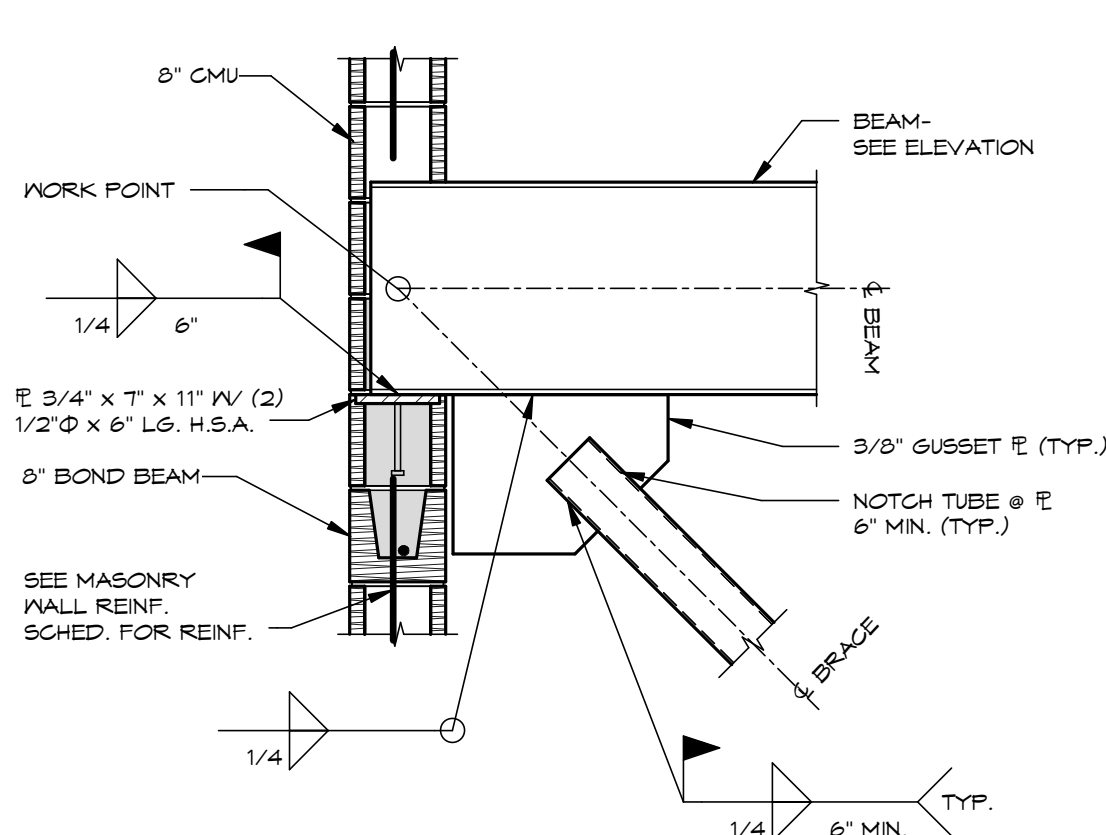
2 PARTIAL ELEVATION
1/8" = 1'-0"



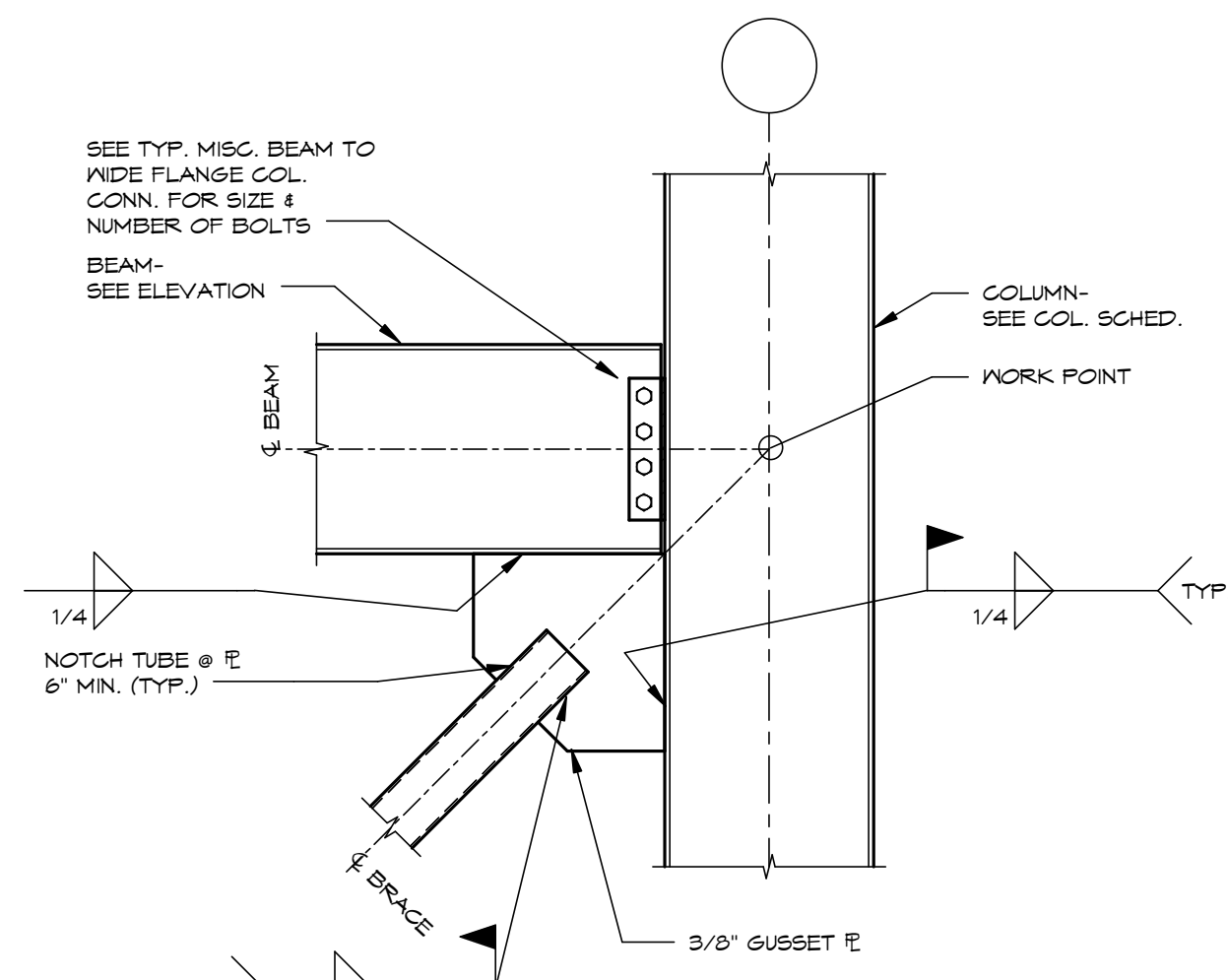
3 PARTIAL ELEVATION
1/8" = 1'-0"



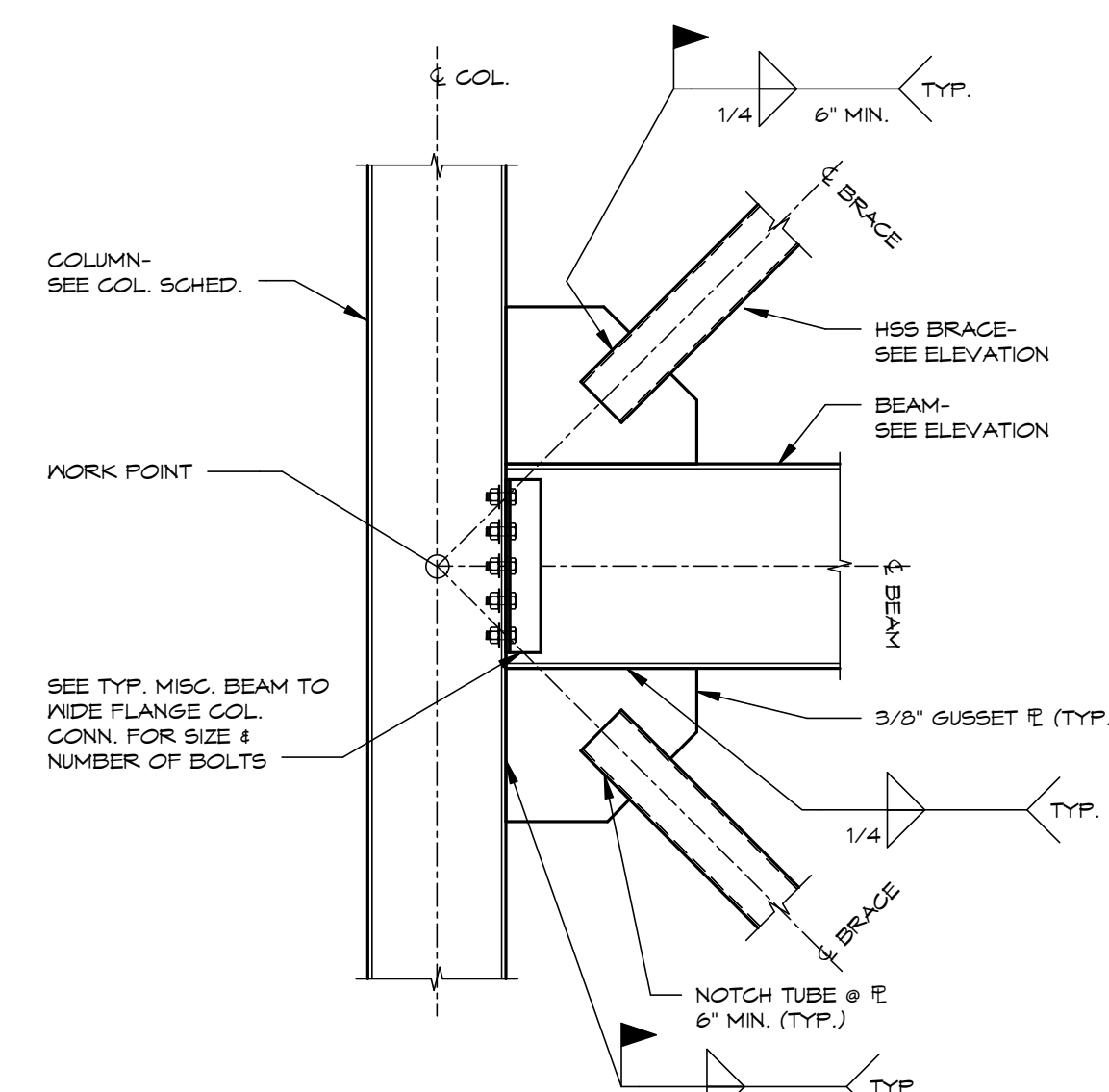
4 DETAIL
3/4" = 1'-0"



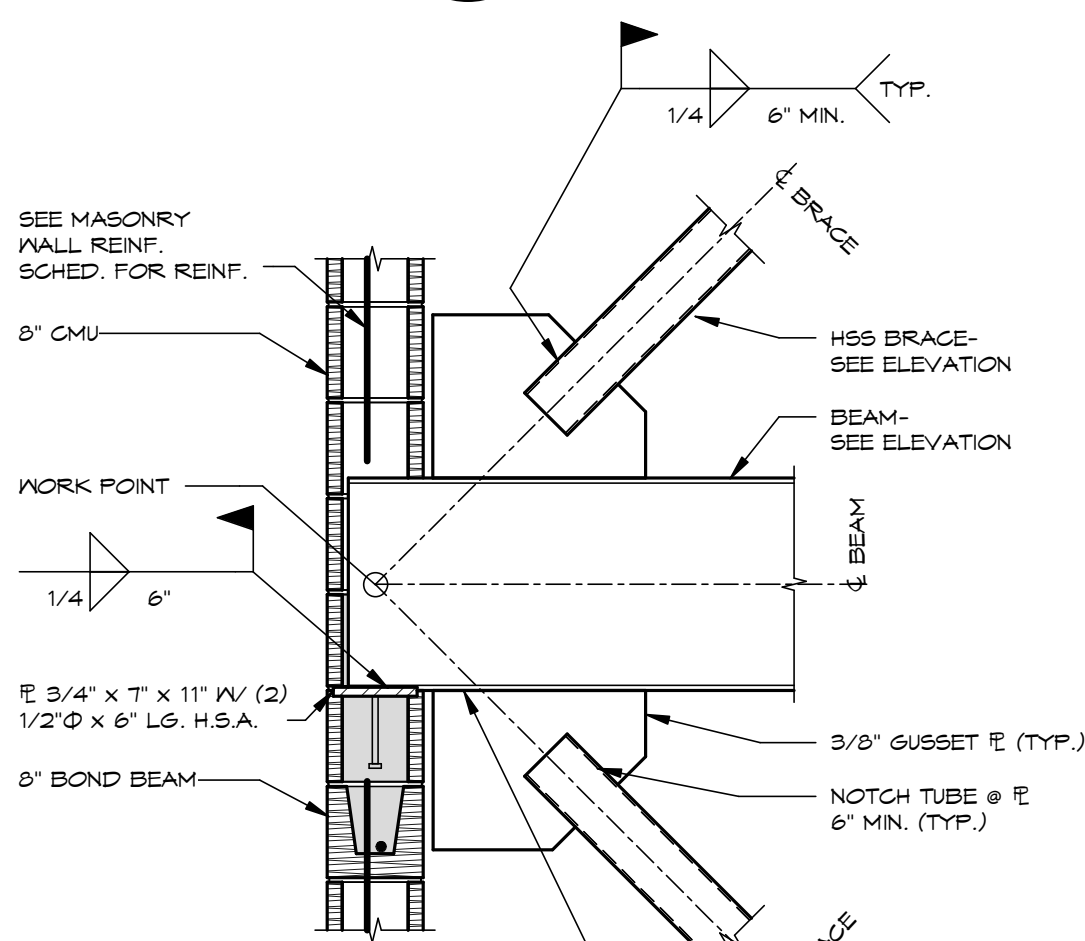
5 DETAIL
3/4" = 1'-0"



6 DETAIL
3/4" = 1'-0"



7 DETAIL
3/4" = 1'-0"



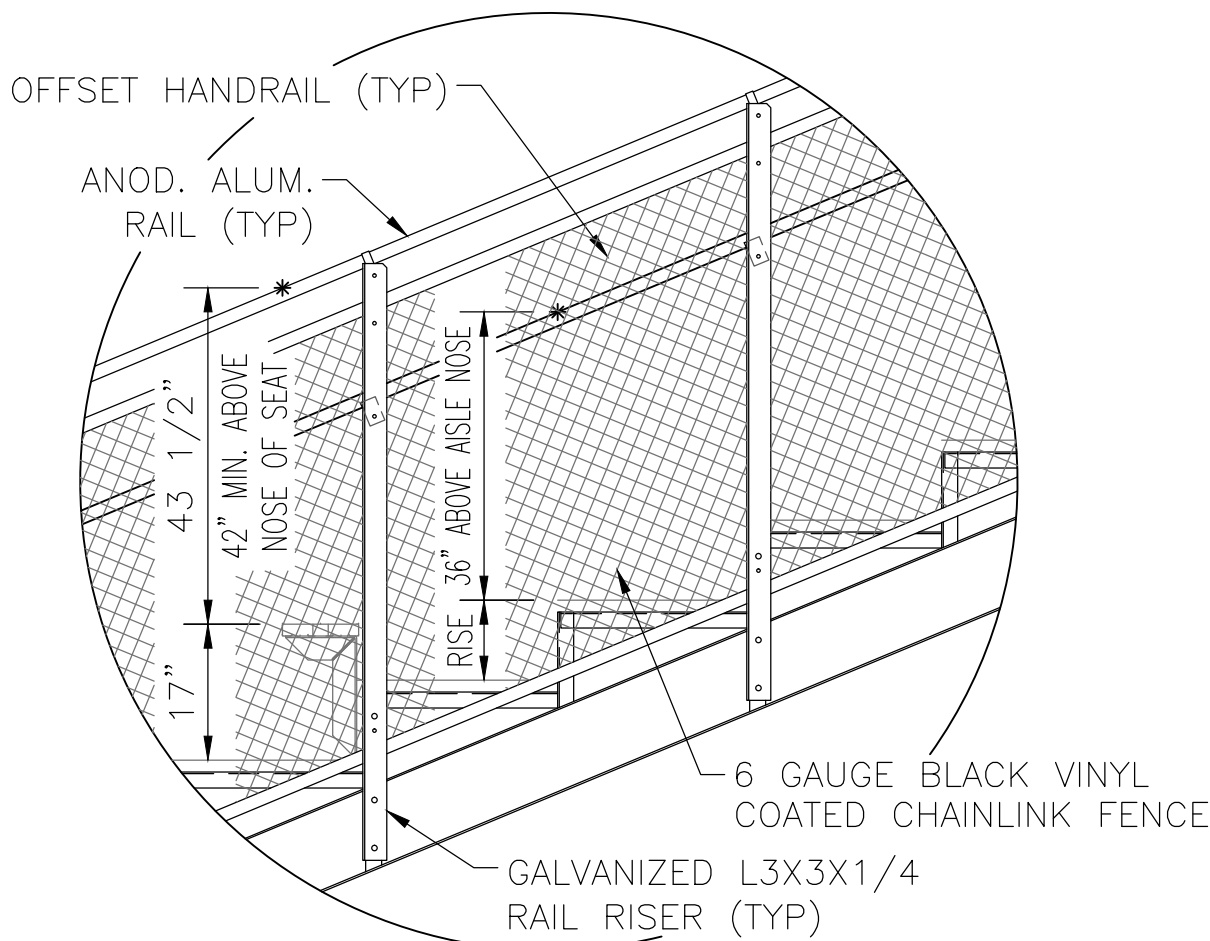
8 DETAIL
3/4" = 1'-0"

DESIGN LOADING

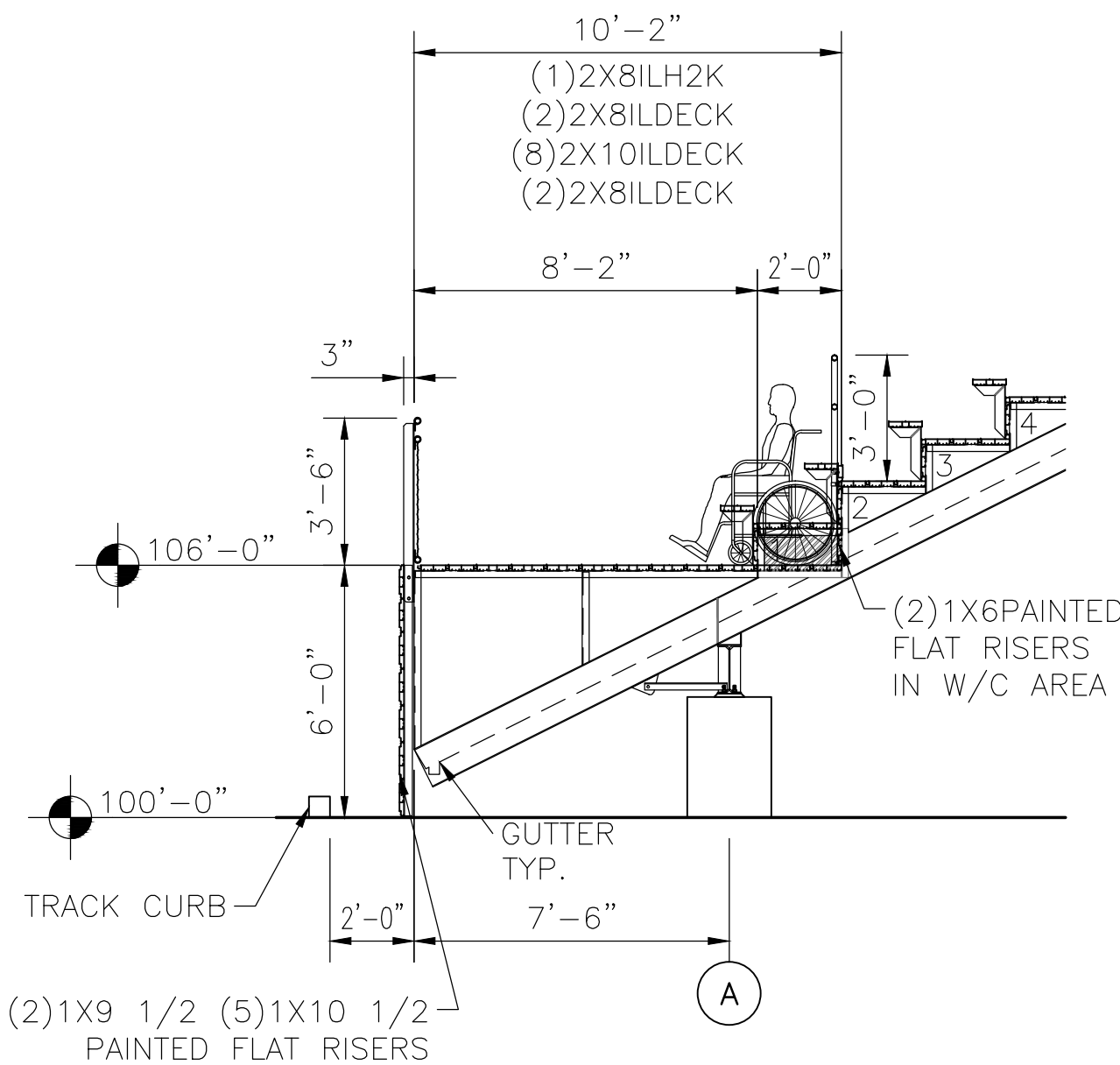
TREAD & SEAT AREA 100psf UNIFORM LIVE LOAD.
SEAT (VERTICAL) 120 lbs/lf.
SEAT (HORIZONTAL SWAY) 24lbs/lf PARALLEL AND
10lbs/lf PERPENDICULAR TO SEAT.
HANDRAIL & GUARDRAIL 50 lbs/lf IN ANY DIRECTION.
HANDRAIL & GUARDRAIL 200 lbs CONCENTRATED IN ANY DIRECTION.
SNOW LOADS AS PER STATE ADOPTED CODE
WIND LOADS AS PER STATE ADOPTED CODE
SEISMIC LOADS AS PER STATE ADOPTED CODE

NOTES

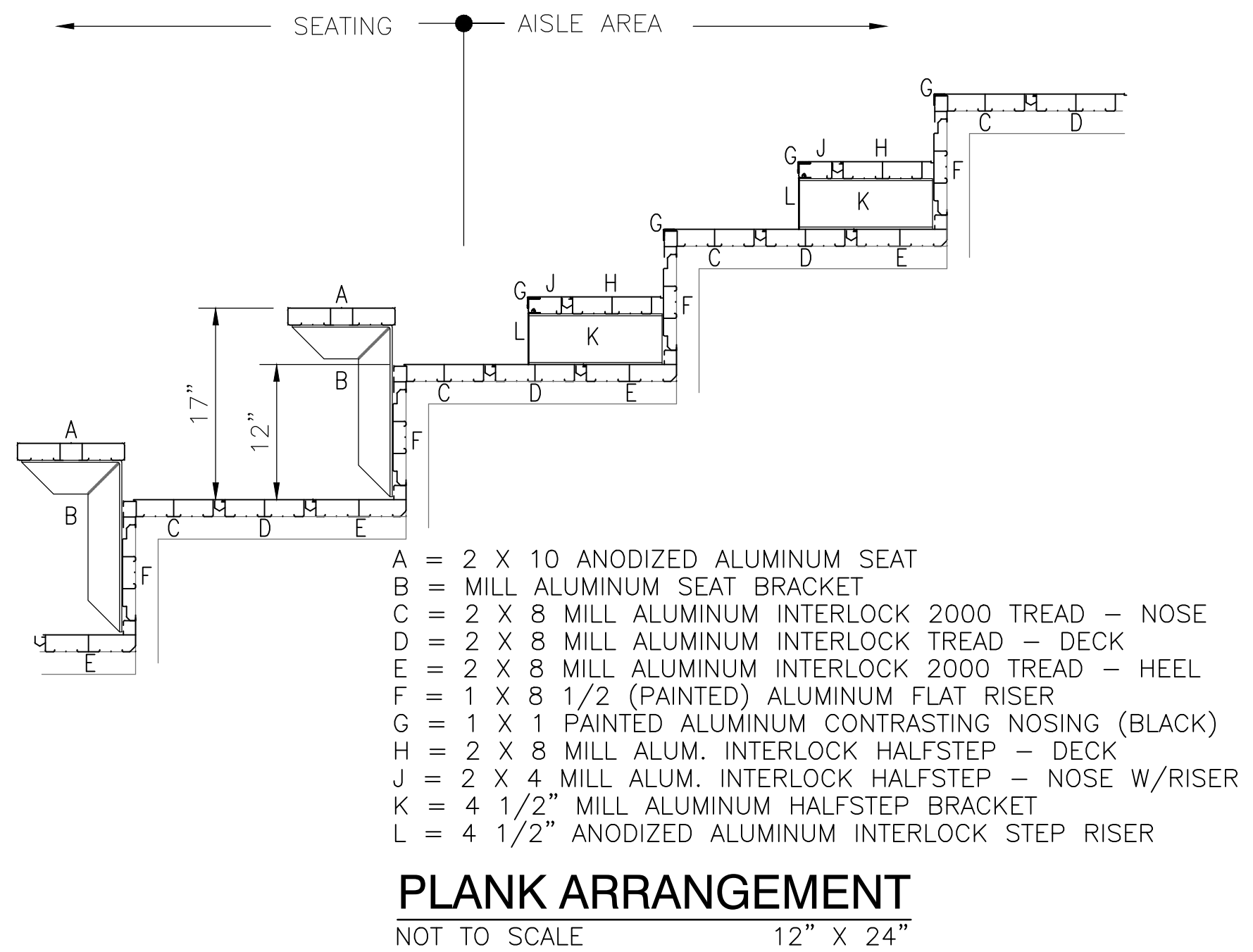
- ALL STRUCTURAL AND MISCELLANEOUS STEEL SHALL CONFORM TO THE FOLLOWING A.I.S.C. STANDARDS
PLATE UP TO 1/2" THICK = A36
PLATE U.N.O. = A572 GRADE 50
ANGLE = A36/A36M GRADE 50
WIDE FLANGE = A992 Fy=50 ksi
CHANNEL = A36/A36M GRADE 50
ROD = A36/A529 GRADE 50
TUBE = A500 GRADE "B" 46 ksi
- WELDS ARE ALL AROUND WITH TYPE ER70S-6 WIRE MIG.
- ALL STEEL TO BE HOT DIPPED GALVANIZED TO A.S.T.M. A-123-89 ae1.
- STRUCTURAL BOLTS ARE HOT DIPPED GALV. AND ARE EQUAL TO OR GREATER THAN A-307.
- NO CONNECTIONS UTILIZING HIGH STRENGTH BOLTS ARE CLASSED AS SLIP CRITICAL.
- ANODIZED ALUMINUM RAIL IS 1 1/4" NOMINAL PIPE SIZE. (1 5/8" O.D.)
- SOUTHERN BLEACHER COMPANY AS A MANUFACTURER AND INSTALLER OF GRANDSTAND SEATING IS NOT AUTHORIZED TO CERTIFY PLANS AS ADA COMPLIANT. HOWEVER, TO THE BEST OF OUR KNOWLEDGE THESE PLANS MEET OR EXCEED ADA REQUIREMENTS FOR QUANTITY OF ADA SEATING, ACCESS/EGRESS TO ADA SEATING, & DISPERSAL OF ADA SEATING.
- ALL FIELD CONNECTIONS ARE NON-SLIP CRITICAL U.N.O. ALL CONNECTIONS ARE DESIGNED TO UTILIZE A307 BOLTS, IT IS ACCEPTABLE TO USE A325N BOLTS IN LIEU OF THE A307 BOLTS. THE INSTALLATION OF THESE BOLTS ARE TO BE TIGHTENED A SNUG TIGHT CONDITION AS SPECIFIED BY AISC.



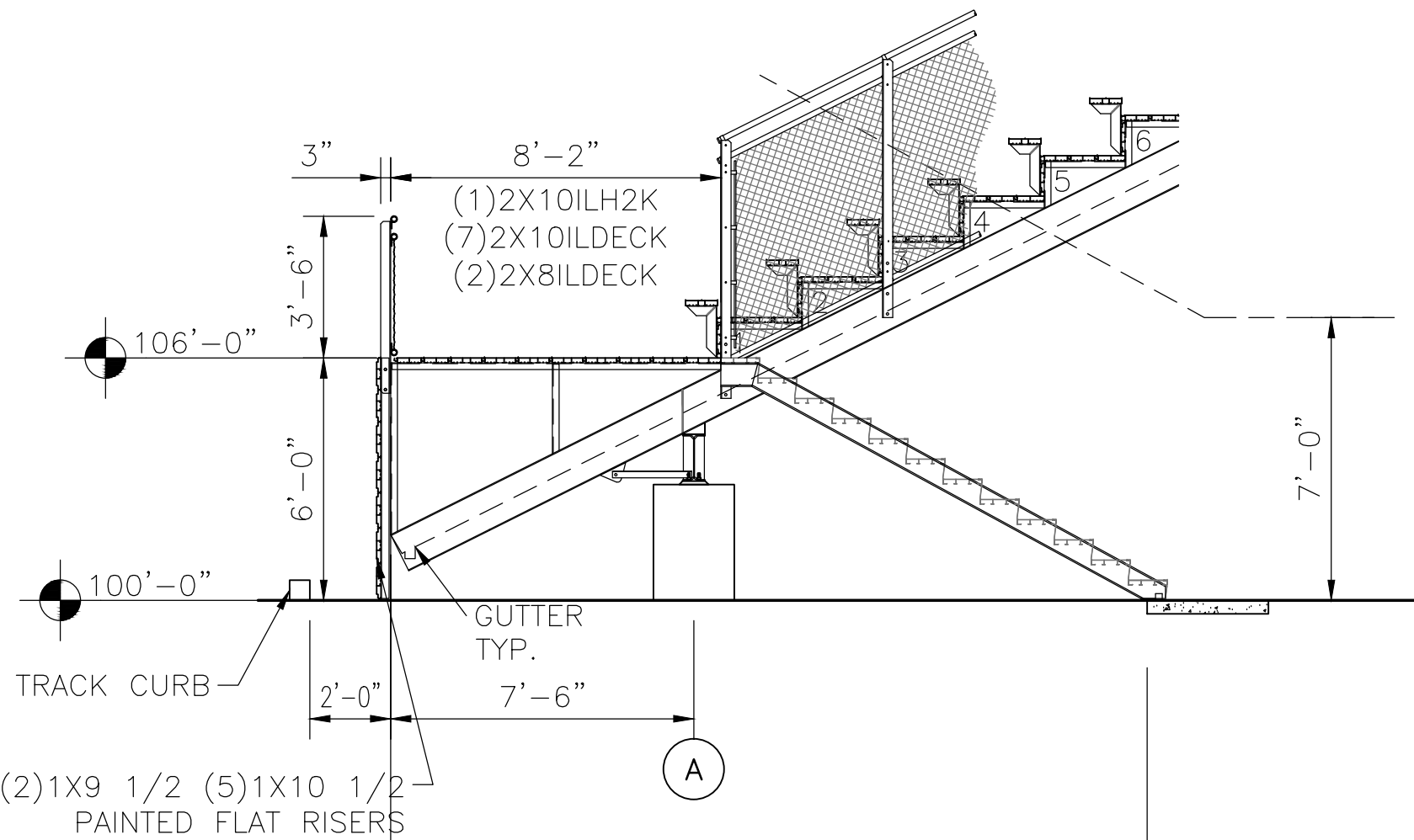
SIDE RAILING @ END AISLE
NOT TO SCALE



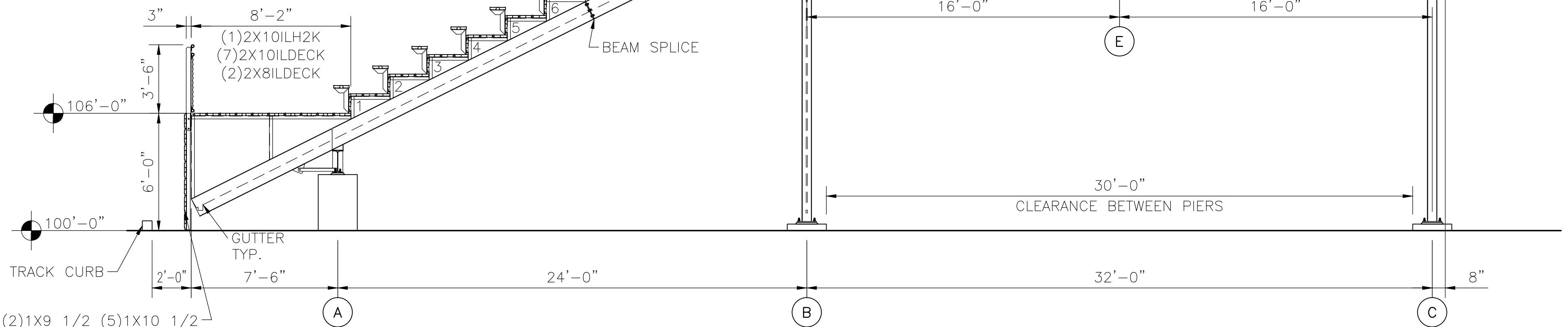
HOME SECTION VIEW @ 27 ROW W/C AREA
SCALE: 1/4" = 1'-0"



PLANK ARRANGEMENT
NOT TO SCALE 12" X 24"



HOME SECTION VIEW @ PORTAL EXIT
SCALE: 1/4" = 1'-0"



HOME SECTION VIEW @ 27 ROW SECTIONS
SCALE: 1/4" = 1'-0"

NOT FOR CONSTRUCTION

DETAIL NUMBER 1-101 SHEET SHOWN ON 101 SHEET CUT FROM

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PO Box One, Graham, Texas 76450
801 Fifth Street,
Phone: 940/549-0733 Fax: 940/549-1365
ESTABLISHED 1946

Southern
BLEACHER COMPANY

GRANDSTANDS • BLEACHERS • STADIUMS

HOME SECTION VIEW @ 27 ROW SECTIONS
12" RISE X 24" TREAD \ 27 ROWS

CARTHAGE R-9 SCHOOL DISTRICT
CARTHAGE STADIUM
CARTHAGE, MISSOURI

REV	BY	DATE	DESCRIPTION
1	CF	12/02/14	NONE THIS PAGE
2	CF	12/17/16	REVISED
3	CF	12/03/13	ADDED LOADS
4	CF	11/17/13	NONE THIS PAGE
5	CF	11/9/13	COLUMN SPACING
6	CF	11/5/13	REDRAW
7	CF	10/27/13	ADDED ROWS & CROSSWALK
8	CF	8/28/13	REDRAW

JOB NUMBER
15sd173

SHEET
1

OF
10

22" X 34" (11x17 = 1/2 indicated scale)

TREAD & SEAT AREA 100psf UNIFORM LIVE LOAD.
SEAT (VERTICAL) 120 lbs/lf.
SEAT (HORIZONTAL SWAY) 24lbs/lf PARALLEL AND
10lbs/lf PERPENDICULAR TO SEAT.

HANDRAIL & GUARDRAIL 50 lbs/lf IN ANY DIRECTION.
HANDRAIL & GUARDRAIL 200 lbs CONCENTRATED IN ANY DIRECTION.
SNOW LOADS AS PER STATE ADOPTED CODE
WIND LOADS AS PER STATE ADOPTED CODE
SEISMIC LOADS AS PER STATE ADOPTED CODE

- 1 ALL STRUCTURAL AND MISCELLANEOUS STEEL SHALL CONFORM TO THE FOLLOWING A.I.S.C. STANDARDS
PLATE UP TO 1/2" THICK = A36
PLATE U.N.O. = A572 GRADE 50
ANGLE = A36/A36M GRADE 50
WIDE FLANGE = A992 Fy=50 ksi
CHANNEL = A36/A36M GRADE 50
ROD = A36/A529 GRADE 50
TUBE = A500 GRADE "B" 46 ksi
- 2 WELDS ARE ALL AROUND WITH TYPE ER70S-6 WIRE MIG.
- 3 ALL STEEL TO BE HOT DIPPED GALVANIZED TO A.S.T.M. A-123-89 ae1.
- 4 STRUCTURAL BOLTS ARE HOT DIPPED GALV. AND ARE EQUAL TO OR GREATER THAN A-307.
- 5 NO CONNECTIONS UTILIZING HIGH STRENGTH BOLTS ARE CLASSED AS SLIP CRITICAL.
- 6 ANODIZED ALUMINUM RAIL IS 1 1/4" NOMINAL PIPE SIZE. (1 5/8" O.D.)
- 7 SOUTHERN BLEACHER COMPANY AS A MANUFACTURER AND INSTALLER OF GRANDSTAND SEATING IS NOT AUTHORIZED TO CERTIFY PLANS AS ADA COMPLIANT. HOWEVER, TO THE BEST OF OUR KNOWLEDGE THESE PLANS MEET OR EXCEED ADA REQUIREMENTS FOR QUANTITY OF ADA SEATING, ACCESS/EGRESS TO ADA SEATING, & DISPERSAL OF ADA SEATING.

8 ALL FIELD CONNECTIONS ARE NON-SLIP CRITICAL U.N.O. ALL CONNECTIONS ARE DESIGNED TO UTILIZE A307 BOLTS, IT IS ACCEPTABLE TO USE A325N BOLTS IN LIEU OF THE A307 BOLTS. THE INSTALLATION OF THESE BOLTS ARE TO BE TIGHTENED A SNUG TIGHT CONDITION AS SPECIFIED BY AISC.



SHEET		2		OF		10	
JOB NUMBER		15sd173					
REV BY	DATE	DESCRIPTION					
CF	09/11/15	CF 12/02/18 NONE THIS PAGE					
CF	12/18/18	REVISED					
CF	12/03/19	ADDED LOADS					
CF	11/17/15	NONE THIS PAGE					
CF	11/9/19	COLUMN SPACING					
CF	11/5/15	REDRAW					
EM/JM	10/27/19	ADDED ROWS & CROSSLINK					
CF	9/28/15	REDRAW					
TYPE		HOME SECTION VIEW @ 18 ROW SECTION 18" RISE X 36" TREAD\ 18 ROWS					
TITLE		CARTHAGE R-9 SCHOOL DISTRICT CARTHAGE STADIUM CARTHAGE, MISSOURI					
NOT FOR CONSTRUCTION		 PO Box One, Graham, Texas 76450 801 Fifth Street, Phone: (940)549-0733 Fax: (940)549-1365 Established 1946 1 GRANDSTANDS • BLEACHERS • STADIUMS					
 1- DETAIL NUMBER 3/10- SHEET SHOWN ON SHEET CUT FROM		THIS DRAWING AND ALL INFORMATION HEREON IS THE PROPERTY OF THE SOUTHERN BLEACHER COMPANY, AND IS CONFIDENTIAL AND MUST NOT BE MADE PUBLIC OR COPIED. THIS DRAWING IS LOANED SUBJECT TO RETURN UPON DEMAND, AND IS NOT TO BE USED IN ANY MANNER DISSEMINATE IN ANY WAY DETRIMENTAL TO OUR INTERESTS.					

22" X 34" (11x17 = 1/2 indicated scale)



DESIGN LOADING

TREAD & SEAT AREA 100psf UNIFORM LIVE LOAD.
SEAT (VERTICAL) 120 lbs/lf.
SEAT (HORIZONTAL SWAY) 24lbs/lf PARALLEL AND 10lbs/lf PERPENDICULAR TO SEAT.
HANDRAIL & GUARDRAIL 50 lbs/lf IN ANY DIRECTION.
HANDRAIL & GUARDRAIL 200 lbs CONCENTRATED IN ANY DIRECTION.
SNOW LOADS AS PER STATE ADOPTED CODE
WIND LOADS AS PER STATE ADOPTED CODE
SEISMIC LOADS AS PER STATE ADOPTED CODE

NOTES

- 1

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TUBE = A500 GRADE "B" 46 ksi
- 2

WELDS ARE ALL AROUND WITH TYPE ER70S-6 WIRE MIG.
- 3

ALL STEEL TO BE HOT DIPPED GALVANIZED TO A.S.T.M. A-123-89 ae1.
- 4

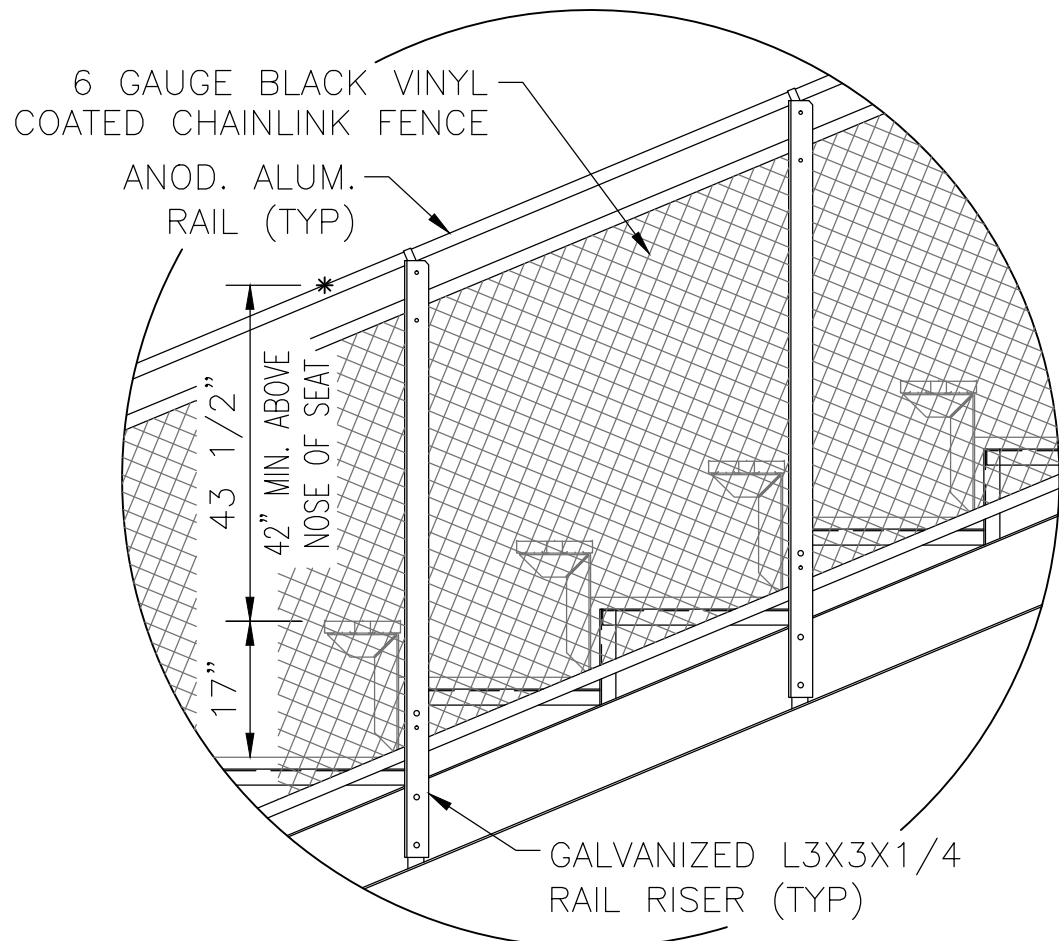
STRUCTURAL BOLTS ARE HOT DIPPED GALV. AND ARE EQUAL TO OR GREATER THAN A-307.
- 5

NO CONNECTIONS UTILIZING HIGH STRENGTH BOLTS ARE CLASSED AS SLIP CRITICAL.
- 6

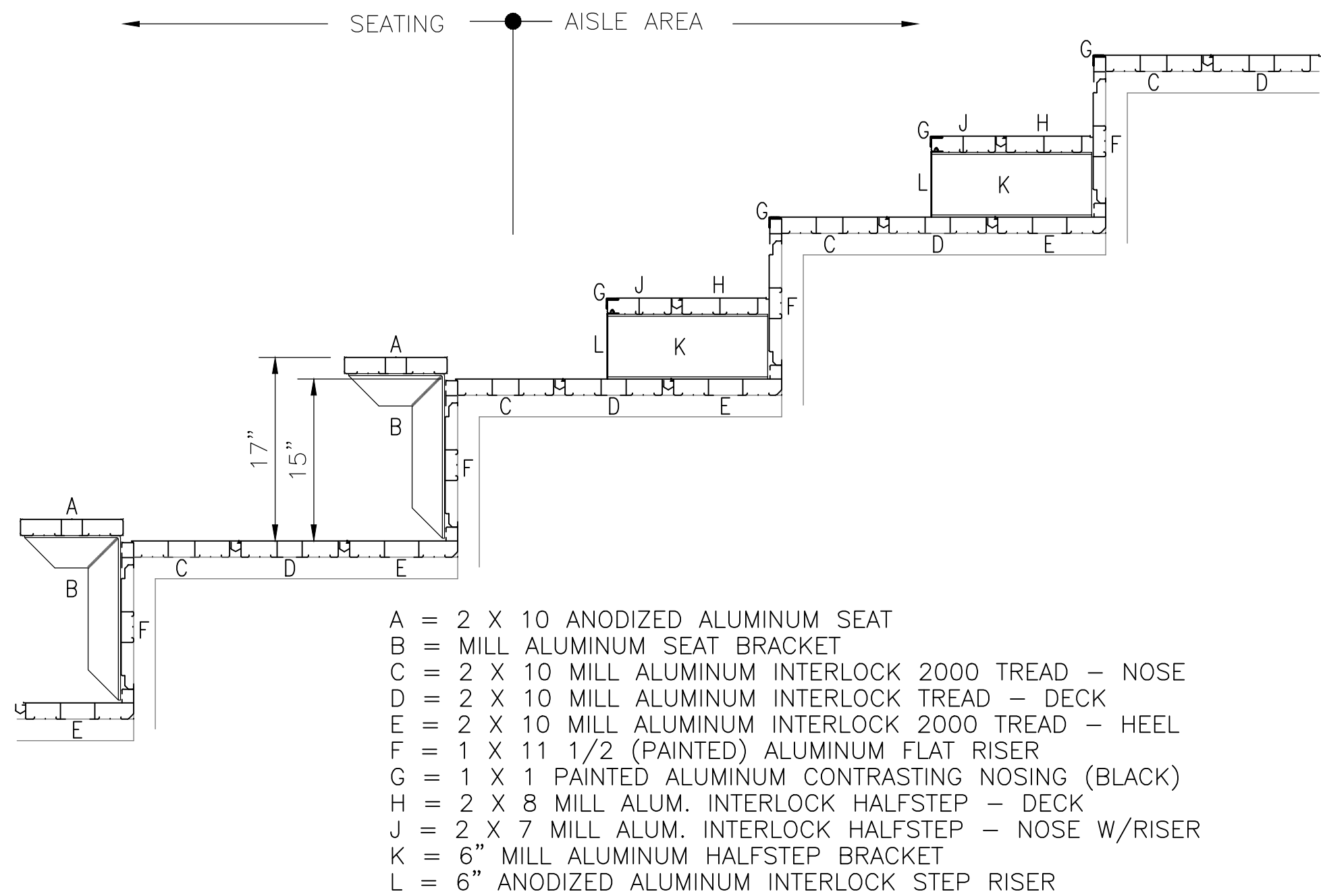
ANODIZED ALUMINUM RAIL IS 1 1/4" NOMINAL PIPE SIZE. (1 5/8" O.D.)
- 7

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- 8

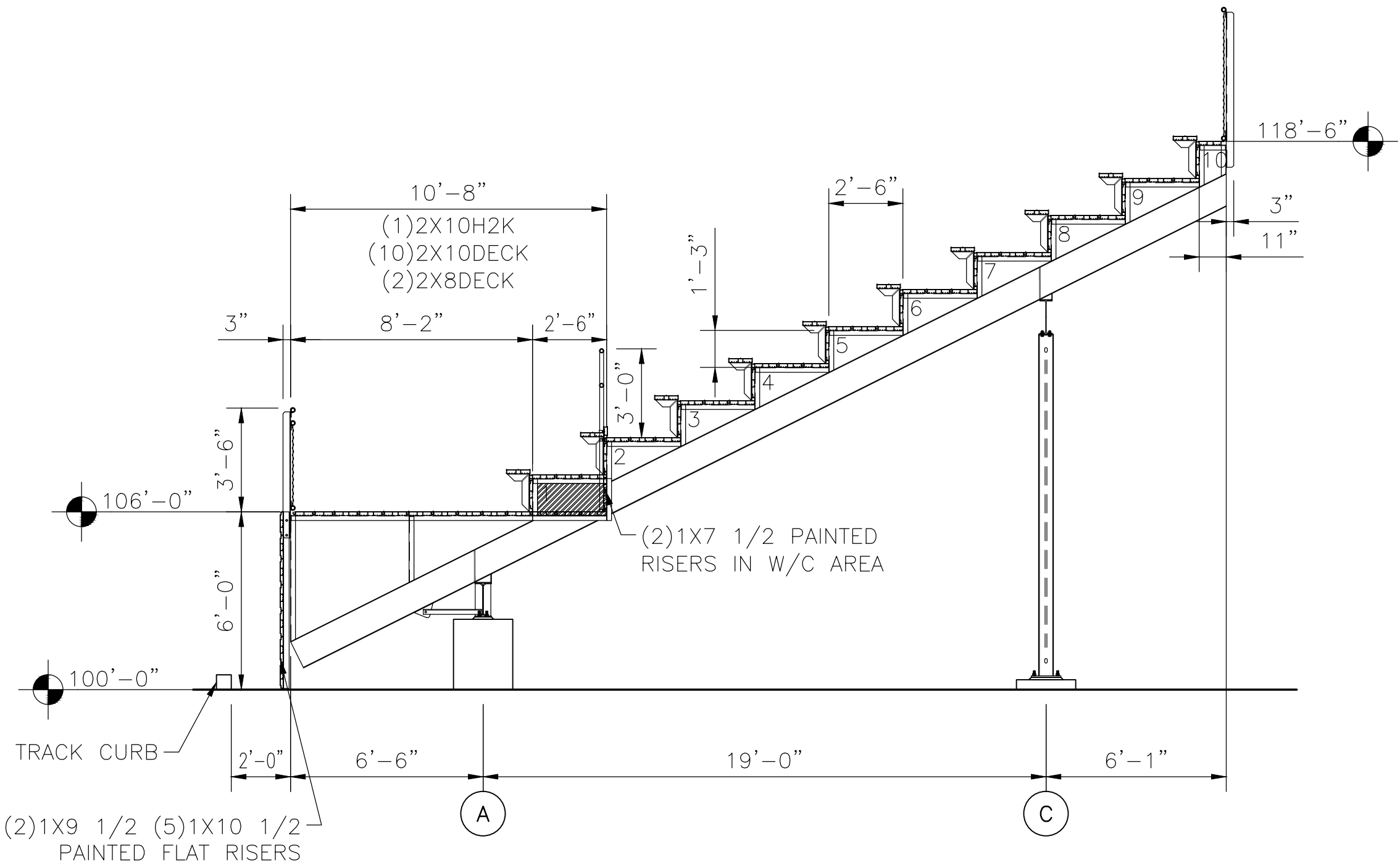
ALL FIELD CONNECTIONS ARE NON-SLIP CRITICAL U.N.O. ALL CONNECTIONS ARE DESIGNED TO UTILIZE A307 BOLTS, IT IS ACCEPTABLE TO USE A325N BOLTS IN LIEU OF THE A307 BOLTS. THE INSTALLATION OF THESE BOLTS ARE TO BE TIGHTENED A SNUG TIGHT CONDITION AS SPECIFIED BY AISC.



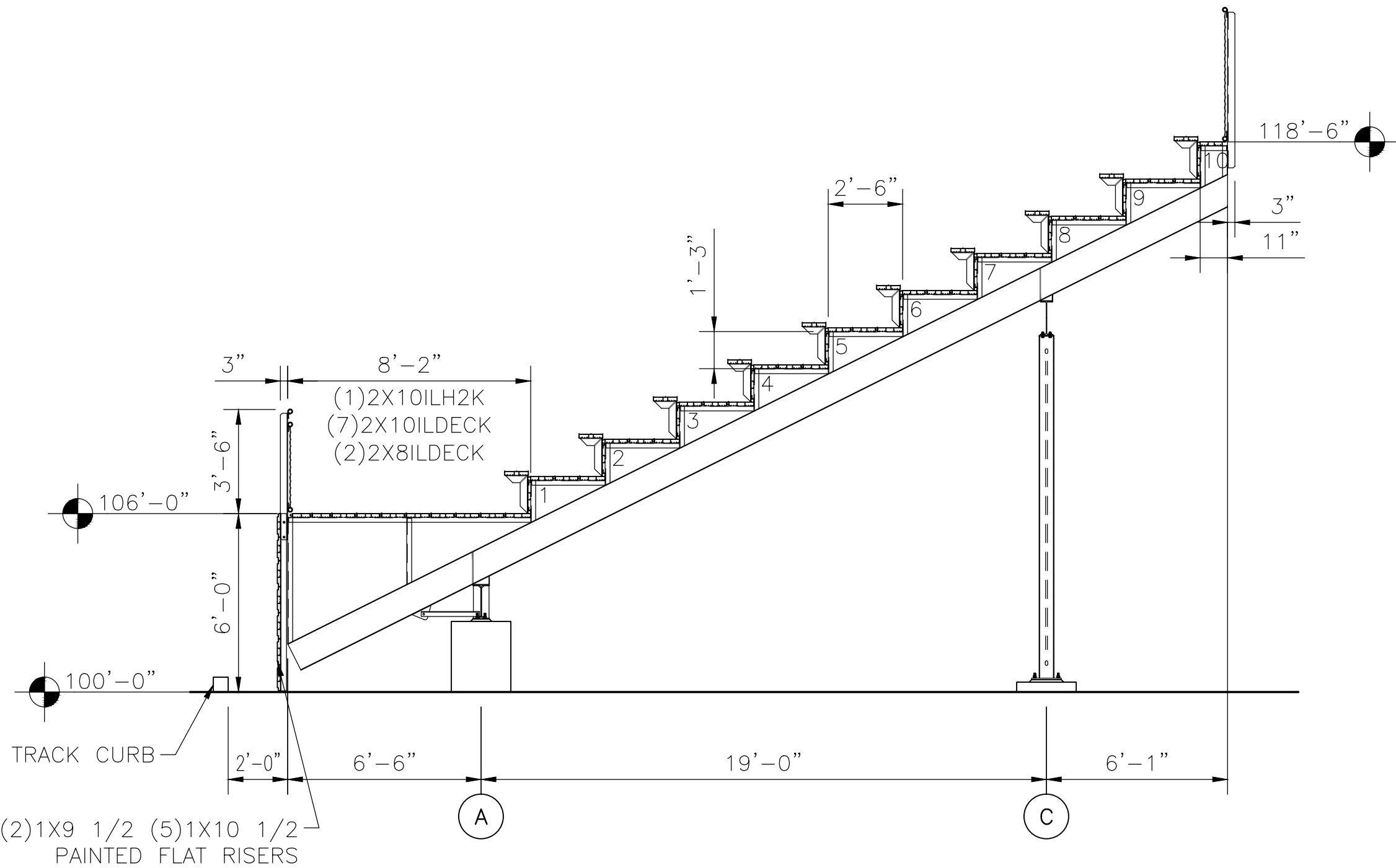
TYPICAL SIDE RAILING
NOT TO SCALE



PLANK ARRANGEMENT
SCALE: 1" = 1'-0" 15" X 30"



HOME SECTION VIEW @ W/C AREA (BAND SECTION)
SCALE:1/4"=1'-0"



HOME SECTION VIEW (BAND SECTION)
SCALE:1/4"=1'-0"

1
310
DETAIL NUMBER
SHEET SHOWN ON
SHEET CUT FROM

NOT FOR CONSTRUCTION

PO Box One, Graham, Texas 76450
801 Fifth Street.
Phone: 940/549-0733 Fax: 940/549-1365
Established 1946

Southern
BLEACHER COMPANY

GRANDSTANDS • BLEACHERS • STADIUMS

HOME SECTION VIEW (BAND BLEACHER)
15" RISE X 30" TREAD \ 10 ROWS

CARTHAGE R-9 SCHOOL DISTRICT
CARTHAGE STADIUM
CARTHAGE, MISSOURI

REV	BY	DATE	DESCRIPTION
1	CF	12/02/14	NONE THIS PAGE
2	CF	1/21/15	REVISED
3	CF	12/03/15	ADDED LOADS
4	CF	11/17/15	REMOVED BAND SEATING & TRAIL
5	CF	11/17/15	CHANGED RISE AND TREAD
6	CF	11/5/15	REDRAW
7	CF	10/27/15	NO CHANGES THIS PAGE
8	CF	8/28/15	REDRAW

DATE
09/11/15

CF

EM/JM

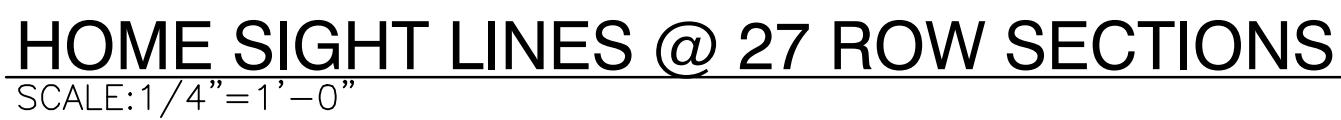
JOB NUMBER
15sd173

SHEET
3

OF
10

22" X 34" (11x17 = 1/2 indicated scale)





SCALE: 1/4" = 1'-0"

22" X 34" (11x17 = 1/2 indicated scale)

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BLEACHER COM
GRANDSTANDS • BLEACHE

PO Box One, Graham, Texas 76450
801 Fifth Street.
Phone: 940/549-0733 Fax: 940/549-1365
Y
DIUMS
Established 1946

DIUMS

1 —————DETAIL NUMBER
3 10 —————SHEET SHOWN ON
—————SHEET

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REV	BY	DATE	DESCRIPTION
1	CF	12/02/16	NONE THIS PAGE
2	CF	12/16/16	REVISED
3	CF	2/03/15	ADDED LOADS
4	CF	1/11/14	ADDED BAND SECTION SIGHT LINES
5	CF	11/9/13	COLUMN SPACING
6	CF	11/5/15	REDRAW
7	CF	02/02/15	ADDED ROWS & CROSSWALK
8	CF	12/21/15	REDO

JOB NUMBER

15sd173

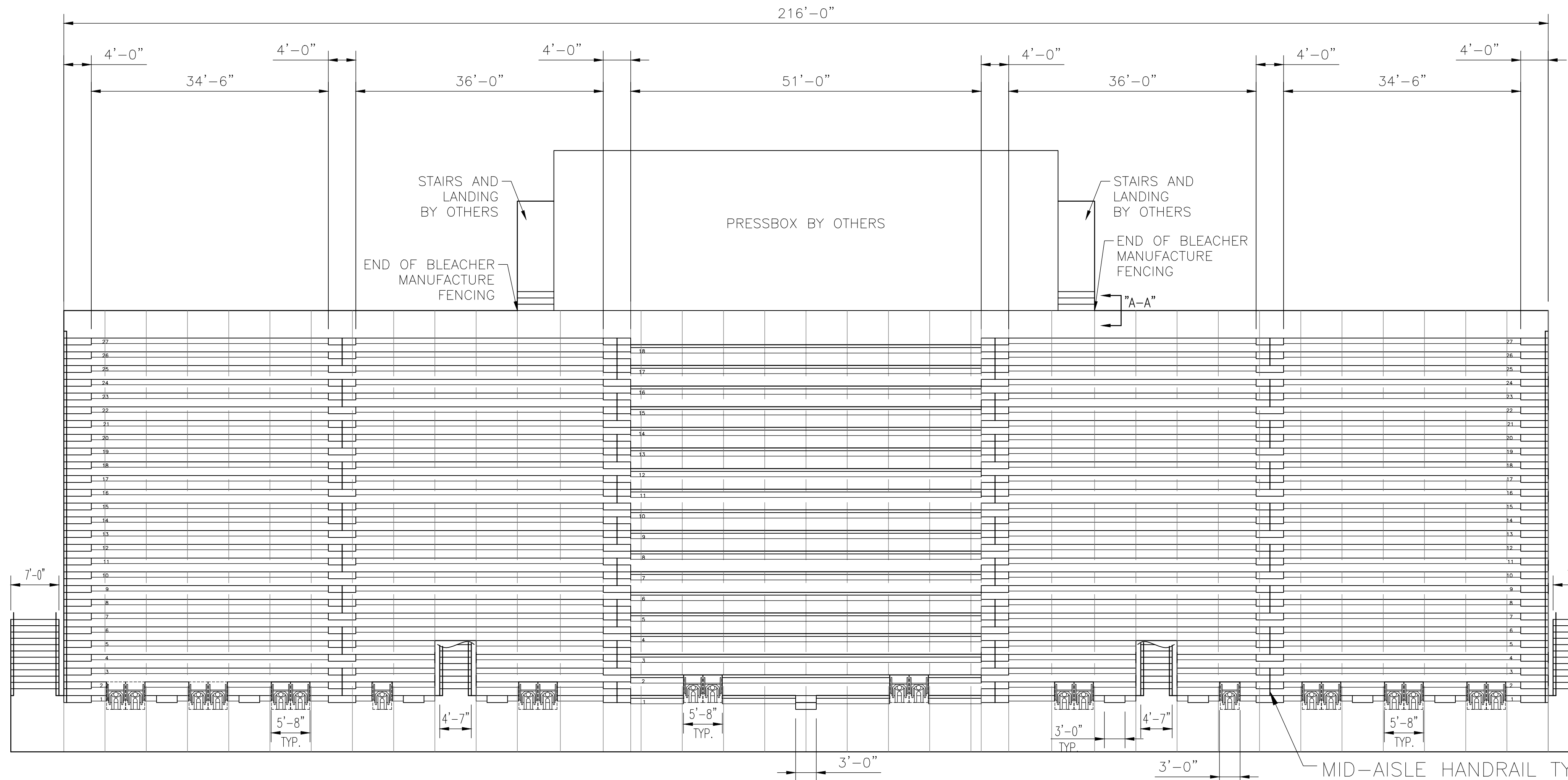
SHEET

5

OF

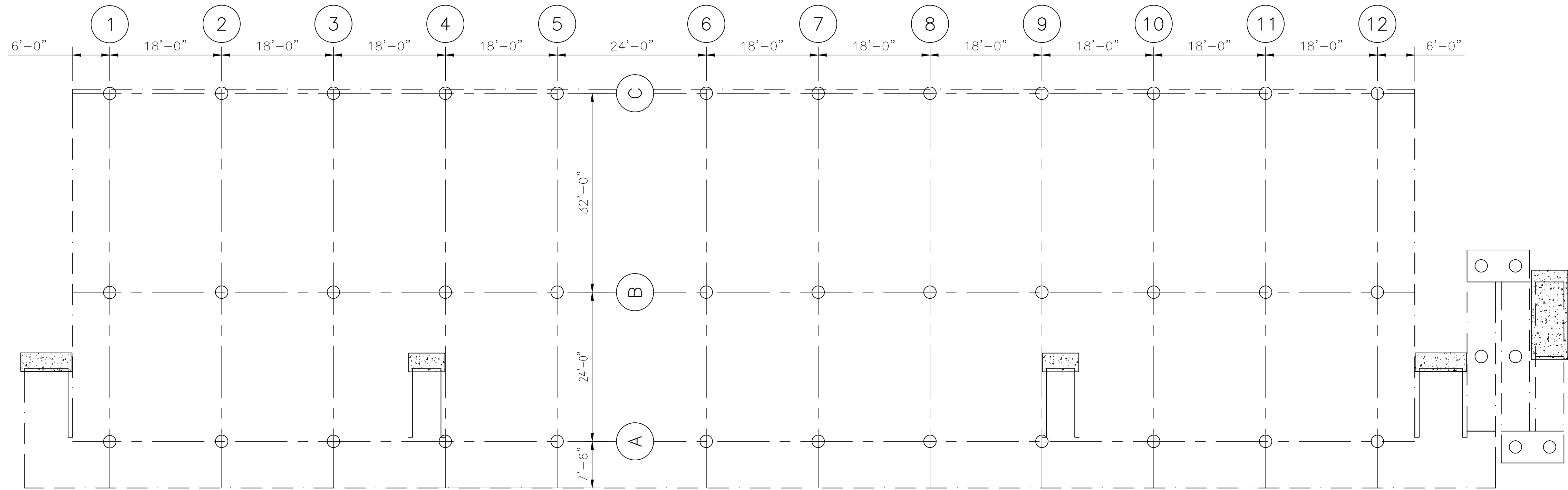
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22" X 34" (11x17 = 1/2 indicated scale)

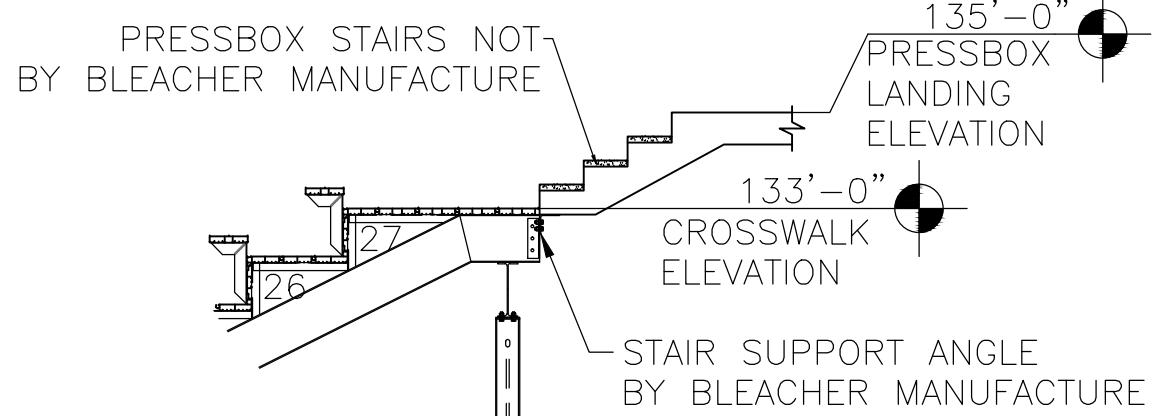


594 TOTAL NET 18" SEATS W/BACKREST
2396 TOTAL NET 18" SEATS
22 TOTAL NET 33" WHEELCHAIR SPACES
3012 TOTAL SEATING CAPACITY

HOME SEATING LAYOUT OPTION 1
SCALE: 3/32"=1'-0"



HOME FOOTING LAYOUT OPTION 1
SCALE: 3/32"=1'-0"



PRESSBOX STAIR CONN. (VIEW "A-A")
SCALE: 3/32"=1'-0"

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HOME PLAN VIEW
27 ROWS @ 24" TREAD/18 ROWS @ 36" TREAD

CARTHAGE R-9 SCHOOL DISTRICT
CARTHAGE STADIUM
CARTHAGE, MISSOURI

REV	BY	DATE	DESCRIPTION
1	CF	12/02/18	RAMP AND PRESSBOX
2	CF	1/21/19	REVISED
3	CF	12/03/19	ADDED LOADS
4	CF	11/17/19	BAND SECTION RISE & TREAD
5	CF	11/9/19	SEAT SECT./RAMPS/COLUMNS
6	CF	11/5/19	REDRAW
7	CF	10/27/19	ADDED ROWS MOVED PIERS
8	CF	8/28/19	REDRAW

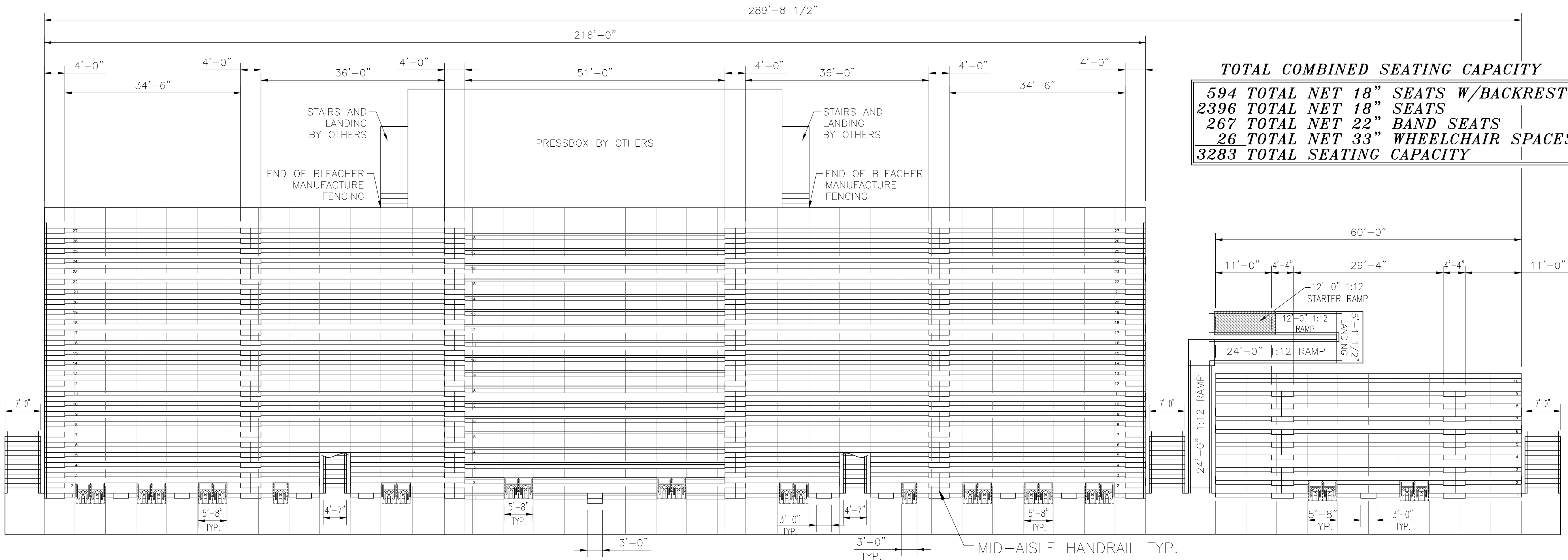
JOB NUMBER
15sd173

SHEET
6

OF
10

22" X 34" (11x17 = 1/2 indicated scale)

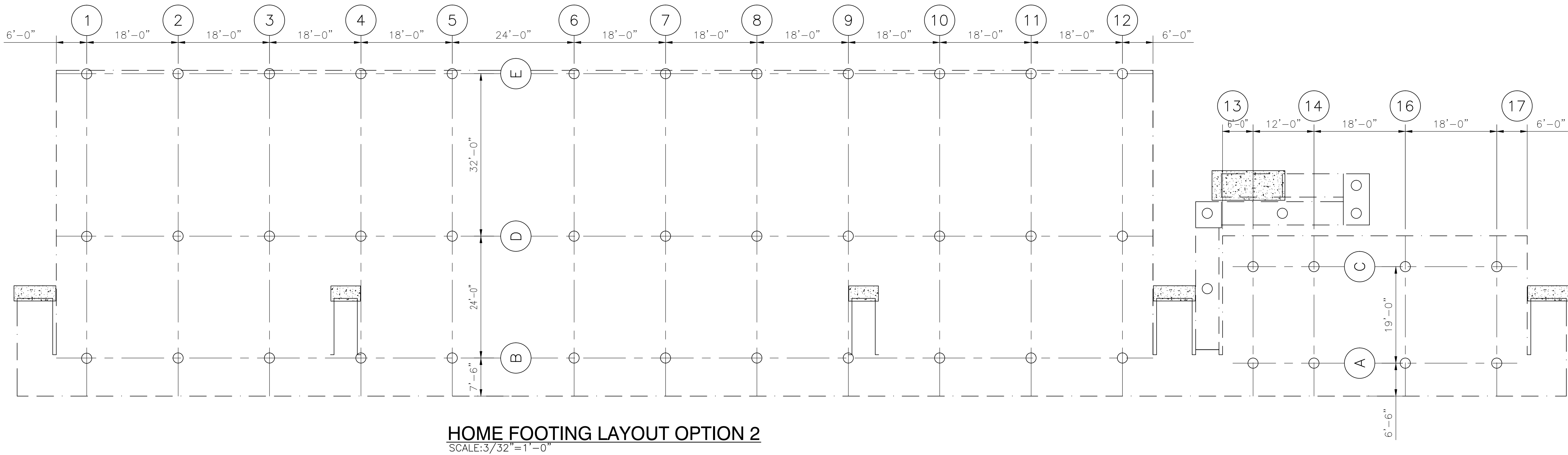




TOTAL COMBINED SEATING CAPACITY	
594	TOTAL NET 18" SEATS W/BACKREST
2396	TOTAL NET 18" SEATS
267	TOTAL NET 22" BAND SEATS
26	TOTAL NET 33" WHEELCHAIR SPACES
3283	TOTAL SEATING CAPACITY

594	TOTAL NET 18" SEATS W/BACKREST
2396	TOTAL NET 18" SEATS
22	TOTAL NET 33" WHEELCHAIR SPACES
3012	TOTAL SEATING CAPACITY

HOME SEATING LAYOUT OPTION 2
SCALE: 3/32" = 1'-0"



HOME FOOTING LAYOUT OPTION 2
SCALE: 3/32" = 1'-0"

267	TOTAL NET 22" SEATS
4	TOTAL NET 33" WHEELCHAIR SPACES
271	TOTAL SEATING CAPACITY

NOT FOR CONSTRUCTION

DETAIL NUMBER 1-110-3 SHEET SHOWN ON 310-3 SHEET CUT FROM

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HOME PLAN VIEW (OPTION 2)
283'-8 1/2" X
@ 27 ROWS @ 24" TREAD/18 ROWS @ 36" TREAD/10 ROWS @ 30" TREAD

CARTHAGE R-9 SCHOOL DISTRICT
CARTHAGE STADIUM
CARTHAGE, MISSOURI

REV	BY	DATE	DESCRIPTION
1	CF	12/02/14	PRESSBOX REVISED
2	CF	12/17/14	ADDED LOADS
3	CF	11/17/15	BAND SEAT SECTION RISE & TREAD
4	CF	11/9/15	SEAT SECT./RAMPS/COLUMNS REDRAW
5	CF	11/5/15	ADDED ROWS MOVED PIERS
6	CF	10/27/15	ADDED ROWS MOVED PIERS
7	CF	8/28/15	REDDRAW

JOB NUMBER 15sd173

SHEET 7 OF 10

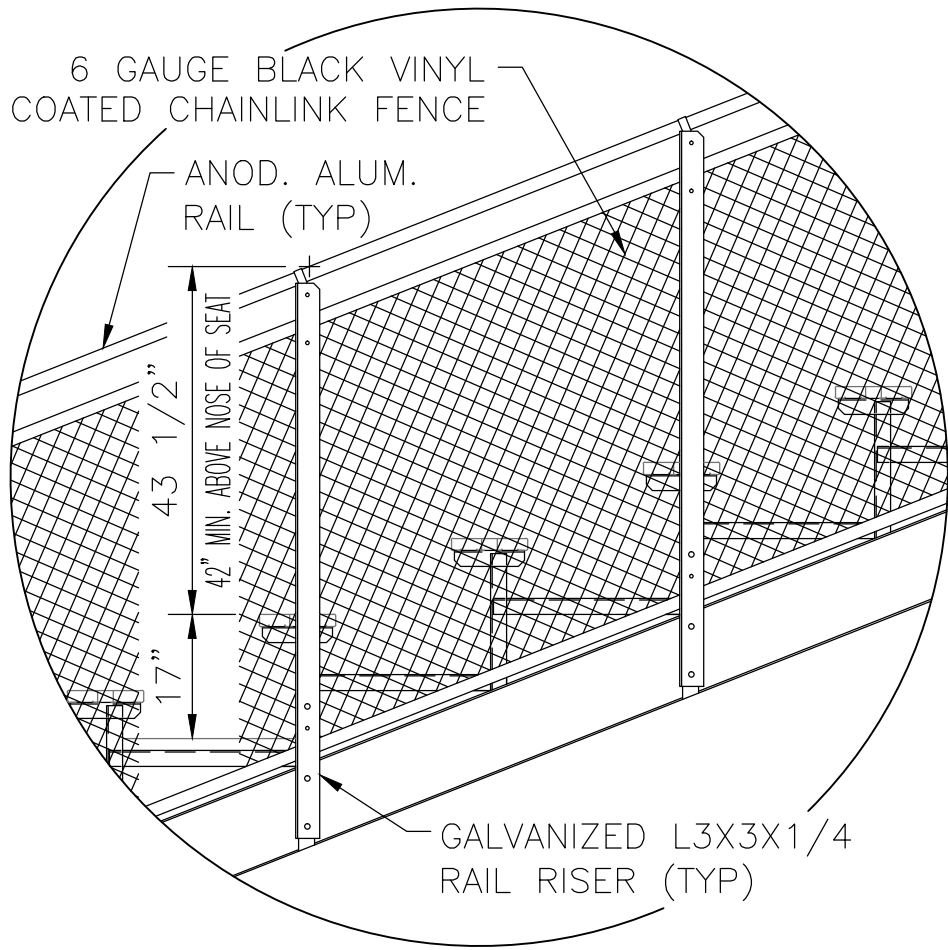
22" X 34" (11x17 = 1/2 indicated scale)

DESIGN LOADING

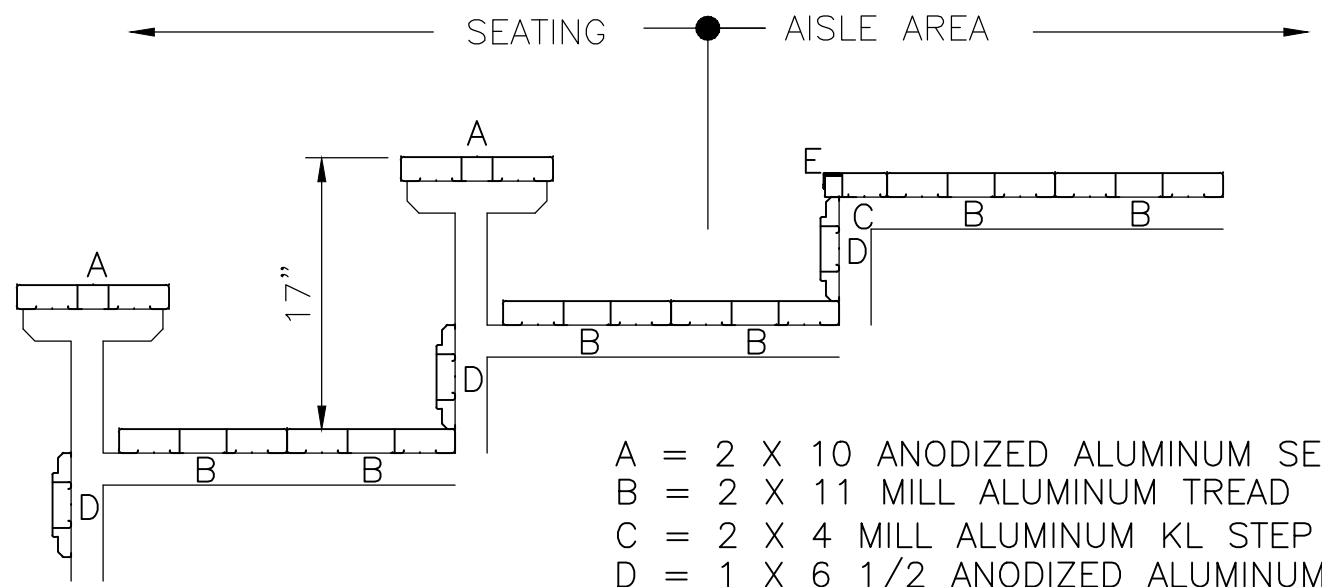
TREAD & SEAT AREA (GROSS AREA) 100psf.
SEAT (PERPENDICULAR SWAY) 10 lbs/lf.
SEAT (PARALLEL SWAY) 24lbs/lf.
HANDRAILS (PERPENDICULAR) 50 lbs/lf.
HANDRAILS (VERTICAL) 100 lbs/lf.
SEAT & TREAD 120 lbs/lf.

NOTES

1. ANODIZED ALUMINUM RAIL IS 1 1/4" NOMINAL PIPE SIZE.

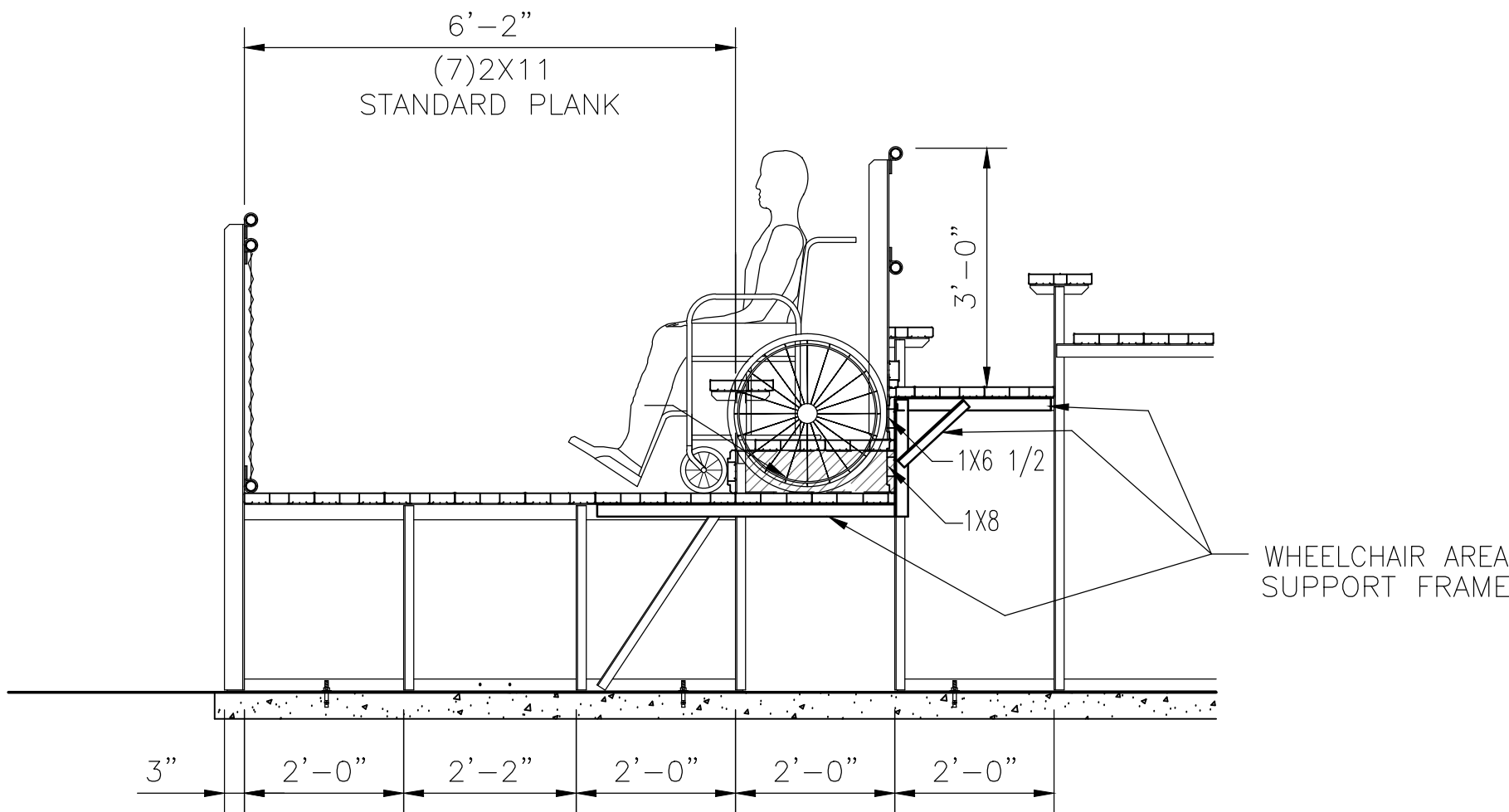


TYPICAL SIDE RAILING
SCALE: 1/2" = 1'-0"

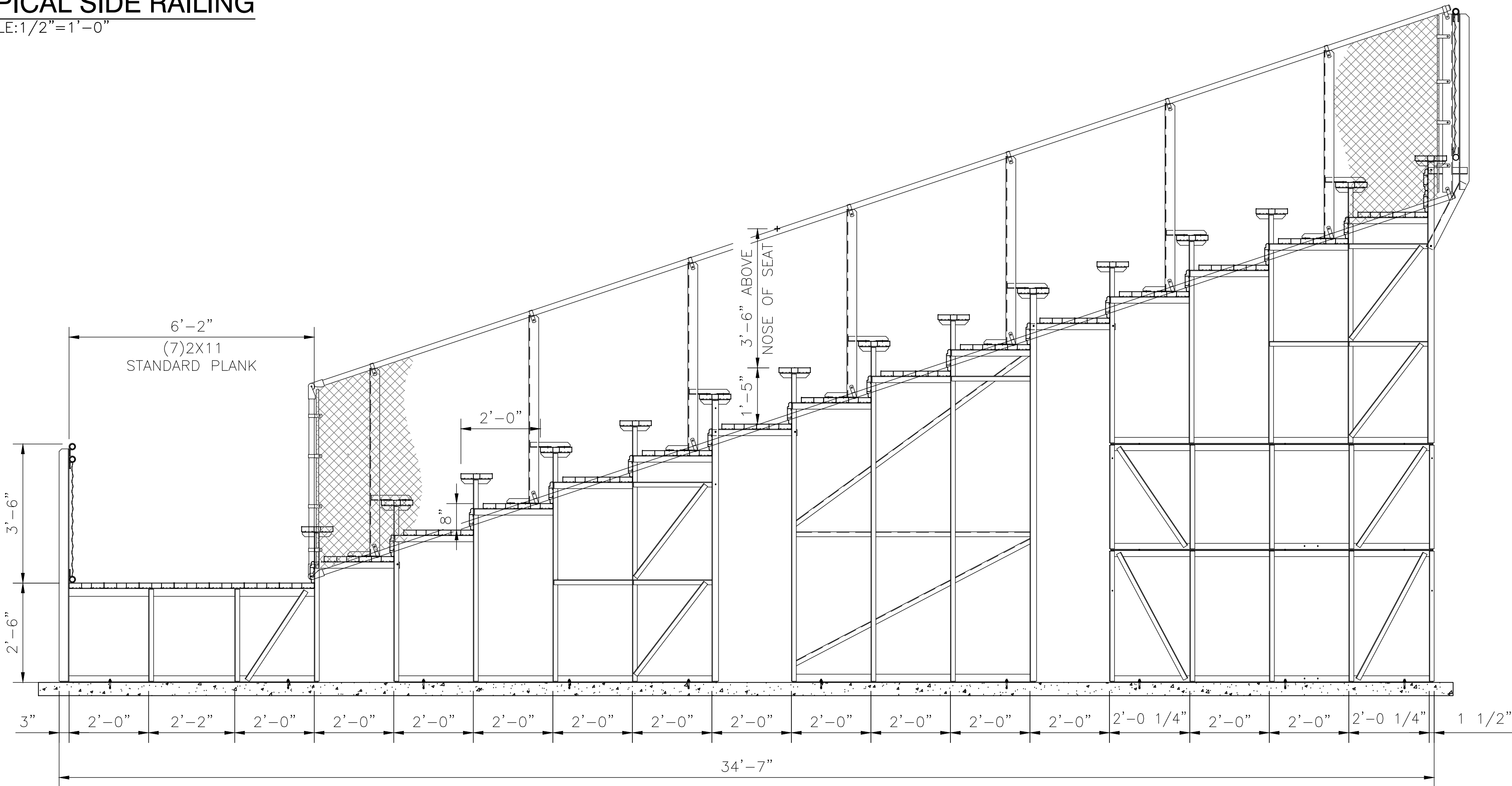


- A = 2 X 10 ANODIZED ALUMINUM SEAT
B = 2 X 11 MILL ALUMINUM TREAD
C = 2 X 4 MILL ALUMINUM KL STEP NOSE AISLE EXTENSION
D = 1 X 6 1/2 ANODIZED ALUMINUM FLAT RISER
E = CONTRASTING AISLE NOSING

PLANK ARRANGEMENT
SCALE: NTS 8" X 24"



SECTION VIEW @ W/C AREA
SCALE: 1/2" = 1'-0"



VISITOR SECTION VIEW
SCALE: 1/2" = 1'-0"

NOT FOR CONSTRUCTION

DETAIL NUMBER
1-10
SHEET SHOWN ON
3-10
SHEET CUT FROM

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VISITORS SECTION VIEW		8" RISE X 24" TREAD \ 15 ROWS		CARTHAGE R-9 SCHOOL DISTRICT		CARTHAGE STADIUM		CARTHAGE, MISSOURI	
REV	BY	DATE	DESCRIPTION	DATE	BY	DATE	DESCRIPTION	DATE	BY
1	CF	12/02/18	NONE THIS PAGE	1	CF	12/02/18	NONE THIS PAGE	1	CF
2	CF	12/17/18	NONE THIS PAGE	2	CF	12/17/18	NONE THIS PAGE	2	CF
3	CF	12/03/19	ADDED LOADS	3	CF	12/03/19	NONE THIS PAGE	3	CF
4	CF	11/17/19	NONE THIS PAGE	4	CF	11/17/19	NONE THIS PAGE	4	CF
5	CF	11/09/19	NONE THIS PAGE	5	CF	11/09/19	NONE THIS PAGE	5	CF
6	CF	11/05/19	NO CHANGES THIS PAGE	6	CF	11/05/19	NO CHANGES THIS PAGE	6	CF
7	CF	10/27/19	NO CHANGES THIS PAGE	7	CF	10/27/19	NO CHANGES THIS PAGE	7	CF
8	CF	8/28/19	REDRAW	8	CF	8/28/19	REDRAW	8	CF

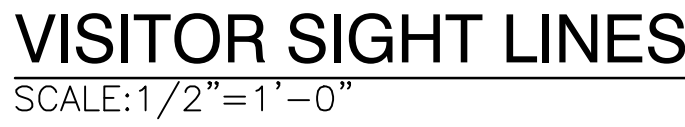
JOB NUMBER
15sd173

SHEET
8

OF
10



22" X 34" (11x17 = 1/2 indicated scale)



PC VISITORS SIGHT LINES
8" RISE X 24" TREAD\15 ROWS

FILE *CARTHAGE R-9 SCHOOL*
CARTHAGE STADIUM
CARTHAGE, MISSOURI

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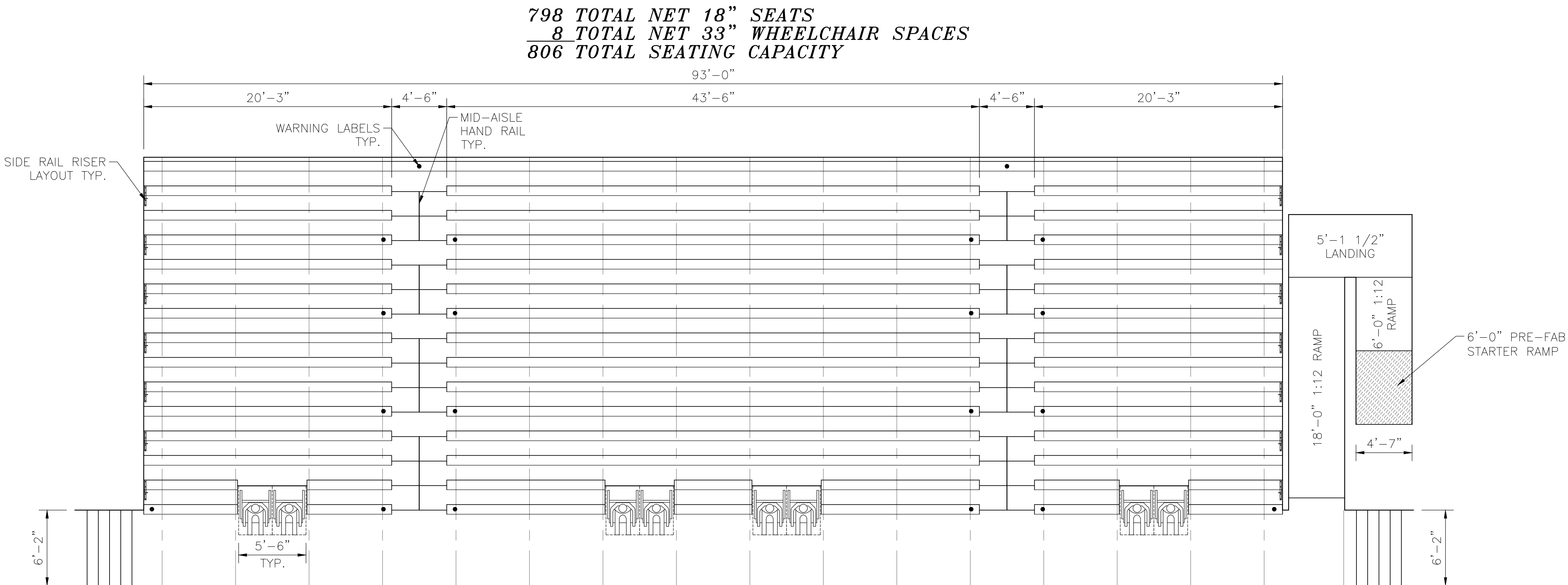
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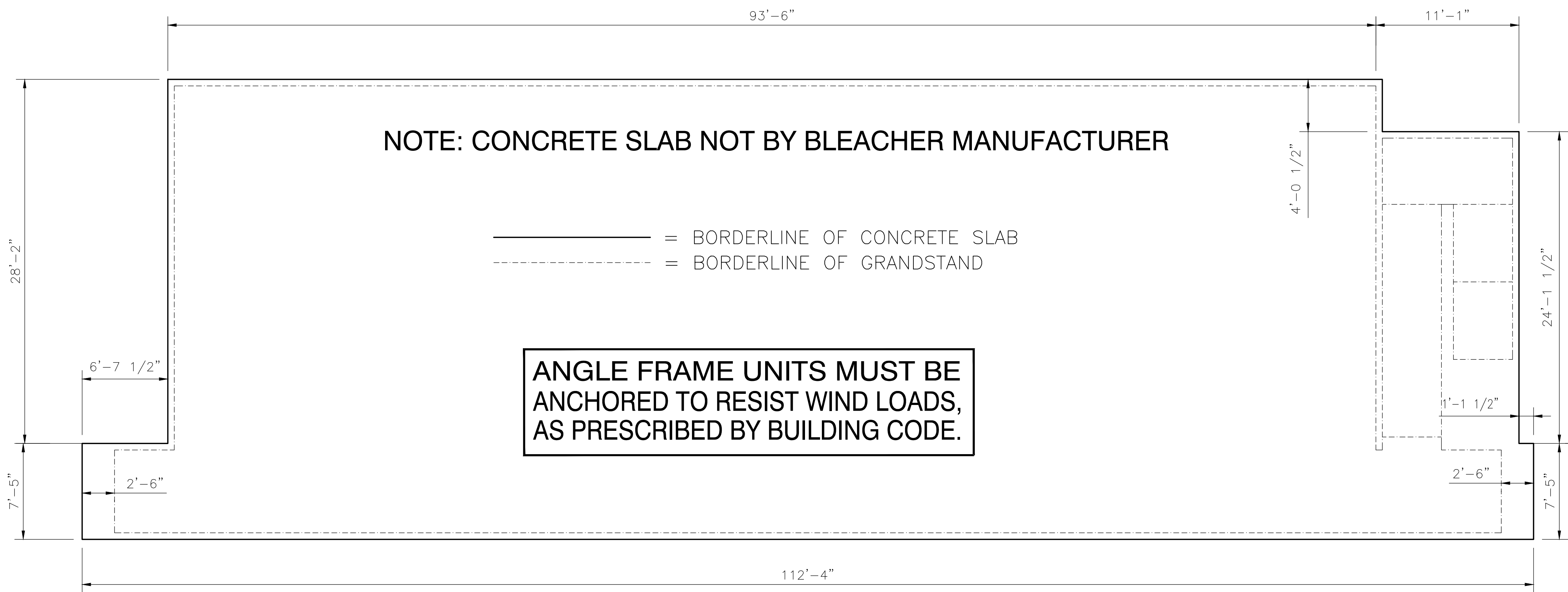
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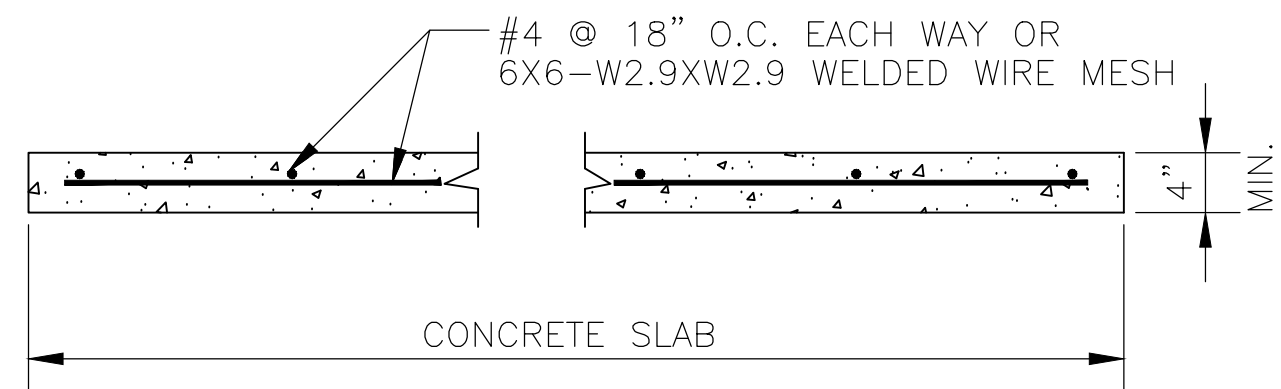




VISITORS SEATING LAYOUT
SCALE: 3/16"=1'-0"



VISITORS CONCRETE LAYOUT
SCALE: 3/16"=1'-0"



SLAB DETAIL
NOT TO SCALE

1-1
3-10
DETAIL NUMBER
SHEET SHOWN ON
SHEET CUT FROM

NOT FOR CONSTRUCTION

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TYPE VISITORS PLAN VIEW
93'-0" X 24" TREAD \ 15 ROWS

TYPE
CARTHAGE R-9 SCHOOL DISTRICT
CARTHAGE STADIUM
CARTHAGE, MISSOURI

REV	BY	DATE	DESCRIPTION	DATE	DESCRIPTION
1	CF	12/02/18	NONE THIS PAGE		
2	CF	1/21/18	NONE THIS PAGE		
3	CF	12/03/18	ADDED LOADS		
4	CF	11/17/18	NONE THIS PAGE		
5	CF	11/9/18	SEATING SECTION WIDTHS		
6	CF	11/5/18	NO CHANGES THIS PAGE		
7	CF	10/27/18	NO CHANGES THIS PAGE		
8	CF	8/28/18	REDDRAW		

JOB NUMBER
15sd173

SHEET
10

OF
10



22" X 34" (11x17 = 1/2 indicated scale)